### **Washoe County Development Application**

Your entire application is a public record. If you have a concern about releasing personal information, please contact Planning and Development staff at 775.328.3600.

Project Information	Staff Assigned Case No.:				
Project Name:	SCIL GUES-	House			
Project Description: Build mother-in-law's quarters.					
Project Address: 니긴	Bellevue D	γ.			
Project Area (acres or square fe	et): 1602 39 F	t Wlgarage			
Project Location (with point of re	eference to major cross	streets AND area locator):			
Bellivue Road ?	016 395 in	Washoe Valley			
Assessor's Parcel No.(s):	Parcel Acreage:	Assessor's Parcel No.(s):	Parcel Acreage:		
055-168-10	5.04				
Section(s)/Township/Range:	16/19				
Indicate any previous Washo Case No.(s). N/A	oe County approval	s associated with this applicat	ion:		
Applicant Inf	<b>ormation</b> (attach	additional sheets if necess	sary)		
Property Owner: 👃 🤊 ഉ	off Ronsse	Professional Consultant:			
Name: Sharri Pinnock	-120nsse, et al	Name:			
Address: 42 Ballavoce					
	Zip: 89704		Zip:		
Phone: 775-857-5446	Fax:	Phone:	Fax:		
Email: sherrironssell	otmail.com	Email:			
Cell:	Other:	Cell:	Other:		
Contact Person:		Contact Person:	-		
Applicant/Developer:		Other Persons to be Contacted:			
Name: Vaughn & Sons	Construction	Name:			
	0 Au #397	Address:			
	Zip: 19503		Zip:		
Phone: 775-324-1935	Fax:	Phone:	Fax:		
Email: service@vauchu	endsous.com	Email:			
Cell: 775-560-9444	Other:	Cell:	Other:		
Contact Person: しっしん		Contact Person:			
	For Office	Use Only			
Date Received:	Initial:	Planning Area:			
County Commission District:		Master Plan Designation(s):			
CAB(s):		Regulatory Zoning(s):			

## **Property Owner Affidavit**

Applicant Name:	Sherri	Pinnoch	Ronsse	Scott	Ronsse
The receipt of this applicat requirements of the Was applicable area plan, the will be processed.	ion at the time o	f submittal does r	not guarantee to b. the Washoe	he application of County Mas	complies with all ter Plan or the
STATE OF NEVADA Carson Cita COUNTY OF WASHOEA				_	
I, Sherri Pinn being duly sworn, depose	and say that I	am the owner*	of the property	y or properties	mvoived in this
application as listed belo information herewith submand belief. I understand Development.  (A separate Affidav	nitted are in all re that no assura	espects complete, nce or guarantee	true, and corre can be given	by members	of Planning and
Assessor Parcel Number(					T RONSSE
				Pinock P	100 552
		Sig	gned <u>Jan M</u>		
		Ac	ddress		
Subscribed and sworn  B day of Nove  Notary Public in and for s  My commission expires:	aid county and s	ol(		NOTA STATE	DEL RAZO ARY PUBLIC OF NEVADA Exp.: 07-18-2020 e No: 16-3196-3
☐ Power of Attorne☐ Owner Agent (Pr	r/Partner (Provid y (Provide copy ovide notarized	e copy of record of Power of Attornated from proper record document	document indic ney.) ty owner giving	legal authority	
• • •		with Stewardship			

### **Administrative Review Permit Application** for a Detached Accessory Dwelling **Supplemental Information**

(All required information may be separately attached)

Ru 110 the Aco	is application is for proposals to establish a Detached Accessory Dwelling unit in the Low Density ral, Medium Density Rural, High Density Rural, and Low Density Suburban regulatory zones. Chapter of the Washoe County Code is commonly known as the Development Code. Specific references to administrative review permit process for Detached Accessory Dwellings may be found in Article 306, dessory Uses and Structures, Section 25(i). A Detached Accessory Dwelling is also referred to as a accondary dwelling" in this application. The "main dwelling" is the original or larger dwelling on the operty.
1.	What is the size (square footage) of the main dwelling unit or proposed main dwelling unit (exclude size of garage)?
	3376
2.	What is the size of the detached accessory dwelling unit or proposed detached accessory dwelling unit (exclude size of garage)?
	1140
3.	How are you planning to integrate both the main dwelling and secondary dwelling to provide architectural compatibility and a sense of project integration of the two structures?
	architectural compatibility and a sense of project integration of the two structures?  The access ony dwelling will be built to closely moster the avointectural style of the existing structure.
	Structure.
4.	How are you planning to provide water and wastewater disposal (sewer or septic) to the secondary dwelling unit?
	In accordance with Washop County Health Dept., we will whilize the existing well & reach field, and

5. What additional roadway, driveway, or access improvements are you planning?

the house will utilize the existing driveway with an additional 40' driveway spur.

6. A parking space is required. How are you providing the additional parking?

The house includes a 2-car garage.

7. When do you plan to complete construction of the secondary dwelling and obtain a certificate of occupancy?

July 2017

8. What will you do to minimize any potential negative impacts (e.g. increased lighting, obstruction of views, removal of existing vegetation, etc.) your project may have on adjacent properties?

The proposed site is currently cleared land and the adjacent house's view is not impacted.

9.	Is the subject property part of an active Home Owners Association (HOA) or Architectural Contro Committee? If yes, please include the name and contact information for the applicable board.
	No.
	Are there any restrictive covenants, recorded conditions, or deed restrictions (CC&Rs) that may prohibit a detached accessory dwelling on your property?
	☐ Yes 💯 No If yes, please attach a copy.
	Only one accessory dwelling unit, whether attached or detached, is allowed per parcel. Please verify that an accessory dwelling (i.e. secondary dwelling) currently does not exist on the subject property.
	No current accessory dwelling exists.
	List the age and size of the unit If you plan to utilize a manufactured or modular home as the secondary dwelling. (Note: manufactured or modular homes must be permanently affixed and converted to real property.)
	N/A
13.	List who the service provider will be for the following utilities:
	a. Sewer Service privati septic
	b. Electrical Service W Engray
	c. Solid Waste Disposal Service Waste Management
	d. Water Service private well
	ı

Washoe County Treasurer P.O. Box 30039, Reno, NV 89520-3039 ph: (775) 328-2510 fax: (775) 328-2500 Email: tax@washoecounty.us

Washoe County Treasurer Tammi Davis

#### Bill Detail

Change of Address	Print this Page
	12 12 · Marco il manusci
Status	Last Update
Active	10/29/2016 2:09:37 AM
SITUS 42 BE WCTY	LLEVUE RD
Geo (	CD:
Legal Description	WAR
	Status Active  SITU: 42 BE WCTY

#### Installments

		Total Due:	\$2,051.62	\$0.00	\$0.00	\$2,051.62
INST 4	3/6/2017	2016	\$1,025.81	\$0.00	\$0.00	\$1,025.81
INST 3	1/2/2017	2016	\$1,025.81	\$0.00	\$0.00	\$1,025.81
INST 2	10/3/2016	2016	\$0.00	\$0.00	\$0.00	\$0.00
INST 1	8/15/2016	2016	\$0.00	\$0.00	\$0.00	\$0.00
Period	Due Date	Tax Year	Tax	Penalty/Fee	Interest	Total Due

#### Tax Detail

	Gross Tax	Credit	Net Tax
State of Nevada	\$275.49	(\$60.21)	\$215.28
Truckee Meadows Fire Dist	\$875.07	(\$191.24)	\$683.83
Washoe County	\$2,255.28	(\$492.88)	\$1,762.40
Washoe County Sc	\$1,844.94	(\$403.21)	\$1,441.73
Total	Tax <b>\$5,250.78</b>	(\$1,147.54)	\$4,103.24

,	Payment History							
Tax Year	Bill Number	Receipt Number	Amount Paid	Last Paid				
2016	2016077798	B16.109312	\$1,025.81	10/3/2016				
2016	2016077798	B16.62935	\$1,025.81	8/22/2016				

#### Pay By Check

Please make checks payable to: WASHOE COUNTY TREASURER

Mailing Address: P.O. Box 30039 Reno, NV 89520-3039

Overnight Address: 1001 E. Ninth St., Ste D140 Reno, NV 89512-2845

#### Change of Address

All requests for a mailing address change must be submitted in writing, including a signature (unless using the online form).

To submit your address change online <u>click here</u>

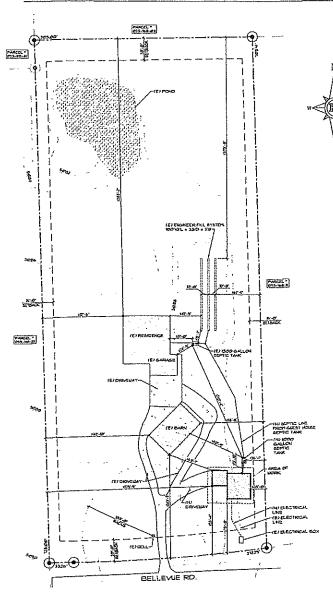
Address change requests may also be faxed to: (775) 328-2500

Address change requests may also mailed to: Washoe County Treasurer P O Box 30039 Reno, NV 89520-3039

The Washoe County Treasurer's Office makes every effort to produce and publish the most current and accurate information possible. No warranties, expressed or implied, are provided for the data herein, its use, or its interpretation. If you have any questions, please contact us at (775) 328-2510 or tax@washoecounty.us

This site is best viewed using Google Chrome, Internet Explorer 11, Mozilla Firefox or Safari.

## PINNOCK GUEST HOUSE



SITE PLAN

CONTACT INFORMATION

JOHN VALGIN

Vaugur 4 sors construction, Inc. 561 Keystone ave. 291 Reno, Ny 88503

PHONE: 175-324-7535 FAX: 215-624-8572

EHAIL: SERVICE \*VAUGHNANDSCNS.COM

#### PROJECT INFORMATION PARCEL.

PARCEL AREA : 20542 SF. ZORNO : FOR

BUILDING

EXISTING RESIDENCE . 3,310 SF EXISTING GARAGE . 616 SF.

PROPOSED GUEST HOUSE : 1(NO SE. ATTACHED GARAGE : 1443 SE.



VICINITY MAP

engineer OF RECORD K2 ENGINEERING

Béo maestro dr. Suite a Reno, ny 6158

775-355-¢565 - ¢FFKE 775-355-¢566 - FAX

#### SHEET MDEX

- A-O BITE PLAN A-I FLOOR PLAN A-2 ELEVATIONS A-3 SECTIONS
- FOUNDATION PLAN FLOOR AND DECK FRANCE PLAN SHEARMALL PLAN ROOF FRANCE PLAN
- E-I ELECTRICAL PLAN
- 50-1 Structural notes and 5chedules 50-2 Structural Details 50-3 Structural Details

SCOPE OF WORK
A NEW 1190 SF. DETACHED GUEST HOUSE BITH 4447 SF.
ATTACHED GARAGE.

- CHEMIERAL SITE NOTES

  L ALL MORE HAS CONFORM OF LOCAL DRAIDING CODES, CITY, CONTY
  AND STATE COPUNIONS SERVICING ATTOCK AND THE
  INTERNATIONAL DRAIDING CODE, STEEPARTONAL RESPONMENT, CODE
  INTERNATIONAL DRAIDING CODE, STEEPARTONAL RESPONMENT, CODE
  INTERNATIONAL DRAIDING CODE, CODE (2019 EDITIONS) AND
  THE 2018 HATCHING CODE (2019 CODE (2019 EDITIONS) AND
- CONTRACTOR SHALL FIELD VERST ALL EXCENS GRADES HIGHES, AND DEPENDING PRIOR TO THE START OF CONSTRUCTION.
- The Contractor is responsible for establishes and comparing all service regularization in the appropria Prolocae Conscisos shall be made in accompance with all applicable codes and recompance with all applicable codes and recompance.
- Placeton of Stricture With Setbacks shall be the Responsibilit of the General Contractor Hotey Coner of any discrepances.
- 5. CONC PLATHORS TO BE FINEHED PER OUTERS REQUIREMENTS.
- 6. SLOPE LAUN AREAS FOR DRAWAGE MIN, DAT PER 1-0".
- HANTAN EXISTS DRAMAGE WIN 58 (78 HN) SLOPE WAY FROM PROPOSED STRUCTURE FOR A TRICTUM OF BY AND CRANAGE BULLE 716 HN HEATH RECORDED TO PROVIDE DRAMAGE BULLE DRAMAGE GUTTA ADJACTIVE PRIVATE PROPERTY, MINTAN BLOPE OF DRAMAGE SOLITE BULLE WIN.
- THIS SITE IS LOCATED IN FOTA FLOOD ZONE X EHICH IS DETERMINED TO BE DUISIDE OF THE WO TELL FLOOD PLAN BONDART OF THE WONLY FLOOD PLAN FLOOD PLAN FLOOD PLAN FLOOD PLAN FLOOD FLOOR FLOOD WITHIN WO FEET OF PROPERTY.
- THIS WIE IS SERVICED BY ON-SITE WELL, AND REPTIC SYSTEM
- THE DESIGN FOR THIS SITE MAS DEEN BLASED OF THE BEST AVAILABLE NORMATION. ALL ASSENDE DESIGNS PROTUTION AND ARRESTED OF THE COMPLICATION OF CONSTRUCTION AND ARREST OF THE COMPLICATION OF CONSTRUCTION AND ARREST OF THE ATTEMPT OF THE ATTEMPT OF THE DESIGNATION OF THE DESIGN OF THE ATTEMPT OF THE DESIGNATION OF THE DE
- NEITHER A TOPOCOLANDOLL OR COLONING ABOVE THE PREVENTION OF THE SITE. IN EXPONENCE HE AND AS FROM THE WAS TO THE SITE OF THE STORE AND ASSOCIATE MADE IN OUR RECORD OF THE MEDITE HIS THE OWNER OF THE PROPERTY CONSTRUCTION A PILL SERVET IS PERSONNERED IN A PROPERTY ACCUMATELY FACE THE PROPERTY.
- MACLD ANY PREMIORIC OR HISTORIC REPANALARIEFACTS EE DISCOVERD DURNO SHE DEVELOPMENT, EDRIC GHALL TO TOPPOCHATELY SE MALTER AT THE SPECIES DIE FAND THE STATE HISTORIC FAND ARTS. BALL ER HOTHER TO RECORD ON EXCEPTION OF A STATE MACLD SHE HOTHER OF THE STATE HISTORIC FAND ARTS. BALL ER HOTHER TO RECORD ON EXCEPTION OF THE STATE HISTORIC FAND AT STATE HISTORIC FAND AT



560 Maestro Dr., Sto. A Reno, NV \$2511 P: (775) \$55-0508 F: (775) 389-0500

House

St See

Pinnock

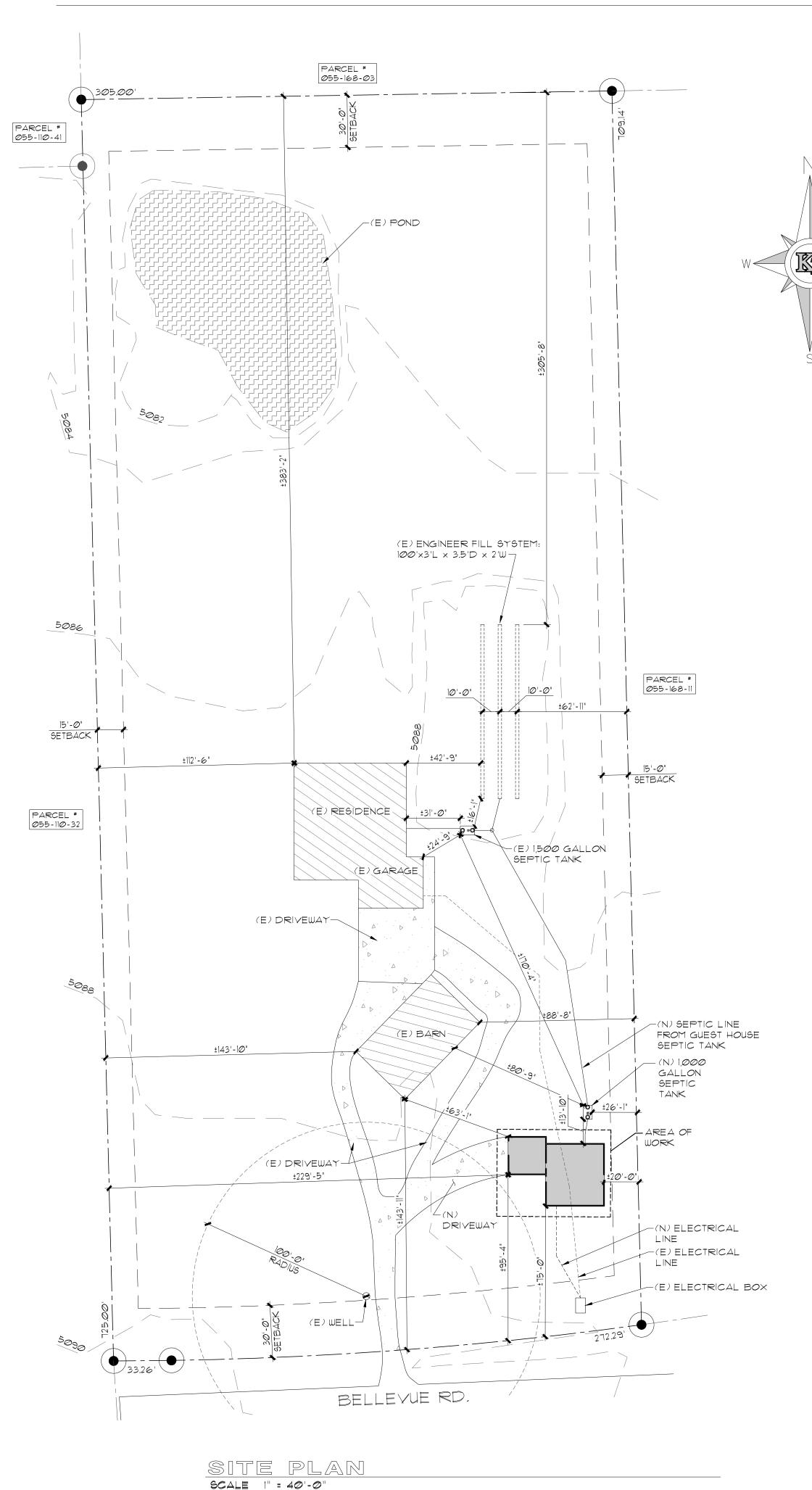
Branck T. Kennedy, P.E. Jarud A. Krupa, P.E.

Revisions

11/12/16 Drawn K3 Checked Project No. 16-26l

Cover Sheet/ Site Plan

# PINNOCK GUEST HOUSE





SCALE: N.T.S.

K2 ENGINEERING

860 MAESTRO DR., SUITE A RENO, NV 89511

775-355-0505 - OFFICE 775-355-*0*566 - FAX

A-Ø SITE PLAN A-I FLOOR PLAN A-2 ELEVATIONS

A-3 SECTIONS

S-1 FOUNDATION PLAN S-2 FLOOR AND DECK FRAMING PLAN

S-3 SHEARWALL PLAN 5-4 ROOF FRAMING PLAN

E-I ELECTRICAL PLAN

SD-1 STRUCTURAL NOTES AND SCHEDULES SD-2 STRUCTURAL DETAILS

SD-3 STRUCTURAL DETAILS

CONTACT INFORMATION

JOHN YAUGHN

VAUGHN & SONS CONSTRUCTION, INC.

561 KEYSTONE AVE. #397 RENO, NV 89503

PHONE: 175-324-1935

FAX: 775-624-8572

EMAIL: SERVICE@VAUGHNANDSONS.COM

PROJECT INFORMATION

PARCEL

42 BELLEYUE RD.

RENO, NV 89704

APN: 055-168-10

PARCEL AREA = 219,542 S.F.

ZONING = MDR

BUILDING

EXISTING RESIDENCE = 3,370 S.F.

EXISTING GARAGE = 676 S.F.

PROPOSED GUEST HOUSE = ±1,160 S.F.

ATTACHED GARAGE = ±442 S.F.

SCOPE OF WORK

A NEW ±1,160 S.F. DETACHED GUEST HOUSE WITH ±442 S.F. ATTACHED GARAGE.

- ALL WORK MUST CONFORM W/ LOCAL BUILDING CODES, CITY, COUNTY AND STATE ORDINANCES, SUBDIVISION REGULATIONS AND THE INTERNATIONAL BUILDING CODE, INTERNATIONAL RESIDENTIAL CODE, UNIFORM MECHANICAL CODE, UNIFORM PLUMBING CODE INTERNATIONAL ENERGY CONSERVATION CODE (2012 EDITIONS) AND THE 2011 NATIONAL ELECTRICAL CODE.
- 2. CONTRACTOR SHALL FIELD VERIFY ALL EXISTING GRADES, UTILITIES, AND DIMENSIONS PRIOR TO THE START OF CONSTRUCTION.
- 3. THE CONTRACTOR IS RESPONSIBLE FOR ESTABLISHING AND COORDINATING ALL SERVICE REQUIREMENTS WITH THE APPROPRIATE PUBLIC AGENCY OR UTILITY PROVIDER. CONNECTIONS SHALL BE MADE IN ACCORDANCE WITH ALL APPLICABLE CODES AND REQUIREMENTS.
- 4. PLACEMENT OF STRUCTURE WITHIN SETBACKS SHALL BE THE RESPONSIBILITY OF THE GENERAL CONTRACTOR, NOTIFY OWNER OF ANY DISCREPANCIES.
- 5. CONC. FLATWORK TO BE FINISHED PER OWNERS REQUIREMENTS.
- 6. SLOPE LAWN AREAS FOR DRAINAGE MIN. 1/4" PER 1'-0".
- 1. MAINTAIN EXISTING DRAINAGE WITH 5% (2% MIN.) SLOPE AWAY FROM PROPOSED STRUCTURE FOR A MINIMUM OF 10' AND DRAINAGE SWALE 2'-6" MIN. IN FROM PROPERTY LINES AS REQUIRED TO PREVENT DRAINAGE ONTO ADJACENT PRIVATE PROPERTY. MINIMUM SLOPE OF DRAINAGE SWALE SHALL BE 1%.
- 8. THIS SITE IS LOCATED IN FEMA FLOOD ZONE X WHICH IS DETERMINED TO BE OUTSIDE OF THE 100 YEAR FLOOD PLAIN. BOUNDARY OF THE 100-YEAR FLOOD PLAN IS NOT WITHIN 100 FEET OF PROPERTY.
- 9. THIS SITE IS SERVICED BY ON-SITE WELL AND SEPTIC SYSTEM.
- 10. THE DESIGN FOR THIS SITE HAS BEEN BASED ON THE BEST AVAILABLE INFORMATION. ALL ASSUMED EXISTING INFORMATION SHALL BE VERIFIED BY THE CONTRACTOR PRIOR TO CONSTRUCTION. ALL PARTIES SHOULD ANTICIPATE THE POTENTIAL NEED FOR MODIFICATIONS TO THE INITIAL DESIGN IN ORDER TO ACCOMMODATE ACTUAL FIELD CONDITIONS. ALL DISCREPANCIES DISCOVERED IN THE FIELD SHALL BE BROUGHT TO THE ATTENTION OF THE ENGINEER IMMEDIATELY.
- II. NEITHER A TOPOGRAPHICAL OR BOUNDARY SURVEY WAS PERFORMED FOR THIS SITE. THE TOPOGRAPHY SHOWN IS FROM THE WASHOE COUNTY GIS WEBSITE AND THE BOUNDARY SHOWN IS BASED ON RECORD INFORMATION. IT IS RECOMMENDED THAT PRIOR TO CONSTRUCTION A FULL SURVEY IS PERFORMED IN ORDER TO ACCURATELY PLACE THE IMPROVEMENTS.
- 12. SHOULD ANY PREHISTORIC OR HISTORIC REMAINS/ARTIFACTS BE DISCOVERED DURING SITE DEVELOPMENT, WORK SHALL TEMPORARILY BE HALTED AT THE SPECIFIC SITE AND THE STATE HISTORIC PRESERVATION OFFICE OF THE DEPARTMENT OF MUSEUMS, LIBRARY AND ARTS, SHALL BE NOTIFIED TO RECORD AND PHOTOGRAPH THE SITE. THE PERIOD OF TEMPORARY DELAY SHALL BE LIMITED TO A MAXIMUM OF TWO (2) WORKING DAYS FROM THE DATE OF NOTIFICATION.



860 Maestro Dr., Ste. A Reno, NV 89511 P: (775) 355-0505 F: (775) 355-0566 www.K2eng.net

•

Brandt T. Kennedy, P.E.

Jared A. Krupa, P.E.

Revisions

11/11/16

Checked

Project No.

BTK

16-261

Cover Sheet/ Site Plan

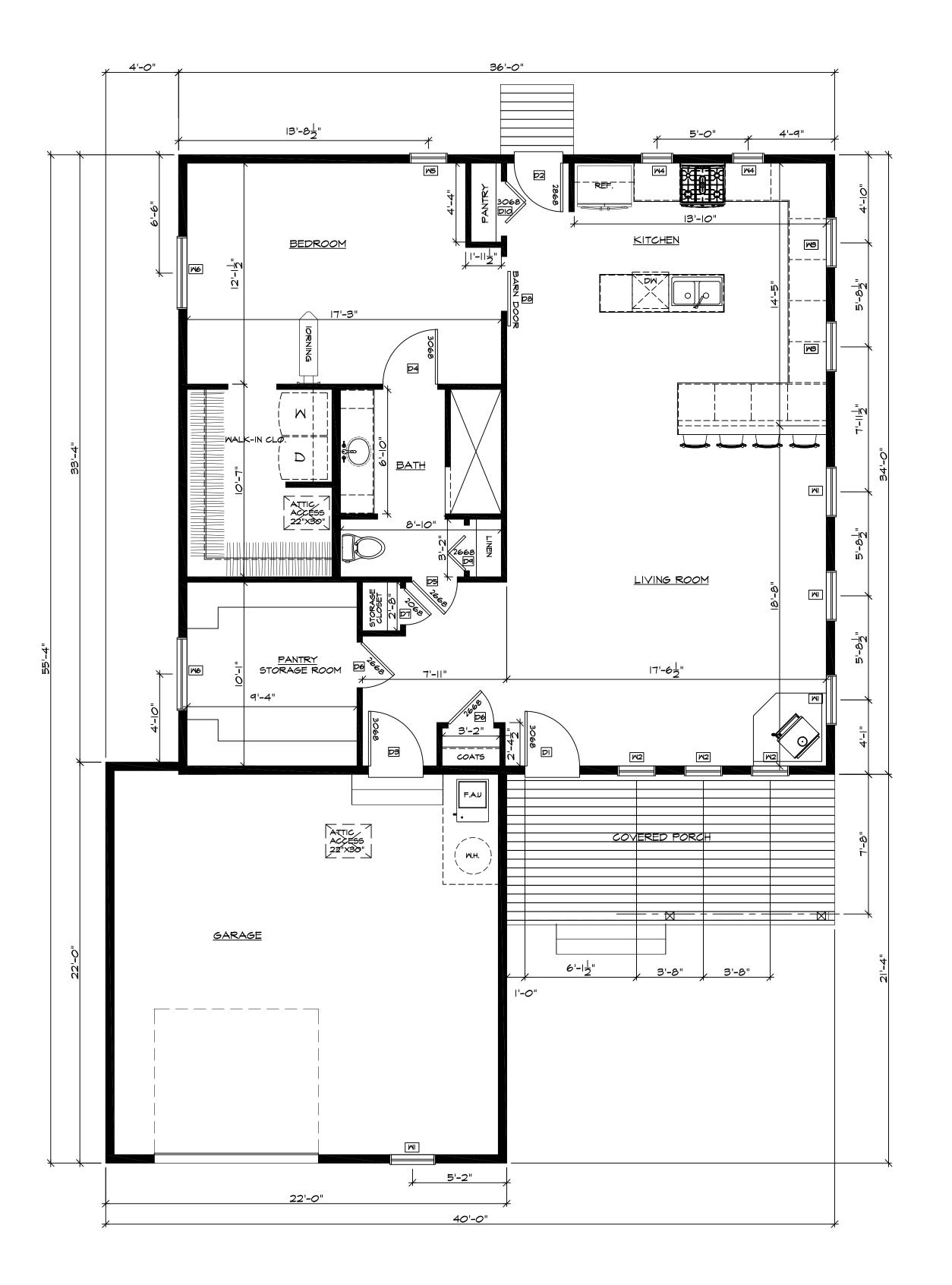


## MINDOM SCHEDULE

KEY	MANF'R.	SERIES/TYPE	SIZE	GLASS	GRILLS	JAMB	QTY
MI		DOUBLE HUNG	2953	SEE NOTES	NONE	6-9/16"	4
M2		DOUBLE HUNG	2553	SEE NOTES	NONE	6-9/16"	3
MS		DOUBLE HUNG	2535	SEE NOTES	NONE	6-9/16"	2
M4		DOUBLE HUNG	2135	SEE NOTES	NONE	6-9/16"	2
M5		DOUBLE HUNG	2547	SEE NOTES	NONE	6-9/16"	I
M6		SLIDING	4040	SEE NOTES	NONE	6-9/16"	2

## DOOR SCHEDULE

KEY	SIZE	HANDING	MANF'R.	MODEL OR SERIES	JAMB	QTY.
ō	3068	LH IN-SMING		ENTRY DOOR WITHRESHOLD	6-9/16"	I
D2	2868	LH IN-SMING		ENTRY DOOR W/THRESHOLD	6-9/16"	ı
<b>D3</b>	3068	LH IN-SMING		STEEL, FIRE RATED WITH THRESHOLD, SELF CLOSING	6-9/16"	ı
D4	3068	RH		CRAFTSMAN STYLE	4-9/16"	I
D5	2668	RH		CRAFTSMAN STYLE	4-9/16"	I
D6	2668	LH		CRAFTSMAN STYLE	4-9/16"	2
<b>D</b> 7	2068	LH		CRAFTSMAN STYLE	4-9/16"	
DВ	38"	TRACK		CUSTOM SINGLE BARN DOOR (BY OWNER)		ı
Da	2668	BIFOLD		CRAFTSMAN STYLE	4-9/16"	ı
DIO	3068	BIFOLD		CRAFTSMAN STYLE	4-9/16"	I



FLOOR PLAN

1/4"=FOOT

SQUARE FOOTAG	<del>J</del> E
FIRST FLOOR AREA	1160 SQ.FT.
TOTAL LIVING AREA HOUSE	1160 SQ.FT.
GARAGE AREA	442 SQ.FT.
COVERED PORCH AREA	144 SQ.FT.

SHEET INDEX

AI- FLOOR PLAN
A2- ELEVATIONS
A3- CROSS SECTIONS
SI- FOUNDATION PLAN
S2- FLOOR FRAMING
S3- SHEARWALL PLAN
S4- ROOF FRAMING PLAN

EI- ELECTRICAL PLAN

SDI- STRUCTURAL DETAILS SD2- STRUCTURAL DETAILS SD3- STRUCTURAL DETAILS

Drawn by: V.C. BUILDERS DESIGN SERVICE

No. Revision/Issue Date

RENO, NV. 775-324-1935

VAUGHN & SONS CONSTRUCTION

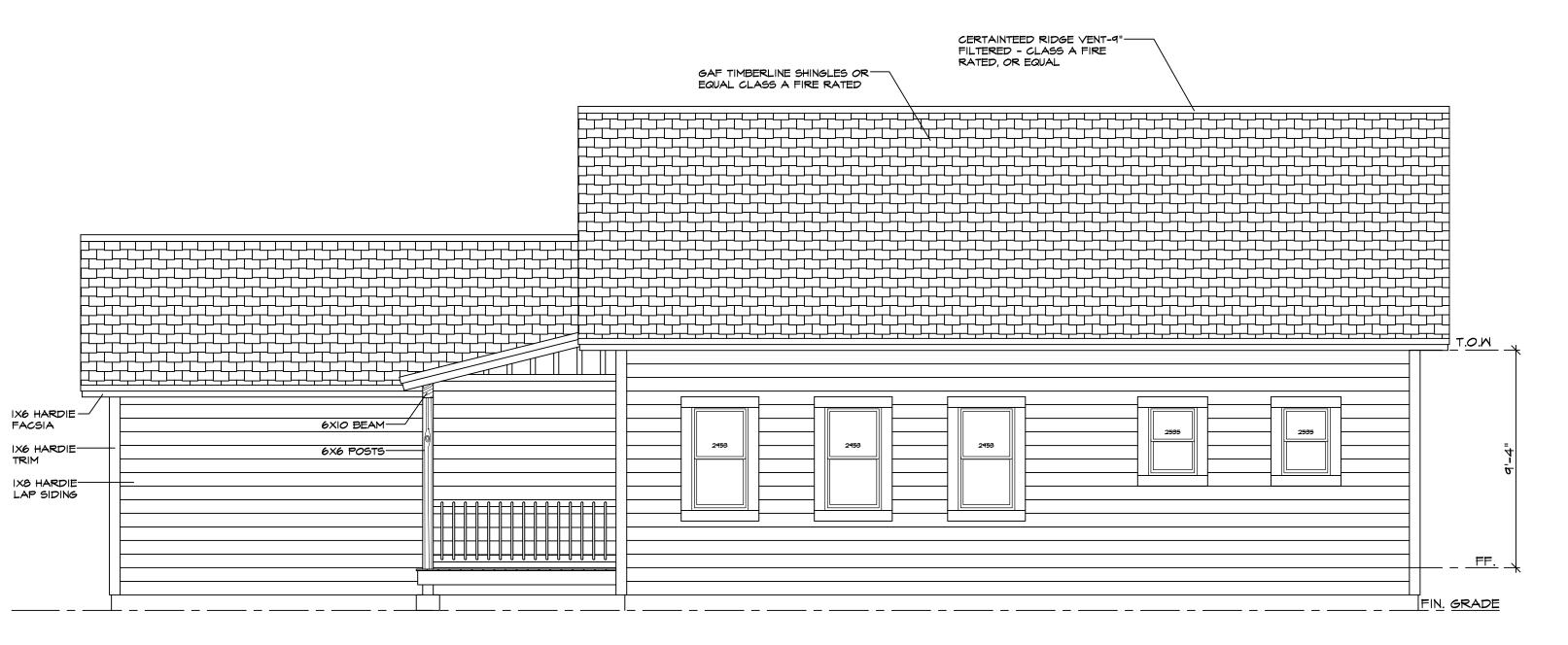
Project Name and Address

JoANN PINNOCK

GUEST HOUSE

42 BELLEVUE RD. CARSON CITY NV. 89704 WASHOE COUNTY

Project	Sheet
Pate 9-1-16	<b>A</b> 1
Scale 1/4"=F00T	



DAIZ BRANDAURD

NUDPIRE COMPLIANT

OABLE VENT

TON

TON

TON

ENT. LIGHT

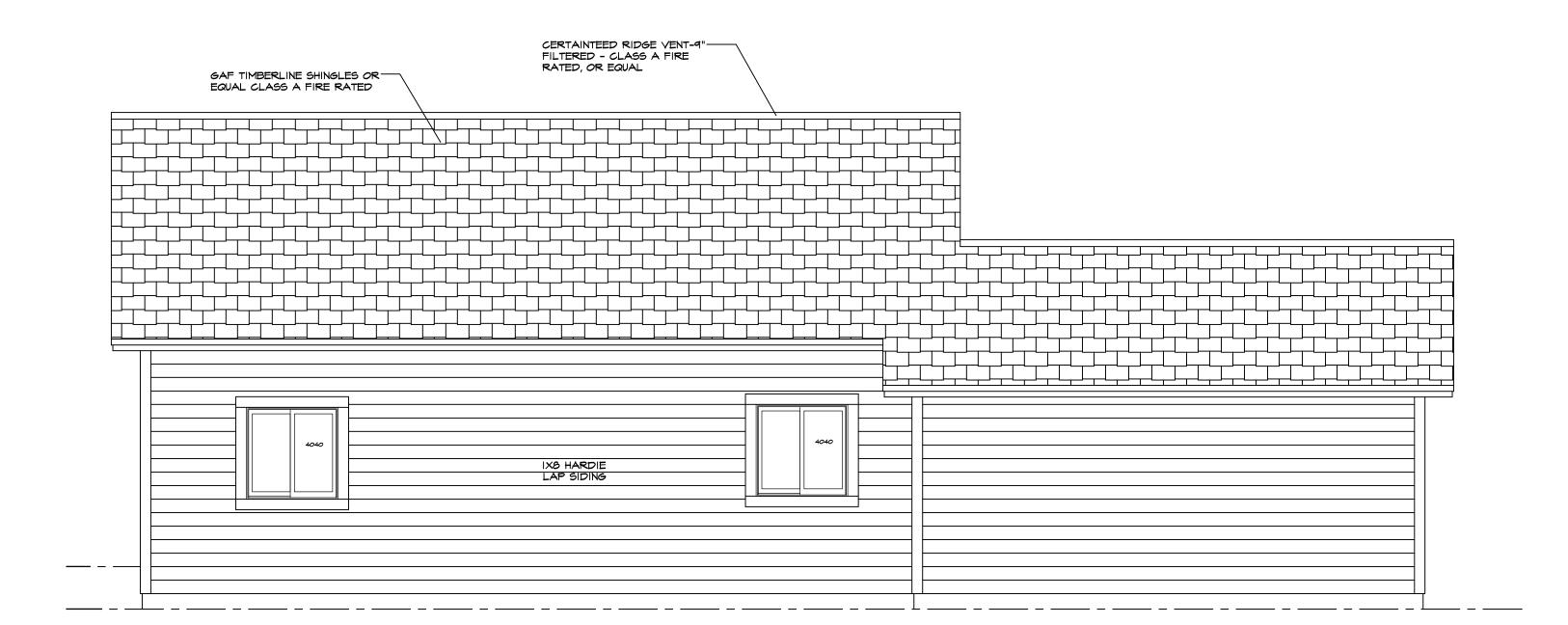
FIN. GRADE

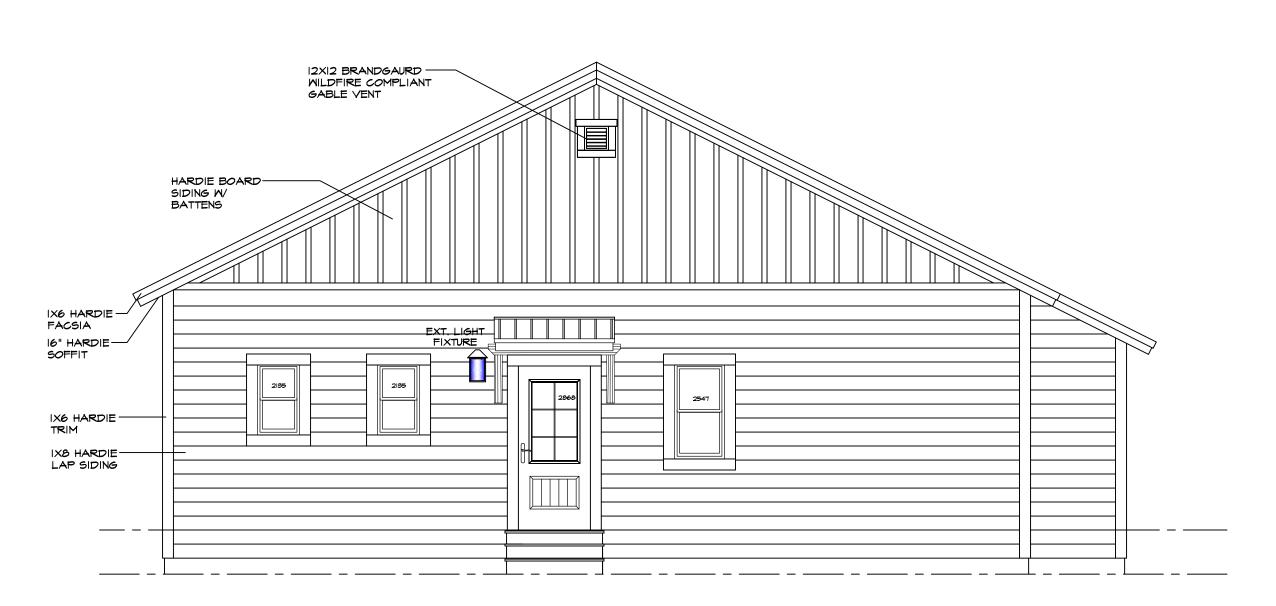
FIN. GRADE

RIGHT/SOUTH ELEVATION
1/4"=FOOT

FRONT/MEST ELEVATION

1/4"=FOOT





LEFT/NORTH ELEVATION

1/4"=FOOT

REAR/EAST ELEVATION

1/4"=FOOT

SHEET INDEX

AI- FLOOR PLAN
A2- ELEVATIONS
A3- CROSS SECTIONS

SI- FOUNDATION PLAN
S2- FLOOR FRAMING
S3- SHEARWALL PLAN

SDI- STRUCTURAL DETAILS SD2- STRUCTURAL DETAILS SD3- STRUCTURAL DETAILS

EI- ELECTRICAL PLAN

54- ROOF FRAMING PLAN

Drawn by: V.C. BUILDERS DESIGN SERVICE

lo. Revision/Issue Data

VAUGHN & SONS CONSTRUCTION

RENO, NV. 775-324-1935

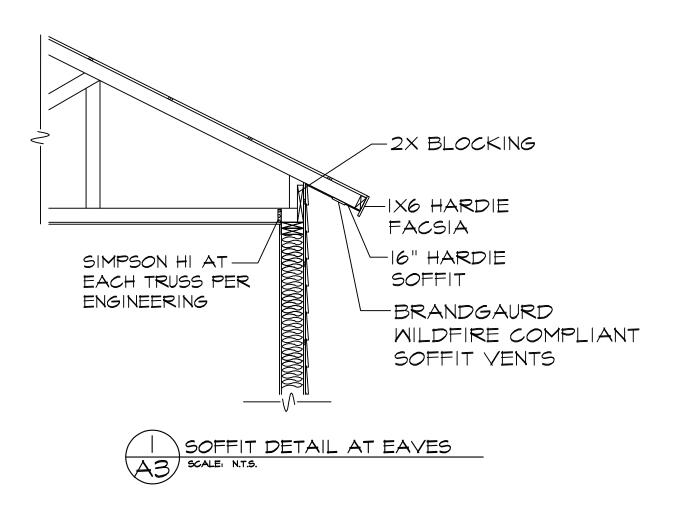
Project Name and Address

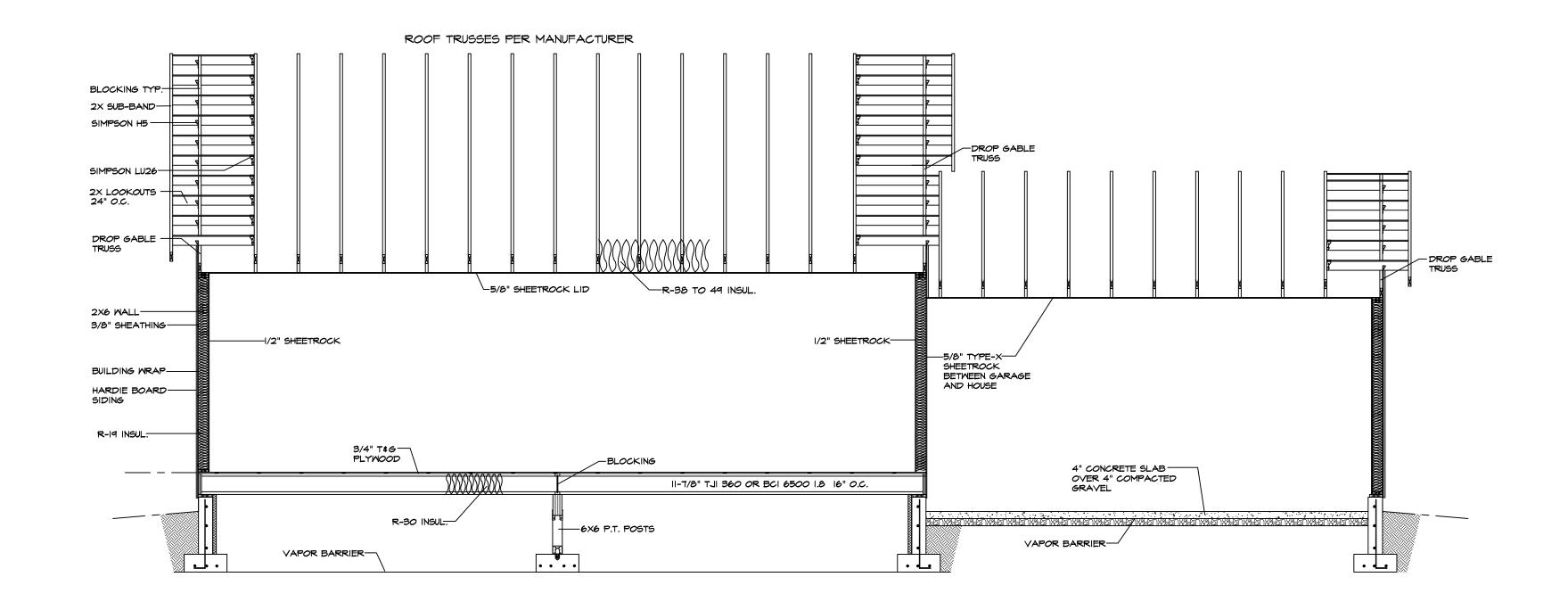
JOANN PINNOCK

GUEST HOUSE

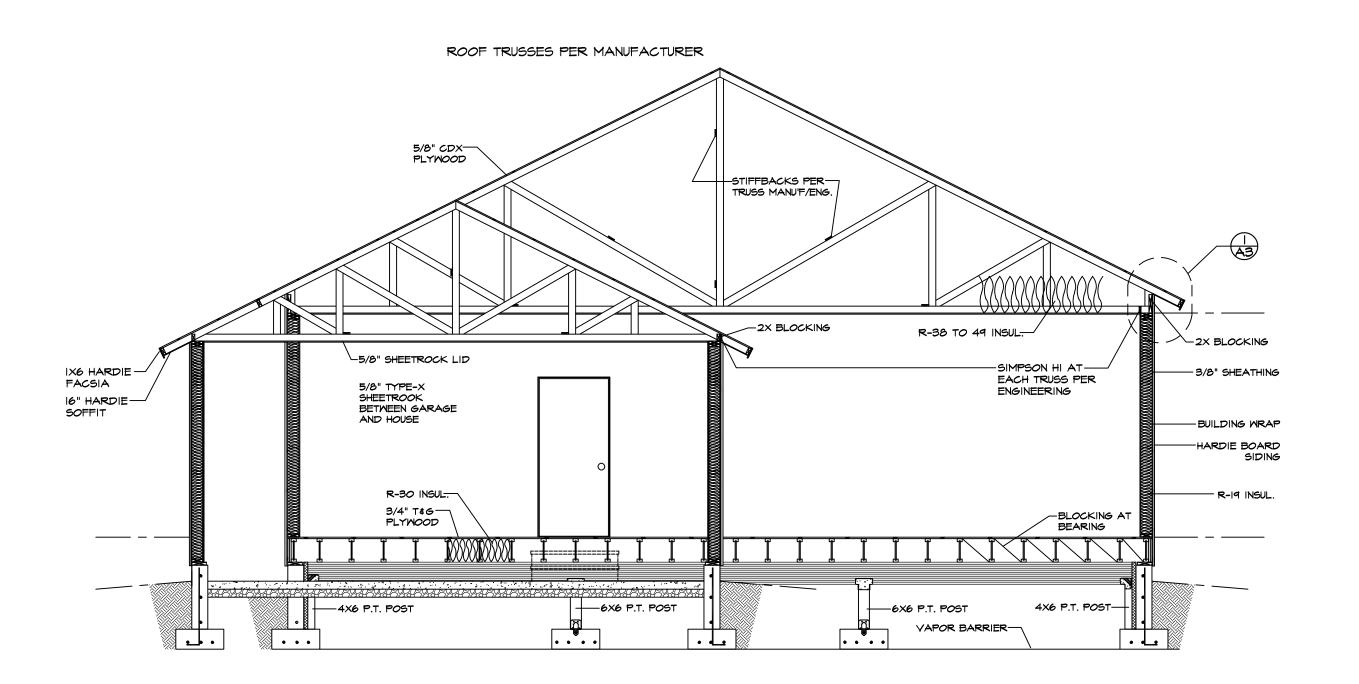
42 BELLEVUE RD. CARSON CITY NV. 89704 WASHOE COUNTY

**A**2





## #"=FOEROSS SECTION 1/4"=F00T



B - CROSS SECTION

1/4"=F00T

SHEET INDEX

AI- FLOOR PLAN A2- ELEVATIONS A3- CROSS SECTIONS

SI- FOUNDATION PLAN S2- FLOOR FRAMING 93- SHEARWALL PLAN 54- ROOF FRAMING PLAN

SDI- STRUCTURAL DETAILS SD2- STRUCTURAL DETAILS SD3- STRUCTURAL DETAILS

EI- ELECTRICAL PLAN

Drawn by: V.C. BUILDERS DESIGN SERVICE

Revision/Issue

Firm Name and Address VAUGHN & SONS CONSTRUCTION

RENO, NV. 775-324-1935

Project Name and Address JOANN PINNOCK GUEST HOUSE

42 BELLEVUE RD. CARSON CITY NV. 89704 MASHOE COUNTY

9-1-16

1/4"=F00T

## CONCRETE NOTES

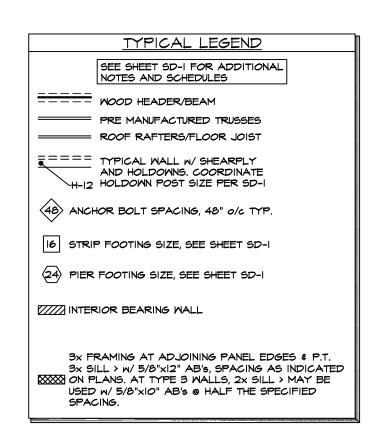
CONC. FOOTINGS PER PLAN W/ 2-#4 REBAR CONT., TYP. U.N.O. STEP FOOTINGS AS REQ'D. TO BEAR ON NATIVE GRADE OR AS DIRECTED BY SOILS ENGINEER. FOOTING SHALL BE A MIN. OF 2'-0" BELOW FINISHED GRADE.

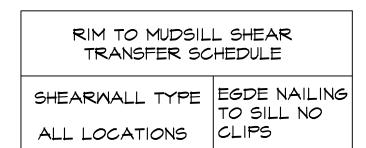
8" CONC. FOUNDATION WALL W/ 1-#4 TOP, MIDDLE, BOTTOM \$ #4 VERT. @ 24" O.C. TYP. U.N.O. PROVIDE  $\frac{5}{6}$ "  $\sim$  x IO" AB'S @ 48" O.C. TO 2x6 P.T. SILL U.N.O. IN SHEAR WALL PLAN. PROVIDE P.T. SOLE PLATE @ GARAGE.

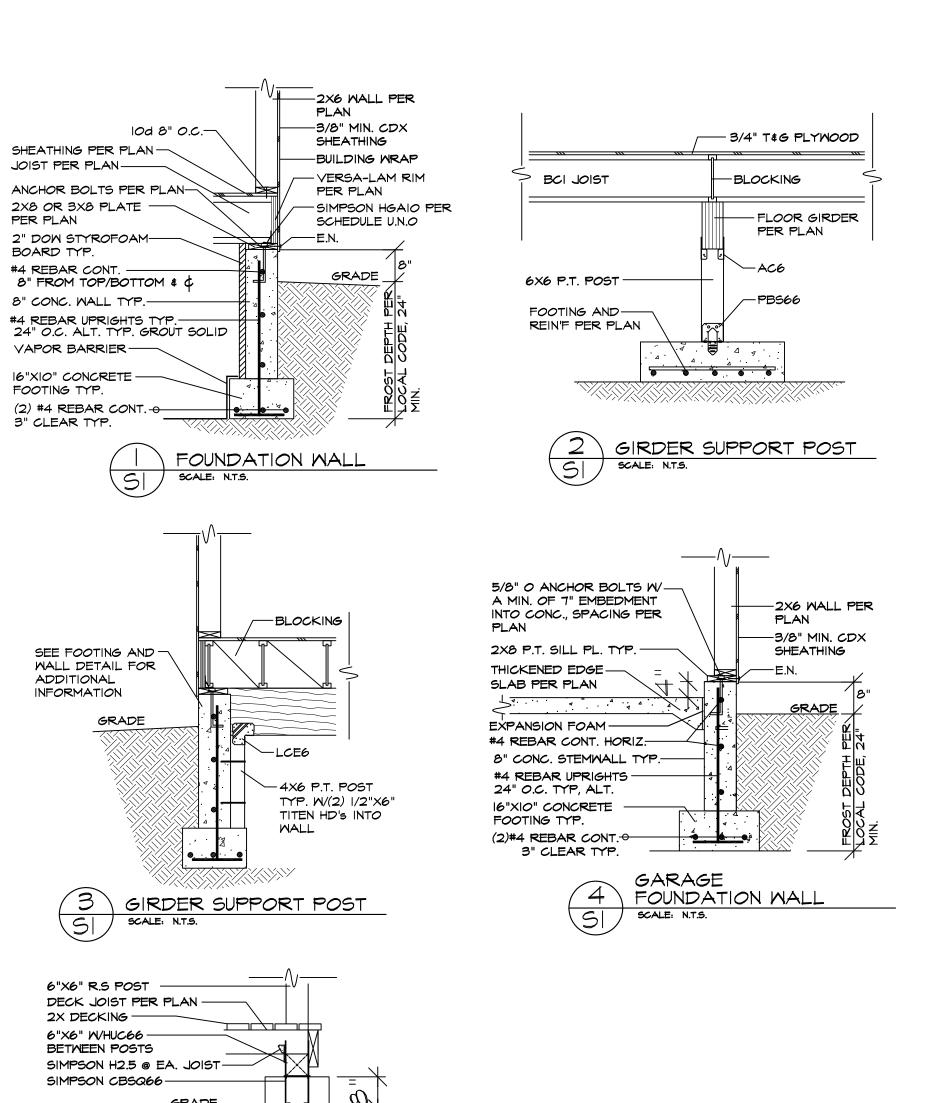
ALL SLABS TO BE 4" THICK CONCRETE W/ 6x6 IO/IO WELDED WIRE FABRIC REINFORCING OR CONCRETE SHALL HAVE FIBERMESH ADDITIVE @ PLANT. SLAB SHALL BE PLACED OVER 4" TYPE-II BASE COMPACTED TO 95% ON COMPACTED NATIVE SOIL, IF SUITABLE.

USE 5/8" A.B. FOR 3x OR DOUBLE SILL PLATE, EXTEND SILL BOLTS 7" INTO FOUNDATION MINIMUM; MAXIMUM SPACING SHALL BE PER DETAIL SCHEDULE WITH MINIMUM (2) BOLTS IN EACH SILL BOARD. BOLTS SHALL BE LOCATED NOT MORE THAN (12) NOR LESS THAN (7) BOLT DIAMETERS FROM EACH END OF SILL PIECE. MINIMUM 3"x3"x $\sqrt{4}$ " THICK PLATE WASHERS SHALL BE INSTALLED ON EACH SILL BOLT.

SILL PLATE: USE FOUNDATION GRADE REDWOOD OR TIMBERSTRAND LSL TREATED W/ ZINC BORATE OR PRESSURE TREATED DOUGLAS FIR MUDSILL. SEE SHEARWALL SCHEDULE FOR IMPORTANT INFORMATION REGARDING SILL PLATES. FOR ALL SILL PLATES NOTED, USE 2x WALL WIDTH WOOD SILL. ALL SHEAR WALLS, EXCEPT TYPE "6" & "4", REQUIRE FOUNDATION SILL PLATES & ALL FRAMING MEMBERS RECEIVING EDGE NAILING FROM ABUTTING PANELS TO BE NOT LESS THAN A SINGLE 3" NOMINAL MEMBER. PLYWOOD JOINT & SILL PLATE NAILING SHALL BE STAGGERED IN ALL CASES.







DOUBLE #3 TIES @ TOP -

16" SONOTUBE W/ (3)#4 REBAR UPRIGHTS AND

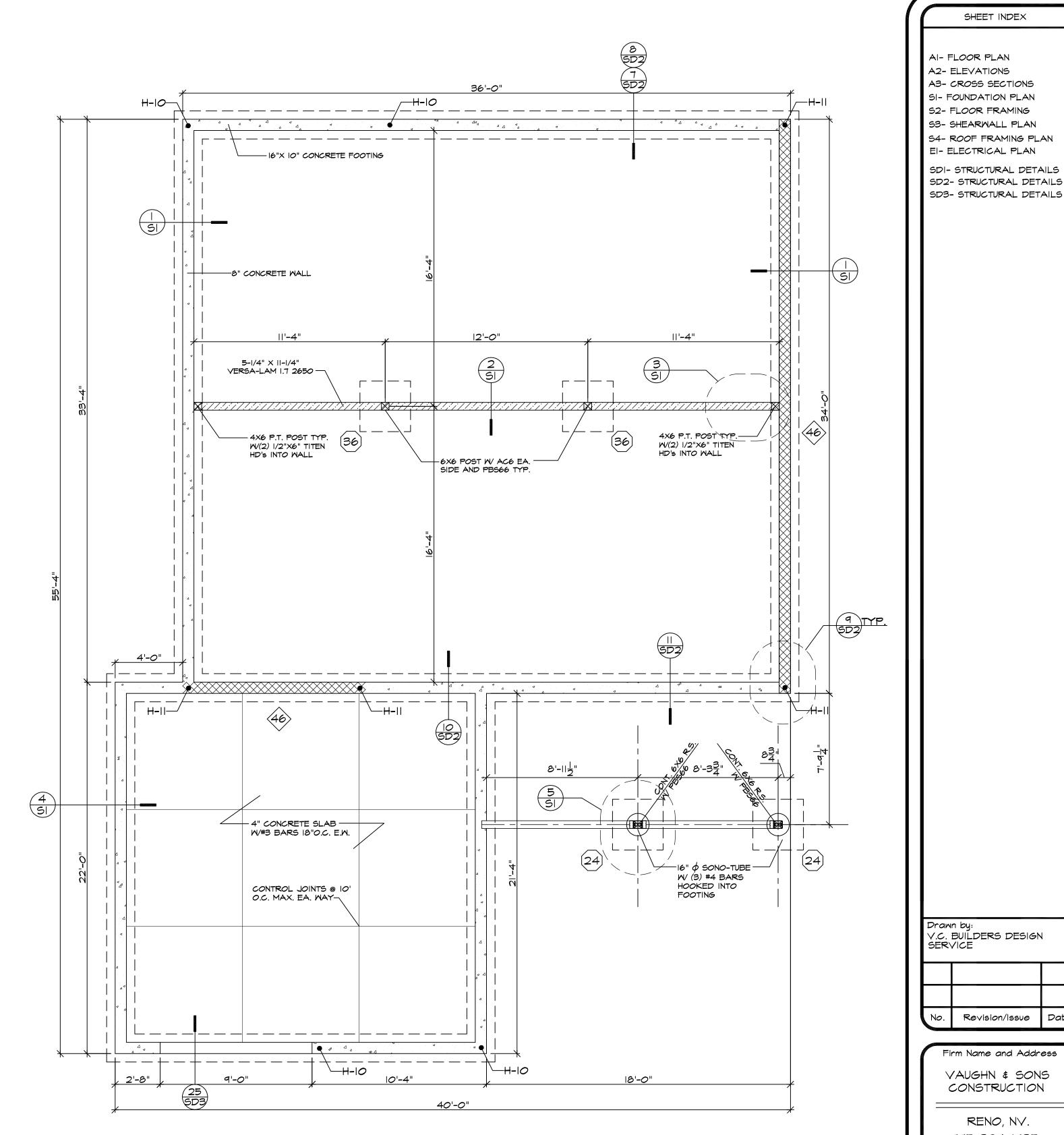
#3 BAR SEISMIC TIES

#4 REBAR EA. WAY -

CONCRETE PAD PER PLAN-

PORCH SUPPORTS SCALE: N.T.S.

12" O.C TYP.



FOUNDATION PLAN - HOUSE (SEE NOTES) 1/4"=F00T

JOANN PINNOCK GUEST HOUSE 42 BELLEVUE RD. CARSON CITY NV. 89704 MASHOE COUNTY

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9-1-16 1/4"=F00T

## FLOOR FRAMING NOTES

 $^{3}\!\!/_{4}$ " T&G PLYWOOD APA RATED STURD-1-FLOOR - 48/24 w/ IOd @ 6" O.C. BOUNDARY, EDGES, & DRAG STRUTS w/ IOd @ IO" O.C. FIELD - GLUE & NAIL THROUGHOUT, TYP.

II 360 OR BCI EQUIVALENT (PER PLAN) @ 16" O.C. TYP. BLK. SOLID @ALL SUPPORT LINES. PROVIDE RIM BOARD THROUGHOUT, TYP. (SEE PLAN) BLOCK SOLID UNDER ALL HOLDOWNS. PROVIDE CRUSH BLOCK, WEB STIFFENERS, ETC. PER MFR.

### TYPICAL FLOOR GIRDERS (SEE PLAN)

PROVIDE INSULATION @ RIM JOISTS & FLOOR.

INSULATE ALL PIPES & DUCTWORK.

PROVIDE SOLID BLKG. UP TO SUB-FLOOR, AS REQUIRED, TO SUPPORT POSTS ABOVE.

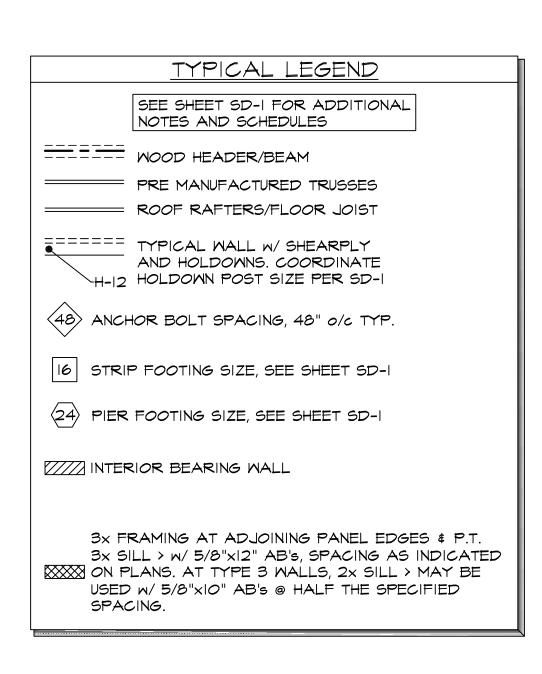
ALL FRAMING HARDWARE SHALL BE 'SIMPSON'. INSTALL PER MANUF, REQUIREMENTS.

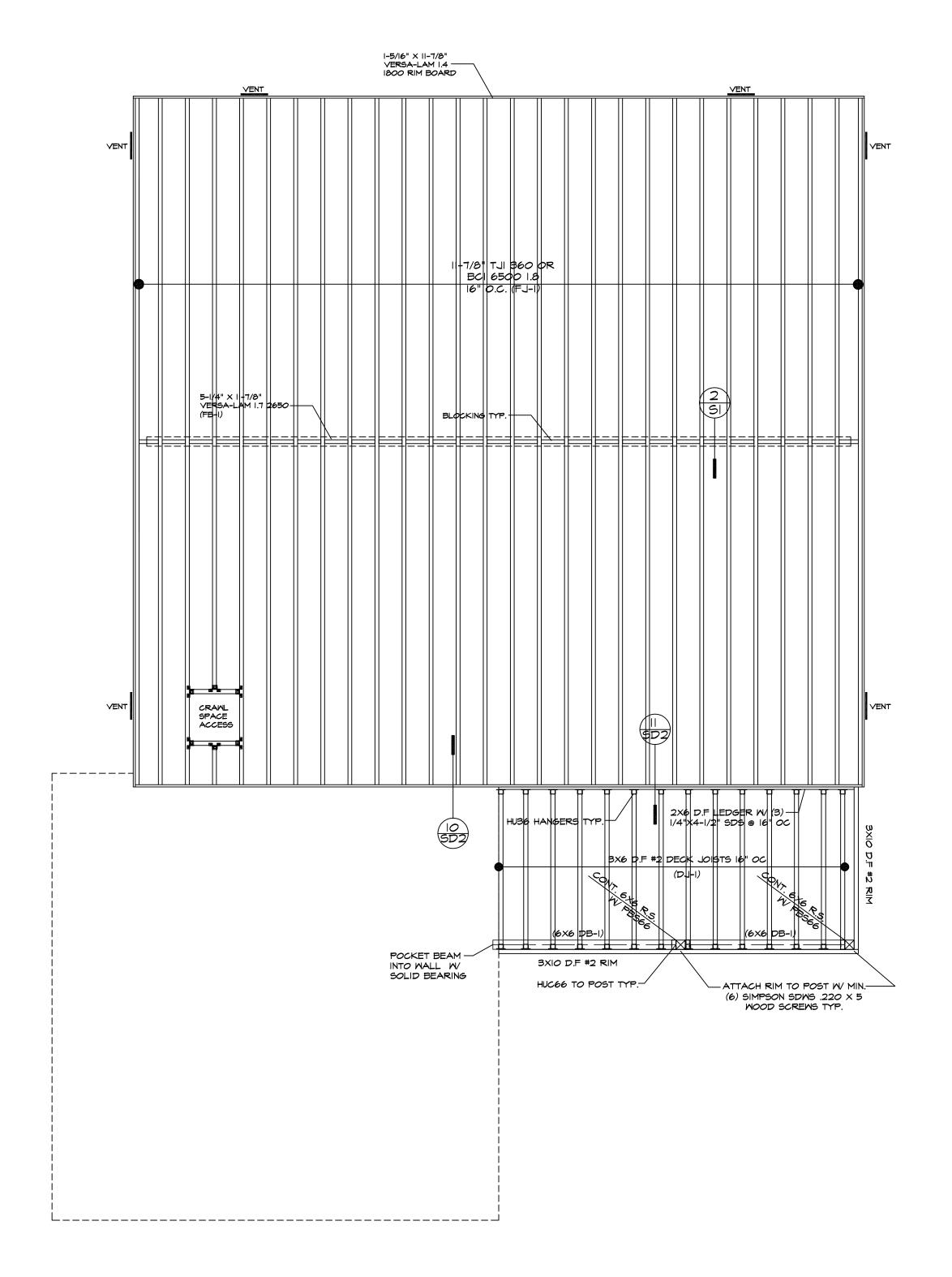
PROVIDE 6"  $\times$  14" VENTS @ RIM JOISTS AS REQ'D. (I PER EA. 25 L.F. OF EXTERIOR WALL AREA).

VERIFY THE ENTIRE CRAWLSPACE IS COVERED BY VAPOR BARRIER TYP.

MINIMUM CLEARANCE FROM GROUND UNDER GIRDERS SHALL BE 12 INCHES; UNDER JOISTS SHALL BE 18 INCHES.

ALL EXTERIOR WALLS SHALL BE CONSIDERED SHEARWALLS FASTENED W/LOG LAGS (SEE SHEARWALL SCHEDULE).





FLOOR AND DECK FRAMING (SEE NOTES)

1/4"=FOOT

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Drawn by: V.C. BUILDERS DESIGN SERVICE

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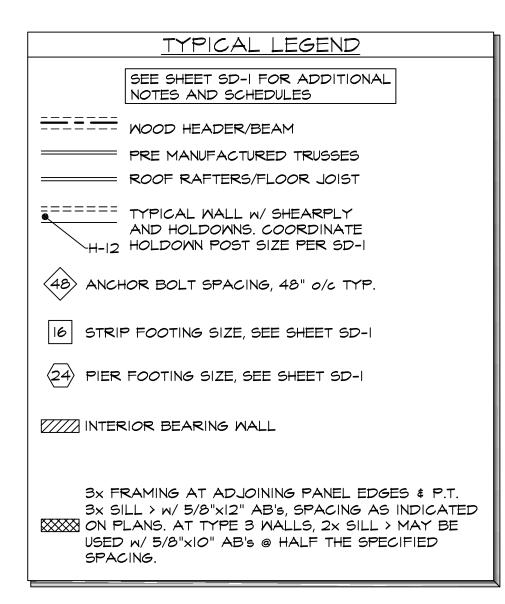
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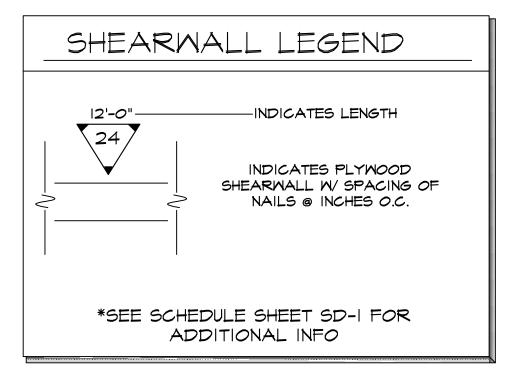
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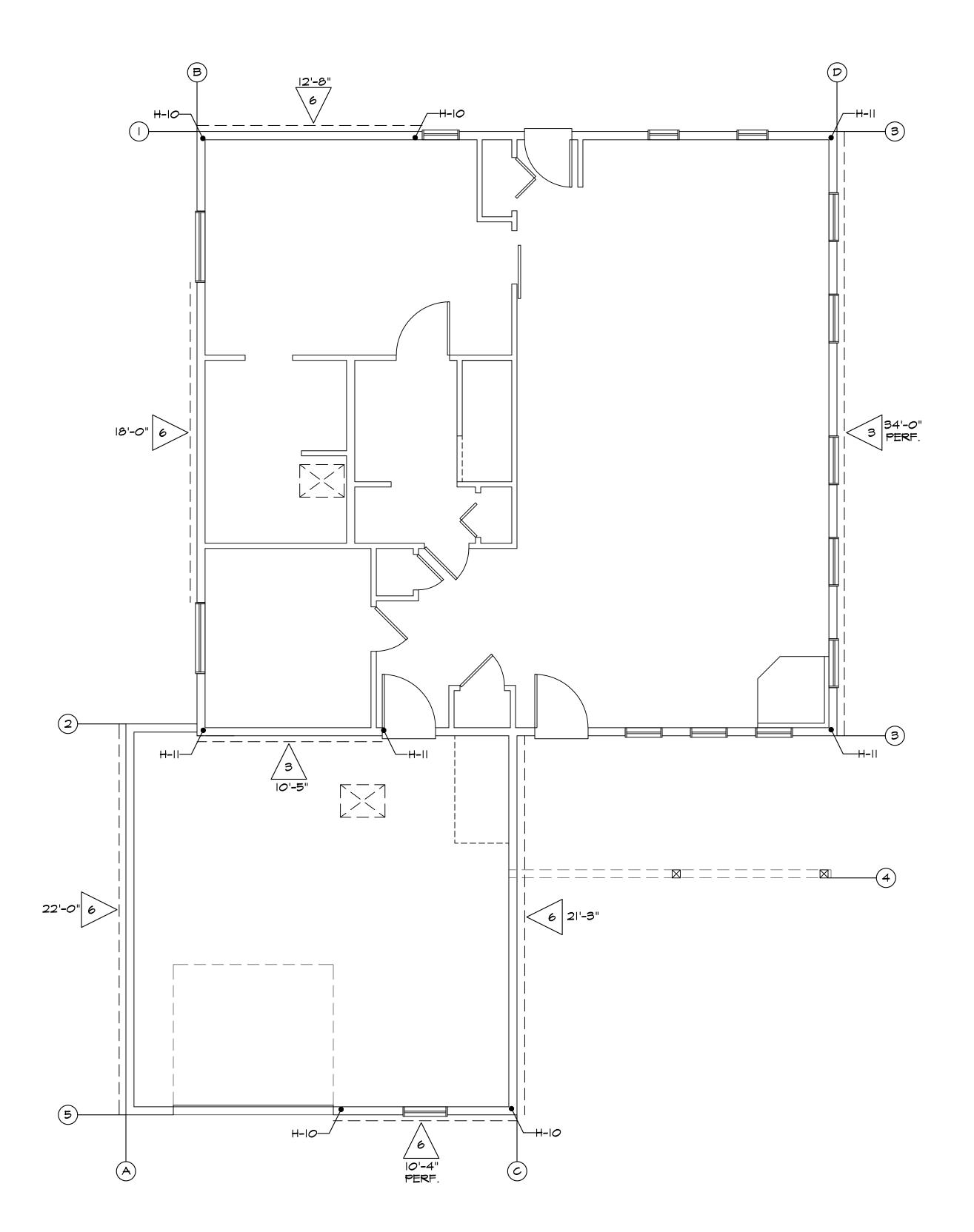
9-|-|6

1/4"=F00T

**S**2







SHEARMALL PLAN

1/4"=FOOT

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## ROOF FRAMING NOTES

ROOF LOADS: LIVE 28# SQ. FT. : DEAD 20# SQ. FT.

USE (1)-LAYER 3" (40/20) CDX APA RATED ROOF SHEATHING, MIN. 24" X TRUSS SPACING, APPLY FACE GRAIN/LONG DIMENSION PERPENDICULAR TO SUPPORT FRAMING. STAGGER PANELS \$ NAIL w/ IOd'S @ 6" O.C. EDGES & BOUNDARIES & IOd'S @ 12" O.C. FIELD. NAIL ALL DRAG MEMBERS, SHEAR PANELS, BLOCKING, E.T.C. w/ NAILS SPACED @ 4" O.C. SEE DETAILS FOR ADDITIONAL NAILING REQUIREMENTS.

ALL FRAMING HARDWARE NOTED SHALL BE "SIMPSON" INSTALL PER MANUFACTURES REQUIREMENTS.

PROVIDE MIN. DOUBLE STUDS BELOW ALL (1) \$ (2) PLY GIRDER TRUSSES W/ SIMPSON H6. USE (3) STUDS @ 3-PLY TRUSSES \$ (4) STUDS @ 4-PLY TRUSSES, U.N.O. CONNECT GIRDER TRUSS TO POST W/ SIMPSON H6 U.N.O.

ALL GLU-LAM BEAMS SHALL BE DOUG-FIR 24F-V4 U.N.O.

USE 12-16d BETWEEN TOP PLATE SPLICES. TYP U.N.O.

USE 2x4 STRONGBACK OUTRIGGERS @ 24" O.C. @ GABLE END

PROVIDE G.I. FLASHING @ ALL VALLEYS & ROOF-TO-WALL CONNECTIONS, TYP PROVIDE BITUTHANE MEMBRANE @ CRICKETS SLOPING LESS THAN 3:12 & AS NOTED ON ROOF PLAN

USE BOUNDARY NAILING @ ALL DRAG TRUSSES U.N.O.

A35 @ 32" O.C. TRUSS BLK'G TO TOP PLATE TYP U.N.O.

PROVIDE ICE & WATER DAM MEMBRANE @ HIPS, EAVES, VALLEYS & RIDGES AS PER LOCAL BLDG DEPT STANDARDS

PROVIDE SNOW DIVERTERS @ ALL ROOF PENETRATIONS PROVIDE ATTIC ACCESS (22"x30") PER I.R.C. SECTION R807.I

PROVIDE ROOF VENTILATION PER I.R.C. SECTION R806.1

PROVIDE BLOCKING @ ALL RIDGES, HIPS & VALLEYS TYP

PROVIDE CONT ROOF PLY UNDER ALL ROOF OVER FRAMING

## PRE-MANUFACTURED WOOD ROOF

## TRUSSES

TRUSS MANUFACTURER SHALL BE RESPONSIBLE FOR ALL ENGINEERING, LAYOUT DRAWINGS CONNECTIONS, BLOCKING, BRACING, & TRUSS ERECTION INFORMATION. THE CONTRACTOR SHALL BE RESPONSIBLE FOR PROPER COORDINATION BETWEEN ENGINEER/ARCHITECT DRAWINGS, TRUSS MANUFACTURER INFORMATION, ANY REQUIRED FIELD CHANGES, PROPER INSTALLATION OF FINAL PRODUCT & ITS CONFORMANCE TO THE ARCHITECT'S DESIGN. THE ARCHITECT & ENGINEER ASSUME NO LIABILITY FOR SAID PRODUCT.

TRUSS MANUFACTURER TO VERIFY LOCATION OF & PROVIDE REINFORCED TRUSSES FOR THE SUPPORT OF ANY MECHANICAL EQUIPMENT WHERE OCCURRING.

TRUSS MANUFACTURER TO VERIFY LOCATION OF \$ DESIGN FOR ALL CEILING HEIGHT CHANGES, ATTIC ACCESSES, RETURN AIR GRILLS, ETC. TRUSS MANUFACTURER TO COORDINATE ANY FINDINGS TO BOTH K2 ENGINEERING & THE ARCHITECT.

DEAD LOAD DEFLECTIONS SHALL BE LIMITED TO L/240.

GABLE END TRUSSES SHALL BE STRUCTURAL, DESIGNED TO SUPPORT OVERHANG & TO ALLOW A TOP CHORD NOTCH OF ONE & A HALF INCHES

ALL NON-BEARING WALLS ARE TO HAVE A ONE-FOURTH OF AN INCH GAP TO THE BOTTOM CHORD OF THE TRUSSES. SECURE BOTTOM CHORD TO WALL BELOW W/ SIMPSON STC CLIPS.

USE PRE-ENGINEERED MANUFACTURED TRUSSES. SOLID BLOCK @ ALL SUPPORTS & PER MANUFACTURER'S SPECIFICATIONS. USE SIMPSON HI @ EACH SUPPORT WALL/BEAM TO EACH TRUSS \$ H6 @ EACH SUPPORT WALL/BEAM TO EACH GIRDER TRUSS.

HANG TRUSSES & GIRDER TRUSSES W/ SIMPSON HUS26 OR AS SPECIFIED ON PLAN. TRUSS CALCULATIONS HOLD PRECEDENCE OVER PLAN @ ALL TRUSS-TO-TRUSS CONNECTIONS.

TRUSSES ARE TO BE HANDLED, INSTALLED, & BRACES IN ACCORDANCE W/ HIB-41 OF THE TRUSS PLATE INSTITUTE (TPI).

NO TRUSSES SHALL BE DELIVERED TO THIS RESIDENCE THAT ARE NOT FROM RENO TRUSS, UNLESS PRIOR WRITTEN APPROVAL IS OBTAINED.

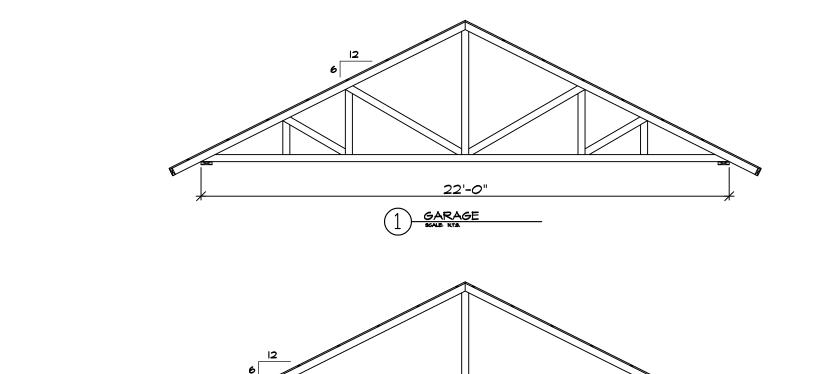
## ATTIC VENTILATION

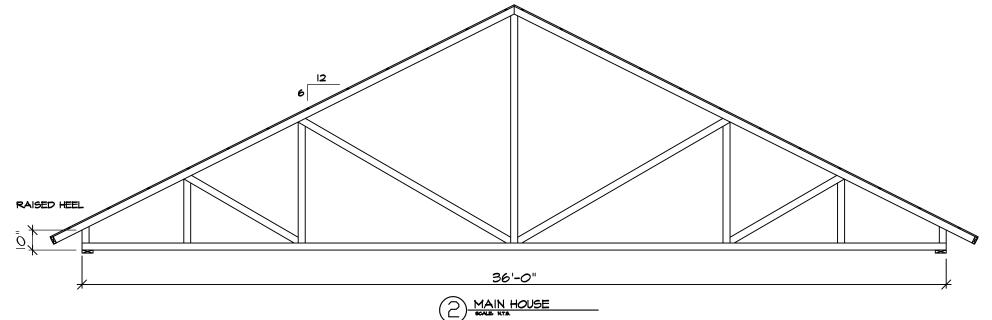
TOTAL VENTILATED ATTIC SQ. FOOTAGE = 1700 SQ. FT. VENTILATION REQUIRED = 5.67 SQ.FT - 1632 SQ. IN.

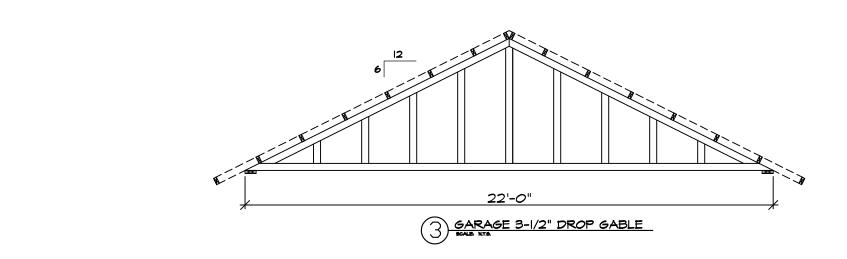
PROVIDED BY:

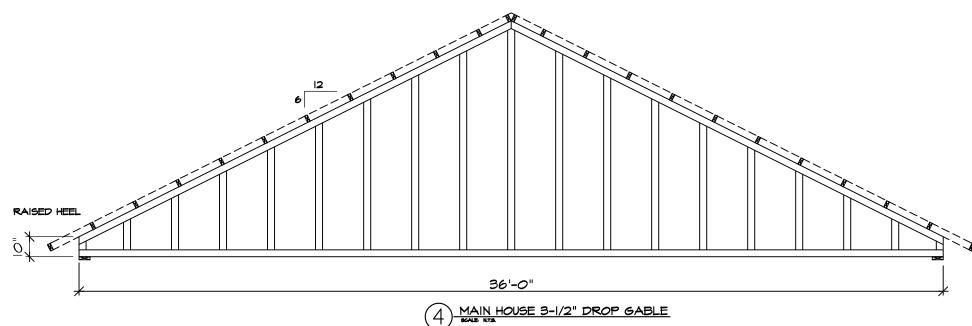
(48) - LINEAL FEET OF RIDGE VENTS = 770 SQ IN. (2) - 12" X 12" GABLE VENTS = 76 SQ. IN. (7) - 120"X3.5" CONT. SOFFIT VENTS = 840 SQ. IN.

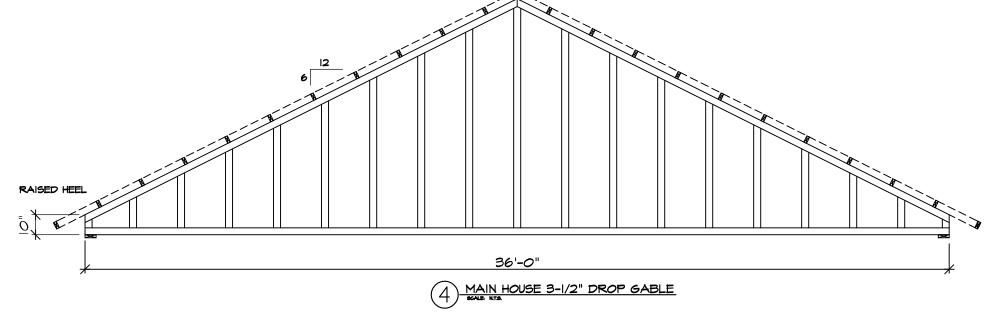
MINIMUM ATTIC VENTILATION AREA IS 1/150 OF THE AREA OF THE SPACE VENTILATED FOR ALL ENCLOSED ATTICS OR RAFTER SPACES. THIS MINIMUM CAN BE DECREASED TO 1/300 PROVIDED AT LEAST 50% AND NOT MORE THAN 80% OF THE REQUIRED VENTILATION IS IN THE UPPER PART OF THE ROOF AT LEAST 3 FEET ABOVE EAVE OR CORNICE YENTS WITH THE BALANCE OF THE REQUIRED VENTILATION PROVIDED BY EAVE OR CORNICE VENTS, OR A CLASS I OR 2 VAPOR BARRIER IS INSTALLED ON THE WARM WINTER SIDE OF THE CEILING.











TYPICAL LEGEND

SEE SHEET SD-I FOR ADDITIONAL

AND HOLDOWNS. COORDINATE

3x FRAMING AT ADJOINING PANEL EDGES & P.T.

ON PLANS. AT TYPE 3 WALLS, 2x SILL > MAY BE

USED w/ 5/8"x10" AB's @ HALF THE SPECIFIED

3x SILL > w/ 5/8"x12" AB's, SPACING AS INDICATED

H-17 HOLDOWN POST SIZE PER SD-1

NOTES AND SCHEDULES

PRE MANUFACTURED TRUSSES

ROOF RAFTERS/FLOOR JOIST

E===== TYPICAL WALL W/ SHEARPLY

48 ANCHOR BOLT SPACING, 48" 0/c TYP.

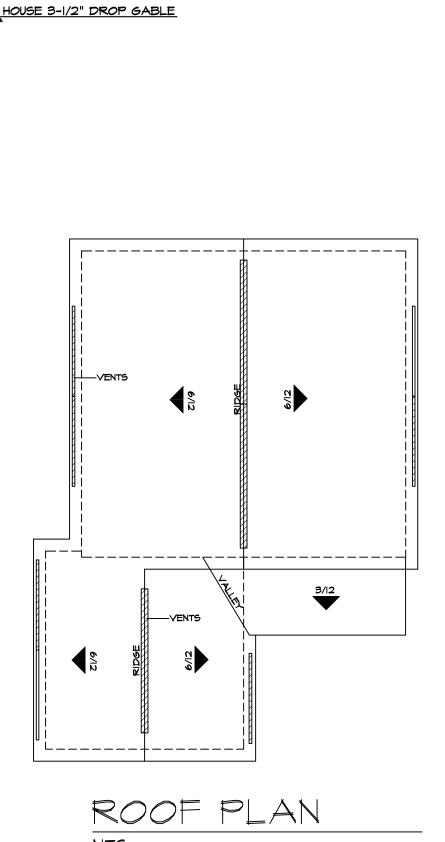
16 STRIP FOOTING SIZE, SEE SHEET SD-1

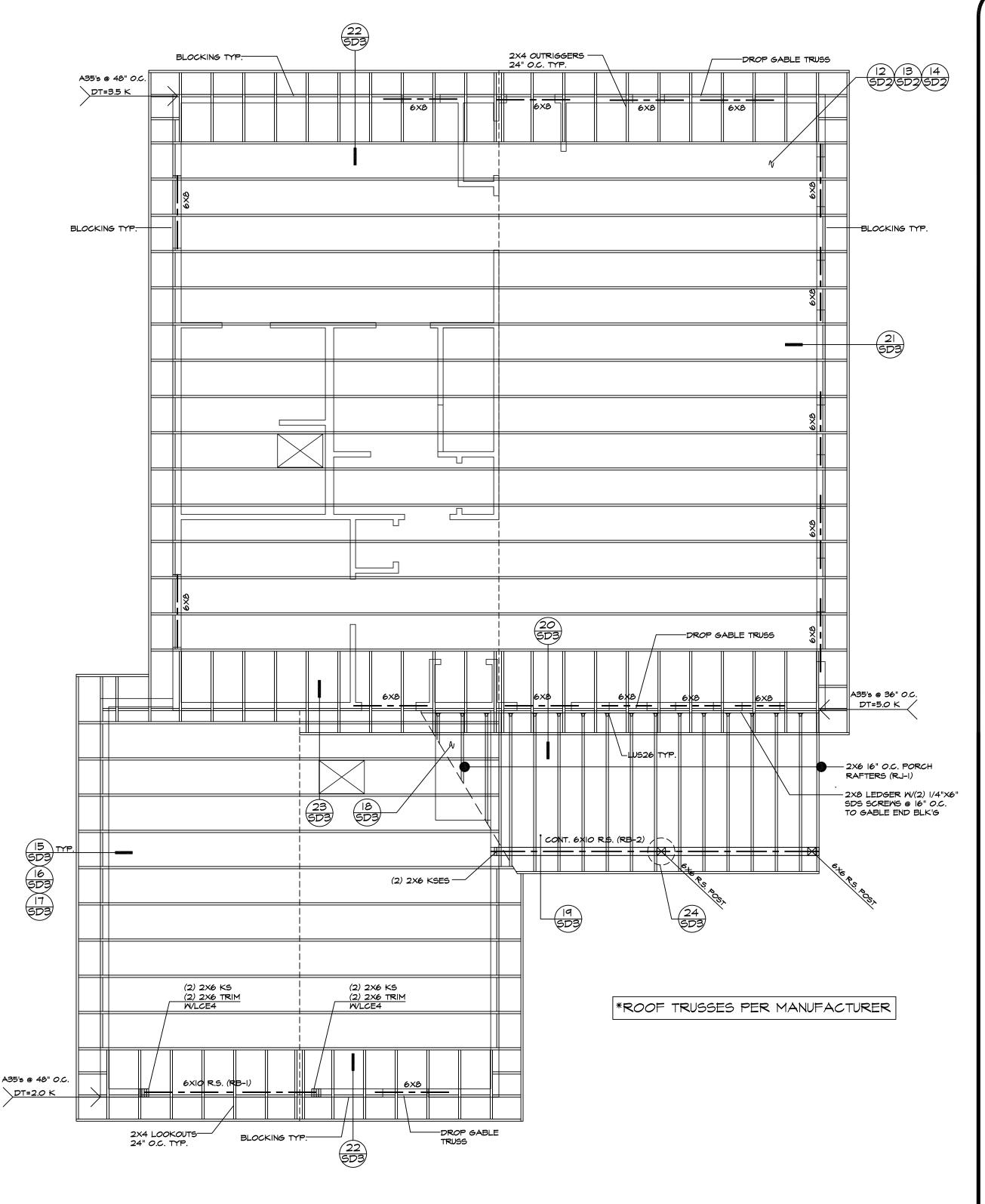
(24) PIER FOOTING SIZE, SEE SHEET SD-I

INTERIOR BEARING WALL

SPACING.

MOOD HEADER/BEAM





## ROOF FRAMING HOUSE (SEE NOTES)

1/4"=F00T

THIS PLAN IS FOR REFERENCE, FINAL LAYOUT, PLAN AND CONNECTIONS BY TRUSS MANUFACTURER AND ENGINEERING

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V.C. BUÍLDERS DESIGN

Firm Name and Address VAUGHN & SONS CONSTRUCTION

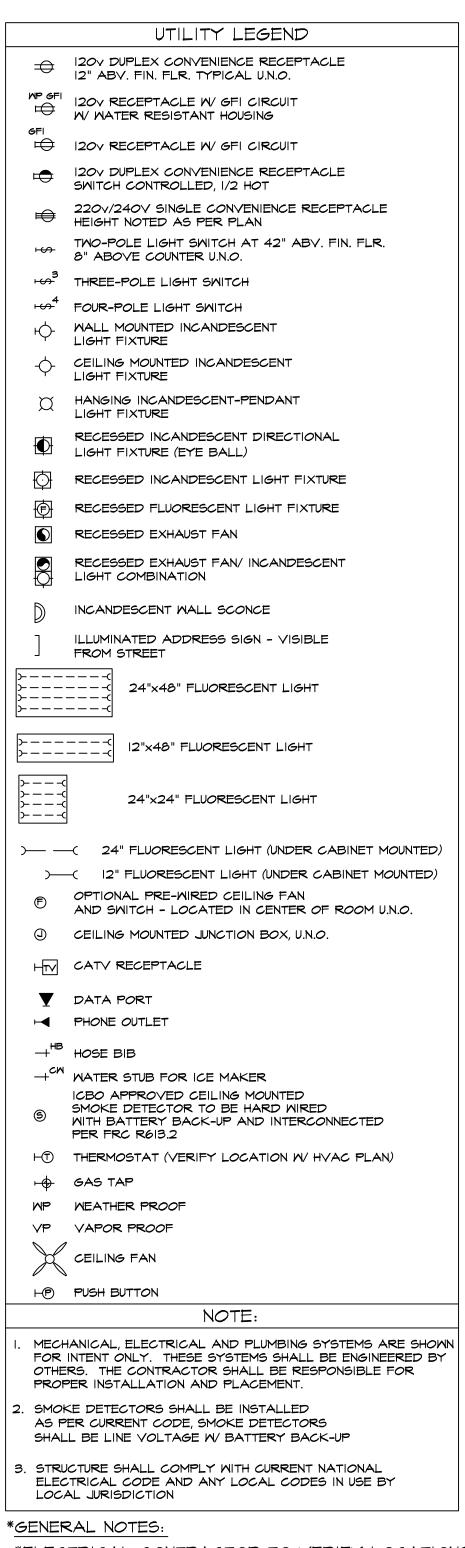
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9-1-16 1/4"=F00T



\*ELECTRICAL CONTRACTOR TO VERIFY LOCATIONS, HEIGHT OF TV'S, PHONE / COMPUTER AND FINAL ELECTRICAL LAYOUT WITH OWNER

\*ALL ELECTRICAL FIXTURES TO BE SELECTED BY OWNER

\*BUILDING TO BE WIRED TO CURRENT NATIONAL ELECTRIAL CODE AND ANY LOCAL CODES.

AND SHITCH IN ATTIC >-----)-----

## ELECTRICAL PLAN

1/4"=F00T

## CARBON MONOXIDE PROTECTION

THE CARBON MONOXIDE POISONING PREVENTION ACT MANDATES THAT DETECTORS MUST BE INSTALLED IF THE RESIDENTIAL UNIT HAS ANY OF THE FOLLOWING:

GAS APPLIANCES SUCH AS GAS STOVE, FIREPLACE, GAS MATER HEATER, ETC.

FIREPLACE. AN ATTACHED GARAGE.

CARBON MONOXIDE DETECTORS REQUIRED BY THE LAW IN THE STATE OF CALIFORNIA SHOULD BE INSTALLED PROPERLY. AS A GENERAL PRACTICE SMOKE DETECTORS SHALL BE INSTALLED:

I) ON A WALL ABOUT FIVE FEET ABOVE FROM FLOOR LEVEL.
IT IS RECOMMENDED INSTALLING THE DETECTOR AT LEAST 6
INCHES FROM ALL EXTERIOR WALLS AND AT LEAST 3 FEET
FROM HVAC VENTS.

(2) INSTALLED OUTSIDE OF EACH SEPARATE SLEEPING AREA IN THE IMMEDIATE VICINITY OF THE BEDROOM(S) IN DWELLING UNITS AND ON EVERY LEVEL INCLUDING BASEMENTS

NEW CONSTRUCTION OR REMODELS SHALL BE HARD-WIRED WITH BATTERY BACK-UP, INTERCONNECTING ALL DETECTORS SO THAT WHEN ONE ALARM SOUNDS, THEY ALL DO. S4- ROOF FRAMING PLAN
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Firm Name and Address

VAUGHN & SONS

CONSTRUCTION

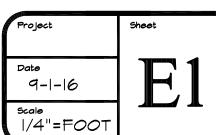
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- ALL WORK, DETAILS OF DESIGN, WORKMANSHIP, AND MATERIALS SHALL CONFORM TO REQUIREMENTS OF THE 2012 EDITION OF THE INTERNATIONAL BUILDING CODE (IBC) OF THE INTERNATIONAL CODE COUNCIL AND THE APPLICABLE
- COUNTY/CITY BUILDING CODES. 2. K2 ENGINEERING EXPRESSLY RESERVES ITS COMMON LAW COPYRIGHT AND OTHER PROPERTY RIGHTS IN THESE PLANS. THESE PLANS ARE NOT TO BE REPRODUCED, CHANGED.

OR COPIED IN ANY MATTER WHATSOEVER, NOR ARE THEY TO BE ASSIGNED TO A THIRD PARTY WITHOUT THE EXPRESS WRITTEN CONSENT OF K2 ENGINEERING. IN THE EVENT OF UNAUTHORIZED REUSE OF THESE PLANS BY A THIRD PARTY, THE THIRD PARTY SHALL HOLD K2 ENGINEERING HARMLESS.

- . K2 ENGINEERING RESERVES THE RIGHT TO PERFORM OBSERVATION VISITS TO THE SITE AT ANY TIME. OBSERVATIONS ARE PERFORMED SOLELY FOR THE PURPOSE OF DETERMINING IF THE CONTRACTOR UNDERSTANDS DESIGN INTENT CONVEYED IN THE PLANS. OBSERVATIONS DO NOT GUARANTEE CONTACTOR'S PERFORMANCE AND ARE NOT TO BE CONSTRUED AS SUPERVISION OF THE PROJECT.
- . IN THE EVENT THAT CERTAIN EXISTING DIMENSIONS AND/OR CONDITIONS ARE FOUND TO BE DIFFERENT FROM THOSE SHOWN ON THE PLANS AND DETAILS, THE ENGINEER SHALL BE IMMEDIATELY NOTIFIED SO THAT THE PROPER REVISIONS CAN BE MADE IF NECESSARY. THE CONTRACTOR SHALL BE HELD RESPONSIBLE FOR THE RESULTS OF ANY ERRORS, DISCREPANCIES, OR OMISSIONS WHICH THE CONTRACTOR FAILED TO NOTIFY K2
- ENGINEERING OF BEFORE CONSTRUCTION AND/OR FABRICATION OF THE WORK ). K2 ENGINEERING 15 RESPONSIBLE FOR THE STRUCTURAL ITEMS IN THE PLANS ONLY. SHOULD ANY CHANGES BE MADE, OR SHOULD THE RESULTS OF THESE CALCULATIONS NOT BE FULLY OR PROPERLY TRANSFERRED TO THE PLANS, K2 ENGINEERING ASSUMES NO RESPONSIBILITY FOR THE STRUCTURE.
- 5. THE DETAILS SHOWN ON THE DRAWINGS ARE TYPICAL. SIMILAR DETAILS APPLY TO SIMILAR CONDITIONS. NO DEVIATIONS FROM STRUCTURAL DETAILS SHALL BE MADE WITHOUT THE PRIOR WRITTEN APPROVAL OF K2 ENGINEERING.
- . THE CALCULATIONS ARE BASED UPON A COMPLETE STRUCTURE. TEMPORARY SUPPORTS, ETC., ARE THE SOLE RESPONSIBILITY OF THE CONTRACTOR AND HAVE NOT BEEN CONSIDERED BY K2 ENGINEERING. SHOULD AN UNFINISHED STRUCTURE BE SUBJECT TO LOADS, K2 ENGINEERING SHOULD BE CONSULTED FOR AN INTERIM DESIGN OR IF NOT, WILL ASSUME NO LIABILITY.
- 3. ALL NOTES ARE TYPICAL UNLESS NOTED OTHERWISE ON THE PLANS. ALL HARDWARE AND FRAMING MEMBERS SPECIFIED IN THE CALCULATIONS AND/OR PLANS ARE MINIMUMS AND LARGER MEMBERS OF EQUAL OR BETTER GRADE MAY BE SUBSTITUTED.

- . K2 ENGINEERING HAS NOT MADE A GEOTECHNICAL REVIEW OF THE BUILDING SITE AND IS NOT RESPONSIBLE FOR GENERAL SITE STABILITY OR SOIL SUITABILITY FOR THE PROPOSED PROJECT. K2 ENGINEERING RECOMMENDS A REVIEW OF THE SITE BY A GEOLOGICAL ENGINEER OR A QUALIFIED CIVIL ENGINEER TO DETERMINE GENERAL SITE STABILITY AND SOIL SUITABILITY FOR THE PROJECT.
- . BUILDING SITES ARE ASSUMED TO BE DRAINED AND FREE OF CLAY OR EXPANSIVE SOIL. ALL FOOTINGS SHALL BE LEVEL OR STEPPED AND BEAR ON FIRM, STABLE, NATURAL, UNDISTURBED SOIL OR AN APPROVED COMPACTED FILL
- 4. PERIMETER OR EXTERIOR FOOTING DEPTHS MUST EXTEND BELOW FROSTLINE (18" OR 24" AS PER LOCAL CODE REQUIREMENTS). ALL OTHER FOOTINGS (INTERIOR) SHALL BOTTOM 12" MINIMUM BELOW NATURAL UNDISTURBED GRADE.
- 5. BUILDING PADS SHALL BE GRADED 2% TOWARD APPROVED DRAINAGE FACILITIES AND PROVISIONS SHALL BE MADE TO CONTROL AND DRAIN SURFACE WATER AROUND BUILDING. 6. ASSUME CLASS D SOILS WITH ALLOWABLE SOIL BEARING PRESSURE OF 2000 PSF WITH A CONSTANT EXPANSION INDEX LESS THAN 20. SOIL BEARING PRESSURE HAS BEEN DETERMINED IN ACCORDANCE WITH IBC TABLE 1806.2.

#### FILL AND BACKFILL

OF THE OWNER AND/OR CONTRACTOR.

## 1. FILL MATERIAL SHALL BE FREE FROM DEBRIS, VEGETATION, AND OTHER FOREIGN

- 2. BACKFILL TRENCHES SHALL BE COMPACTED TO 90% DENSITY PER ASTM 1557 TO WITHIN 12" OF FINISHED GRADE. THE TOP 12' SHALL BE LANDSCAPE FILL.
- 3. BACKFILL AT PIPE TRENCHES SHALL BE COMPACTED ON BOTH SIDES OF PIPE IN 6' LIFTS. 4. WATERPROOF EXTERIOR FACES OF ALL FOUNDATION WALLS ADJACENT TO USABLE SPACES. WATERPROOFING OF ALL FOUNDATION AND RETAINING WALLS TO BE THE RESPONSIBILITY
- 5. ALL BACKFILL AGAINST FOUNDATION WALLS MUST BE COMPACTED TO 90% RELATIVE
- 5. PROVIDE A 4" DIAMETER PYC PERFORATED DRAINPIPE AT GRADE SIDE OF ALL RETAINING WALLS. SLOPE PIPE TO DRAIN TO DAYLIGHT AND DRYWELL.

- . REINFORCED CONCRETE WORK SHALL CONFORM TO APPLICABLE REQUIREMENTS OF THE IBC AND ACI STANDARD 318-11.
- . AGGREGATE SHALL CONFORM TO ASTM C33 FOR STONE CONCRETE. 3. CONCRETE STOOPS TO BE MACHINED MIXED AND PLACED IN ACCORDANCE WITH THE IBC.
- 4. COMPRESSION STRENGTH OF ALL REINFORCED CONCRETE SHALL NOT BE LESS THAN 3000 PSI AT 28 DAYS.
- 5. STRUCTURAL DESIGN BASED ON FIG = 2500 PSI (SPECIAL INSPECTION NOT REQUIRED). 6. USE NORMAL WEIGHT CONCRETE (145 PCF) FOR ALL CONCRETE. USE TYPE II CEMENT TYPICAL. IF SOIL CONTAINS SULFATE CONCENTRATIONS OF 2% OR MORE,
- USE TYPE Y CEMENT. 7. THE MAXIMUM SLUMP SHALL NOT EXCEED 3". PLASTICIZERS MAY BE USED TO INCREASE SLUMP TO 8' MAXIMUM PROVIDED THEY DO NOT INCREASE SHRINKAGE.
- 8. MAXIMUM WATER/CEMENT RATIO SHALL BE .55 FOR 3000 PSI CONCRETE. 9. EXTERIOR SLABS ON GRADE SHALL CONTAIN NOT LESS THAN 5% NOR MORE THAN 6% ENTRAINED AIR
- Ø. FOLLOW RECOMMENDED PRACTICES FOR HOT AND COLD WEATHER CONCRETING BY OBSERVING ACI 305 AND ACI 306 GUIDELINES. I. PROVIDE STANDARD CRACK CONTROL JOINTS IN ALL SLABS ON GRADE USING MAXIMUM DIMENSION OF 10 FEET FOR 4" SLABS AND 12 FEET FOR 6" SLABS. JOINT DEPTH SHALL
- NOT EXCEED ONE-FOURTH OF SLAB DEPTH. 2. TOP OF CONCRETE SLABS SHALL BE MINIMUM 6" ABOVE FINISHED GRADE. 3. PIPES MAY PASS THROUGH STRUCTURAL CONCRETE IN SLEEVES, BUT SHALL NOT BE EMBEDDED THEREIN. PIPES OR DUCTS EXCEEDING ONE-THIRD THE SLAB OR WALL
- THICKNESS SHALL NOT BE PLACED IN STRUCTURAL CONCRETE 14. DO NOT PLACE CONCRETE UNTIL ALL REINFORCEMENT, CONDUIT, OUTLET BOXES, ANCHORS HANGERS, SLEEVES, BOLTS, HOLDOWNS, ANCHOR BOLTS OR OTHER EMBEDDED MATERIALS AND ITEMS ARE SECURELY AND PROPERLY FASTENED IN THEIR PROPER PLACES AND POSITIONS.

- . ALL CONCRETE MASONRY UNITS SHALL CONFORM TO ASTM C90, GRADE N. F'M = 1500 PSI. USE 85 PCF MINIMUM WEIGHT UNITS ABOVE GRADE AND 110 PCF MINIMUM WEIGHT UNITS BELOW GRADE. USE MOISTURE CONTROLLED UNITS ONLY. USE OPEN-END MASONRY UNITS AS MUCH AS POSSIBLE AND AT WALL INTERSECTIONS.
- 2. ALL BRICK SHALL CONFORM TO ASTM C62, GRADE MW. 3. MORTAR FOR CONCRETE MAGONRY SHALL CONFORM TO AGTM C279, TYPE S. 4. GROUT FOR CONCRETE MASONRY SHALL BE IN ACCORDANCE WITH IBC SECTION 2103.
- MINIMUM 28-DAY COMPRESSIVE STRENGTH SHALL NOT BE LESS THAN 2000 PSI. 5. ALL WALLS SHALL BE GROUTED SOLID. GROUT SHALL BE YIBRATED INTO PLACE AND SHALL BE PLACED IN LIFTS NOT EXCEEDING 4' UNLESS APPROPRIATE CLEANOUT HOLES ARE PROVIDED IN ACCORDANCE WITH IBC.
- 5. AGGREGATES FOR MORTAR AND GROUT SHALL BE NATURAL SAND AND ROCK CONFORMING TO ASTM C-144 (MORTAR) AND C-404 (GROUT). T. CEMENT SHALL BE PORTLAND CEMENT CONFORMING TO ASTM C-150, TYPE I OR II,
- 8. ALL CONCRETE BLOCK AND BRICK SHALL BE LAID IN RUNNING BOND. 9. WHEN ABSOLUTELY NECESSARY FOR CONSTRUCTION PURPOSES TO STOP OFF LONGITUDINAL RUNS OF MASONRY, STOP OFF ONLY BY RACKING BACK ONE-HALF UNIT LENGTH IN EACH COURSE. TOOTHING SHALL NOT BE PERMITTED.
- Ø. MASONRY WALLS SHALL BE REINFORCED WITH \*4'S VERT @ 16" O.C. EACH WAY \$ #4 @ 24" O.C. HORIZ. BAR SPLICES SHALL BE STAGGERED.

## REINFORCING STEEL

- I. REINFORCING BARS SHALL BE DEFORMED BARS CONFORMING TO THE REQUIREMENTS OF ASTM A615 GRADE 60 FOR ALL \*5 AND LARGER BARS AND GRADE 40 FOR ALL \*4
- 2. ALL DETAILS OF FABRICATION AND INSTALLATION OF REINFORCING STEEL SHALL BE IN ACCORDANCE WITH THE ACI MANUAL OF STANDARD PRACTICE.
- 3. WELDED FABRIC (MESH) SHALL CONFORM TO LATEST REVISED ASTM A185 AND BE FURNISHED IN FLAT SHEETS. SMOOTH WIRE FABRIC SHALL CONFORM TO ASTM A-85 HAYING A YIELD STRENGTH OF 60 KSI.
- 4. WELDING OF REINFORCING STEEL SHALL CONFORM TO AWS D12-1 USING LOW HYDROGEN ELECTRODES.
- 5. ALL BARS SHALL BE LAPPED WITH A MINIMUM OF 40 BAR DIAMETERS (2' MINIMUM) AT ALL 6. SPLICES OF HORIZONTAL REBAR IN WALLS AND FOOTINGS SHALL BE STAGGERED 4' MINIMUM.
- 1. DOWELS FOR WALLS AND COLUMNS SHALL BE THE SAME SIZE AND SPACING AS THE WALL/COLUMN REINFORCING. 8. ALL REINFORCING STEEL SHALL BE ACCURATELY LOCATED AND ADEQUATELY SECURED IN
- POSITION BEFORE AND DURING PLACEMENT OF CONCRETE 9. MASONRY REINFORCEMENT, BOLTS, ETC. SHALL HAVE MINIMUM GROUT COVERAGE OF THREE-FOURTHS OF AN INCH.
- IØ. REINFORCEMENT COVER IN CAST-IN-PLACE CONCRETE SHALL BE AS FOLLOWS:
- A. 3" CONCRETE CAST AGAINST AND PERMANENTLY EXPOSED TO EARTH B. 2" - FORMED SURFACES EXPOSED TO GROUND OR WEATHER

## STRUCTURAL STEEL

- 1. STRUCTURAL STEEL AND MISCELLANEOUS IRON SHALL CONFORM TO ASTM A-36.
- W SECTIONS SHALL CONFORM TO ASTM A992, GRADE 50. 2. STEEL PIPE COLUMNS SHALL CONFORM TO ASTM A-53, TYPE E OR S, GRADE B.
- 3. STEEL TUBE SECTIONS SHALL CONFORM TO ASTM A500, GRADE B.
- 4. STEEL PLATES SHALL CONFORM TO ASTM A-282, GRADE "A".
- 5. ALL DETAILING SHALL CONFORM TO CURRENT AISC SPECIFICATIONS. 6. ALL WELDING SHALL CONFORM TO CURRENT AISC AND AWS 1.1 SPECIFICATIONS, AND SHALL PERFORMED BY CERTIFIED WELDERS APPROVED BY THE LOCAL BUILDING AUTHORITY. ALL SHOP WELDING SHALL BE IN AN APPROVED FABRICATORS SHOP AUTHORIZED BY
- THE BUILDING AUTHORITY OR SPECIFIC INSPECTION PER IBC. 1. ALL COMPLETE JOINT PENETRATION WELDS REQUIRE SPECIAL INSPECTION AND UT TESTING. 8. ALL WELDING ELECTRODES SHALL BE ETØXX OR SHIELDED WIRES WITH FY GREATER THAN
- OR EQUAL TO TO KSI. 9. BOLTS, NUTS, AND SCREWS SHALL CONFORM TO ASTM A307 GRADE "A".
- 10. HIGH STRENGTH BOLTS SHALL BE ASTM A325, CONTACT FACES OF STEEL CONNECTIONS WHERE HIGH STRENGTH BOLTS ARE TO BE USED SHALL NOT BE PAINTED. II. ALL FOUNDATION BOLTS SHALL BE ASTM A-36 GALVANIZED ALL THREAD OR ASTM A307 UNFINISHED BOLTS. ALL SILL BOLTS IN SEISMIC ZONE 4 SHALL BE FIVE-EIGHTHS INCHES IN
- AN INCH LARGER THAN SPECIFIED BOLT. 12. ALL STRUCTURAL STEEL AND MISCELLANEOUS IRON NOT ENCASED IN CONCRETE SHALL RECEIVE ONE SHOP COAT OF APPROVED PRIMER PAINT.
- 13. ALL STEEL EXPOSED TO WEATHER SHALL BE HOT-DIP GALVANIZED AFTER FABRICATION OR OTHER APPROVED WEATHER PROOFING METHOD HAVING EQUIVALENT RESULTS MAY BE USED. 14. WHERE NECESSARY, PROVIDE ONE-HALF INCH DIAMETER X THREE INCH NELSON STUDS

DIAMETER BOLT HOLES TO BE ONE-THIRTY-SECONDS OF AN INCH TO ONE-SIXTEENTH OF

- 15. ALL GROUT UNDER STEEL BEARING PLATES SHALL BE SOLID DRYPACK OR NON-SHRINK GROUT PLACED AS DIRECTED BY THE MANUFACTURER.
- 16. PROVIDE WELDER'S CERTIFICATE FOR ALL SINGLE-PASS FILLET WELDS NOT EXCEEDING 5/16" IN SIZE, OR PROVIDE THE CERTIFICATE OF COMPLIANCE THAT THE WORK WAS PERFORMED IN AN APPROVED FABRICATOR'S SHOP.

#### WOOD FRAMING NOTES

- 1. ALL LUMBER FRAMING AND BEARING STUDS TO BE DOUGLAS FIR-LARCH WITH MOISTURE CONTENT LESS THAN 19%.
- GLUE LAMINATED TIMBER BEAMS TO BE APA/EWS MARKED 24F-V4. GLU-LAMS EXPOSED TO WEATHER SHALL BE RATED FOR EXTERIOR USE BY THE MANUFACTURER OR AN APPROVED PROTECTION FROM EXPOSURE SHALL BE PROVIDED.
- LAMINATED VENEER LUMBER (LVL) TO BE 1.9E, Fb=2600 P61. Fv= 285 P61 EQUIVALENT OR BETTER FOR MEMBERS LESS THAN 10' DEEP, CONNECT PLIES WITH (2) ROWS 16D BOX NAILS AT 12" O.C. FOR MEMBERS GREATER THAN 10" DEEP, CONNECT PLIES WITH (3) ROWS 16d BOX NAILS AT 12"O.C. FOR THREE PIECE MEMBER, NAILING SPECIFIED 15 FROM FACH SIDE
- PARALLEL STRAND LUMBER (PSL) TO BE 2.0E, Fb= 2900 PSI Fv= 290 PSI EQUIVALENT
- OR BETTER • 4x AND SMALLER FRAMING TO BE DF #2.
- 6x AND LARGER FRAMING TO BE DF #1. INTERIOR NON-BEARING STUDS AND PLATES MAY BE CONSTRUCTION GRADE OR BETTER. 2. APA RATED SHEATHING SHALL BE MANUFACTURED WITH EXTERIOR GLUE IN ACCORDANCE
- WITH THE REQUIREMENTS OF THE IBC AND PS 1-1, PS-2, OR APA PRP-108. SHEAR PLYWOOD SHALL BE C-D, C-C, 303 (TI-II), OR AN APPROVED EQUAL 3. ALL RESAUN AND ROUGH SAUN BEAMS ARE TO BE FREE OF HEART CENTER
- 4. ALL FRAMING CLIPS AND DEVICES SHALL BE "SIMPSON TIE" OR ICC APPROVED EQUAL 5. MINIMUM NAILING FOR CONNECTION NOT INDICATED ON THE DRAWINGS SHALL BE IN ACCORDANCE WITH IBC
- 6. ALL MULTIPLE TRIMMERS, MULTIPLE STUDS, OR POSTS SHALL BE STACKED IN ALL WALL FRAMING CONNECTED WITH POSITIVE CONNECTIONS, SOLID BLOCKING SIMILAR IN SIZE TO FRAMING ABOVE SHALL BE PROVIDED AT ALL FLOORS ALL THE WAY DOWN TO THE FOUNDATION.
- T. DO NOT NOTCH BEAMS, JOISTS, OR STUDS. 8. ALL NAILS SHALL BE "COMMON" WIRE NAILS AND SHALL CONFORM TO THE FOLLOWING:

SPECIFICA	ATIONS		
SIZE	SHANK DIA.	LENGTH	EQUIVALENT STAPLE SIZES
8d	Ø.131 <b>'</b>	21/2"	13 GA x 1-3/4"
1Ød	Ø.148 <b>"</b>	3'	12 GA × 1-3/4"
16d	Ø.162"	31/2"	

- NO SUBSTITUTIONS UNLESS APPROVED IN WRITING BY K2 ENGINEERING OR SPECIFICALLY ADDRESSED IN THESE CALCULATIONS OR THE PLANS. ALL NAILS EXPOSED TO WEATHER SHALL BE GALYANIZED. • REFER TO SIMPSON SPECIFICATIONS FOR FRAMING HARDWARE ATTACHMENT REQUIREMENTS.
- ALL NAILS SHALL MEET THE REQUIREMENTS OF ASTM F 1667. 9. SHEATH AND NAIL ALL SHEAR PANELS AND GABLE END TRUSSES THE SAME AS THE SHEAR WALL ABOYE OR BELOW.
- 10. CONNECT DOUBLE STUDS, DOUBLE JOISTS, OR ANY OTHER MULTIPLE PIECE MEMBER W/
- 11. TYPICAL LOAD BEARING AND EXTERIOR STUDWALL CONSTRUCTION: STUD HEIGHT ≤ 10'-0" - 2x4 @ 16" O.C.
- STUD HEIGHT ≤ 14'-0" 2x6 @ 16" O.C. STUD HEIGHT ≤ 18'-0" - 1-3/4" x 5-1/2" L.Y.L. @ 12" O.C.
- STUD HEIGHT ≤ 22'-0" 1-3/4" x 7-1/4" L.V.L. @ 12" O.C. STUD HEIGHT ≤ 27'-0" - 1-3/4" x 9-1/4" L.Y.L. @ 12" O.C. 12. USE (2) CONT. KING STUDS E.S. OF OPENINGS WHERE STUD HEIGHT EXCEEDS 10'-6" UN.O.
- ALWAYS RAKE/BALLOON FRAME STUDWALLS. 13. PORTIONS OF STRUCTURAL GLU-LAM BEAMS, WHICH ARE EXPOSED TO WEATHER, SHALL BE PRESSURE TREATED OR WOOD OF NATURAL RESISTANCE TO DECAY, EQUIVALENT PROTECTION MAY BE PROVIDED WITH TWO COATS MINIMUM OF SEALER.

DO NOT BREAK CONT. KING STUDS BY SPANNING HEADER OVER MULTIPLE OPENINGS.

## ROOF FRAMING NOTES

- 1. ROOF LOADS: LIVE = 30 PSF : DEAD = 20 PSF 2. USE (1)-LAYER 5/8" (40/20) CDX APA RATED ROOF SHEATHING OR OSB EQUIVALENT, APPLY FACE GRAIN/LONG DIMENSION PERPENDICULAR TO SUPPORT FRAMING. STAGGER PANELS AND NAIL WITH 100'S AT 6' O.C. EDGES AND BOUNDARIES AND 100'S AT 12' O.C. FIELD. NAIL ALL DRAG MEMBERS, SHEAR PANELS, BLOCKING, E.T.C. W/ NAILS SPACED AT 4" O.C. SEE DETAIL FOR ADDITIONAL NAILING REQUIREMENTS.
- 3. USE (2) TRIMMERS AND (1) KING STUD UNDER ALL OPENINGS  $\underline{6'-0'}$  OR GREATER. 4. CONNECT TRUSS BLOCKING AND GABLE END TRUSSES TO TOP PLATE OR BEAM BELOW WITH A35's, LTP4's, LTO's, OR L650's @ 48" O.C. UNLESS NOTED OTHERWISE. 5. DOUBLE TOP PLATE LAP SPLICES SHALL BE 4'-@' MINIMUM AND FACE NAILED WITH
- (12)-16d NAILS. 6. THE FOLLOWING COLUMN/POST CAPS ARE INTERCHANGEABLE: CC, ECC, CCQ, & ECCQ. 7. WHERE HEADERS ARE PLACED HIGH IN THE WALL AND BREAK THE DOUBLE TOP PLATE,
- AN MSTC28 SHALL CONNECT THE HEADER TO THE TOP PLATE AT EACH END. 8. ENCLOSED ATTIC AND RAFTER SPACES SHALL HAVE CROSS VENTILATION BY OPENINGS EQUAL TO 1/150TH OF THE AREA. WHERE EAVE OR CORNICE VENTS ARE INSTALLED, INSULATION SHALL NOT BLOCK THE FREE FLOW OF AIR. A MINIMUM OF 1" OF AIR SPACE SHALL BE PROVIDED BETWEEN THE INSULATION AND ROOF SHEATHING. ROOFS WITH

### RAFTERS, BAYS AND/OR YAULTED CEILINGS MUST BE VENTILATED TO OUTSIDE AT RIDGE. PRE-MANUFACTURED WOOD ROOF TRUSSES

- 1. TRUSS MANUFACTURER SHALL BE RESPONSIBLE FOR ALL ENGINEERING, LAYOUT DRAWINGS, CONNECTIONS, BLOCKING, BRACING, AND TRUSS ERECTION INFORMATION. THE CONTRACTOR SHALL BE RESPONSIBLE FOR PROPER COORDINATION BETWEEN ENGINEER/ARCHITECT DRAWINGS, TRUSS MANUFACTURER INFORMATION, ANY REQUIRED FIELD CHANGES, PROPER INSTALLATION OF FINAL PRODUCT AND ITS CONFORMANCE TO THE ARCHITECT'S DESIGN.
- THE ARCHITECT AND ENGINEER ASSUME NO LIABILITY FOR SAID PRODUCT. 2. TRUSS MANUFACTURER TO VERIFY LOCATION OF AND PROVIDE REINFORCED TRUSSES FOR THE SUPPORT OF ANY MECHANICAL EQUIPMENT WHERE OCCURRING. 3. TRUSS MANUFACTURER TO VERIFY LOCATION OF AND DESIGN FOR ALL CEILING HEIGHT
- CHANGES, ATTIC ACCESSES, RETURN AIR GRILLS, ETC. TRUSS MANUFACTURER TO COORDINATE ANY FINDINGS TO BOTH K2 ENGINEERING AND THE ARCHITECT.
- 4. DEAD LOAD DEFLECTIONS SHALL BE LIMITED TO L/240. 5. GABLE END TRUSSES SHALL BE STRUCTURAL, DESIGNED TO SUPPORT OVERHANG AND TO
- ALLOW A TOP CHORD NOTCH OF ONE AND A HALF INCHES. 6. ALL NON-BEARING WALLS ARE TO HAVE A ONE-FOURTH OF AN INCH GAP TO THE BOTTOM CHORD OF THE TRUSSES. SECURE BOTTOM CHORD TO WALL BELOW WITH

8. HANG TRUSSES AND GIRDER TRUSSES W/SIMPSON HUS26 OR AS SPECIFIED ON PLAN.

- 7. USE PRE-ENGINEERED MANUFACTURED TRUSSES @ 24" O.C. SOLID BLOCK @ ALL SUPPORTS AND PER MANUFACTURER'S SPECIFICATIONS. USE SIMPSON HI @ EACH SUPPORT WALL/BEAM TO EACH TRUSS AND HG @ EACH SUPPORT WALL/BEAM TO EACH GIRDER TRUSS.
- TRUSS CALCULATIONS HOLD PRECEDENCE OVER PLAN AT ALL TRUSS TO TRUSS 9. TRUSSES ARE TO BE HANDLED, INSTALLED, AND BRACED IN ACCORDANCE WITH HIB-91 OF THE TRUSS PLATE INSTITUTE (TPI).

## FOUNDATION/FLOOR FRAMING NOTES

- 1. ALL EXTERIOR WALLS SHALL BE CONSIDERED SHEARWALLS NAILED AS TYPE "6" WALLS (SEE SHEARWALL SCHEDULE).
- 2. FLOOR SHEATHING SHALL BE T. &G. APA RATED STURD-I-FLOOR APPLY FACE GRAIN/LONG DIMENSION PERPENDICULAR TO SUPPORT FRAMING. STAGGER PANELS AND NAIL WITH 10d AT 6" O.C. AT ALL EDGES AND BOUNDARIES (BLOCKING AT INTERIOR SHEAR WALLS,
- DRAG MEMBERS, ETC.), AND 10d AT 10" O.C. FIELD. GLUE AND NAIL THROUGHOUT. 3. FLOOR JOISTS SHALL BE BLOCKED SOLID @ ALL SUPPORT LINES (CONNECT BLOCKING TO WALL/BEAM BELOW WITH A35'S @ TWICE THE JOIST SPACING), BENEATH ALL INTERIOR-BEARING WALLS, AND UNDER ALL HOLDOWNS. USE DOUBLE JOISTS BELOW ALL PARALLEL INTERIOR-BEARING WALLS. PROVIDE L.S.L. RIM BOARD THROUGHOUT. PROVIDE CRUSH BLOCKS, WEB STIFFENERS, ETC. PER MANUFACTURER'S SPECIFICATIONS.
- 4. ALL FLOOR OPENINGS SHALL BE BETWEEN JOISTS. 5. ALL HOLDOWNS SHALL BE INSTALLED AT THE TIME APPROPRIATE MEMBERS ARE FRAMED AND ACCORDING TO MANUFACTURER'S SPECIFICATIONS. IF STRUCTURE IS MULTIPLE STORIES, AS MUCH AS POSSIBLE, LINE FLOOR-TO-FLOOR HOLDOWNS UP WITH FLOOR-TO-FOUNDATION HOLDOWNS SO THAT HOLDOWNS ARE ATTACHED TO COMMON MEMBERS. USE SHEAR PLY
- NAILING TO ALL HOLDOWN MEMBERS. 6. PROVIDE FULL BEARING, FULL DEPTH BLOCKING UP TO FLOOR TO SUPPORT POSTS, DOUBLE STUDS, OR DOUBLE TRIMMERS ABOVE.
- WHERE COLUMN BASE OR POST BASE IS CALLED OUT ON A PIER BENEATH THE SUBFLOOR PROVIDE POST UP TO SUBFLOOR TO SUPPORT IDENTICAL POST ABOVE, USE (2) SIMPSON 5T6224 ON OPPOSITE SIDES OF POST TO STRAP POST ABOVE THROUGH THE FLOOR TO THE POST BELOW.
- T. ANCHOR BOLTS: FOR 2x SILL PLATE, USE ½ v x 10 AB.
- FOR 3x OR DOUBLE SILL PLATE, USE <sup>5</sup>/<sub>8</sub> " + x 12" A.B. EXTEND SILL BOLTS 7' INTO FOUNDATION MINIMUM, MAXIMUM SPACING SHALL BE 4'-0' O.C. WITH MINIMUM (2) BOLTS IN EACH SILL BOARD. BOLTS SHALL BE LOCATED NOT MORE THAN (12) NOR LESS THAN (7) BOLT DIAMETERS FROM EACH END OF SILL PIECE. MINIMUM 3'x3'x'4' THICK PLATE WASHERS SHALL BE INSTALLED ON EACH SILL BOLT. SPACE WASHER
- 1/2" FROM SHEATING OR RIM. • SILL PLATES: USE FOUNDATION GRADE REDWOOD OR TIMBERSTRAND L.S.L. TREATED WITH ZINC BORATE OR PRESSURE TREATED MUDSILL. SEE SHEARWALL SCHEDULE FOR IMPORTANT INFORMATION REGARDING SILL PLATES, FOR ALL SILL PLATES NOT NOTED, USE 2'X WALL WIDTH WOOD SILL. ALL SHEAR WALLS, EXCEPT TYPE '6' AND '4', REQUIRE FOUNDATION SILL PLATES AND ALL FRAMING MEMBERS RECEIVING EDGE NAILING FROM ABUTTING PANELS TO BE NOT LESS THAN A SINGLE 3' NOMINAL MEMBER PLYWOOD JOINT AND SILL PLATE NAILING SHALL BE STAGGERED IN ALL CASES.
- 8. AN 8' WIDE CONCRETE FOUNDATION WALL SHALL BE CENTERED ON CONTINUOUS FOOTING BELOW W/(1)\*4 CONTINUOUS @ TOP OF WALL & \*4 VERTICALS @ 32" O.C. MAX HOOKED AT FOOTING (ALTERNATE HOOKS).
- 9. CONTINUOUS CONCRETE FOOTINGS TO BE 16"X10" W/ (2) \*4's CONT. STEP FOOTING AS REQUIRED TO BEAR ON NATIVE GRADE OR AS DIRECTED BY SOILS ENGINEER EXTEND EXTERIOR FOOTING DEPTHS TO FROST LINE (2'-0").

10. THE FOLLOWING COLUMN/POST BASES ARE INTERCHANGEABLE: CB & CBQ OR CBS & CBSQ.

- 11. ALL SLABS TO BE 4" THICK CONCRETE W/ 6x6 10/10 WELDED WIRE FABRIC REINFORCING OR CONCRETE SHALL HAVE FIBERMESH ADDITIVE @ PLANT. SLAB SHALL BE PLACED OVER 4" TYPE-II BASE COMPACTED TO 90% RELATIVE DENSITY ON UNDISTURBED NATIVE SOIL. 12. REFERENCE HOLDOWN SCHEDULE FOR IMPORTANT INFORMATION PERTAINING TO FOOTINGS. 13. STAIRWAYS SHALL NOT BE LESS THAN 36" IN WIDTH. EVERY STAIRWAY SHALL HAVE
- MINIMUM 6'-8" HEADROOM. THE MAXIMUM VERTICAL HEIGHT ALLOWED BETWEEN LANDINGS IS 12'-0". THE RISE OF STEPS IN THE STAIRWAY SHALL NOT EXCEED 8', AND THE TREAD SHALL BE NOT LESS THAN 9". 14. STAIR HANDRAILS SHALL BE PLACED NOT LESS THAN 34" NOR MORE THAN 38" ABOVE
- LANDINGS AND THE NOSING OF THE TREADS. THEY SHALL BE CONTINUOUS FOR THE FULL LENGTH OF THE STAIRS AND THE ENDS SHALL BE RETURNED. IN RESIDENTIAL OCCUPANCIES HANDRAILS MAY HAVE STARTING NEWELS WITHIN THE FIRST TREAD. HANDGRIP PORTION OF HANDRAILS SHALL BE NOT LESS THAN 11/4" NOR MORE THAN 2" IN CROSS-SECTIONAL DIMENSION AND HAVE A SMOOTH GRIPPING SURFACE. A SPACE OF NOT LESS THAN  $1\frac{1}{2}$
- SHALL BE PROVIDED BETWEEN THE WALL AND THE RAIL. 15. GUARDRAILS SHALL BE A MINIMUM OF 42" HIGH, UN.O. NO OPENINGS OVER 4", TRIANGULAR OPENINGS FORMED BY THE RISER, TREAD AND BOTTOM RAIL OF A GUARD AT THE OPEN SIDE OF A STAIRWAY ARE PERMITTED TO BE OF SUCH SIZE THAT A SPHERE 6' IN DIAMETER
- CANNOT PASS THROUGH. 16. FIRE BLOCKING BETWEEN CHIMNEYS AND COMBUSTIBLE CONSTRUCTION SHALL BE INSTALLED AT 10'-0' INTERVALS, BOTH VERTICAL AND HORIZONTAL
- 17. INSTALL ADHERED VENEER IN COMPLIANCE WITH LOCAL CODES, FOUNDATION SUPPORT REQUIRED FOR EXTERIOR ROCK VENEER. ANCHOR TIES SHALL BE PROVIDED TO HORIZONTAL JOINT REINFORCEMENT WIRE OF NO. 9 GAUGE OR EQUIVALENT. 18. EXTERIOR STUCCO WALLS SHALL HAVE A WEEP SCREED AT OR BELOW THE FOUNDATION
- PLATE LINE AND 4' ABOVE GRADE OR 2' ABOVE PAVED AREAS THAT WILL ALLOW TRAPPED WATER TO DRAIN TO THE EXTERIOR OF THE BUILDING. THE WEATHER-RESISTIVE BARRIER SHALL LAP THE ATTACHMENT FLANGE, AND THE EXTERIOR LATH SHALL COVER AND TERMINATE ON THE ATTACHMENT FLANGE OF THE SCREED. 19. COLUMNS OR POSTS LOCATED ON CONCRETE OR MASONRY FLOORS AND THAT SUPPORT
- PERMANENT STRUCTURES SHALL BE SUPPORTED BY CONCRETE PIERS OR METAL PEDESTALS PROJECTING ABOVE EXPOSED EARTH A MINIMUM OF 6' AND AT LEAST I' ABOVE SUCH FLOORS UNLESS TREATED WOOD IS USED. INDIVIDUAL CONCRETE OR MASONRY PIERS SHALL PROJECT AT LEAST 8" ABOVE EXPOSED GROUND UNLESS THE COLUMNS OR POSTS
- THAT THEY SUPPORT ARE OF WOOD RESISTANT TO DECAY. 20. MINIMUM CLEARANCE FROM GROUND UNDER GIRDERS SHALL BE 12 INCHES±
- UNDER JOISTS SHALL BE 18 INCHES. 21. UNDERFLOOR VENTS SHALL EQUAL 1 SQUARE FOOT FOR EACH 150 SQUARE FEET

5-1/4x9-1/2 PSL 2.0E

5-1/8x10-1/2 GLB 24F-V4

Pier Footing Schedule

< 42 )

( 54 )

 $\langle$  60 angle

42"

48"

54"

60"

#### OF UNDERFLOOR AREA, AND MUST PROVIDE CROSS VENTILATION. Beam Equivalent Table DF No. 1 BEAM EQUIVALENT BEAM DF No. 1 BEAM EQUIVALENT BEAM 6x8 DF No. 1 RMT 6x12 DF No. 1 RMT (2) 1-3/4×9-1/2 LVL 1.9E (2) 1-3/4x14 LVL 1.9E 6×12 6x8 5-1/4×9-1/2 PSL 2.0E 5-1/4x11-7/8 PSL 2.0E 5-1/8x7-1/2 GLB 24F-V4 5-1/8x12 GLB 24F-V4 6x10 DF No. 1 RMT 6x14 DF No. 1 RMT (2) 1-3/4x11-7/8 LVL 1.9E (2) 1-3/4x16 LVL 1.9E 6x10 6x14

#### - DENOTES FOOTING SIZE - DENOTES FOOTING SIZE <u>DEPTH</u> (NA TO <u>SYMBOL</u> <u>DEPTH</u> <u>SYMBOL</u> (EACH SIDE) (EACH WAY) (MIN) (CONTINUOUS) MONOPOUR) 12 12" (2) #4's (2) #4 16 ( 14 ) (2) #4's (2) #4's 14" (16) 18 (2) #4's (2) #4's (ાક (3) #4's (2) #4's 24 < 21 <sup>-</sup> (3) #4's 24" (2) **\***4's ( 24 ) 28 (3) #4's (3) **#**4's 32 (28) 28" (3) #4's (3) #4's 36 ( 32 ) 32' (4) #4's (3) #4's < 36 ° (5) #4's

(6) #4's

(1) #4's

(8) **\***4's

(9) #4's

## LOADING AND EARTHQUAKE DESIGN DATA:

1. LOADING: FLOOR LOADS: LIVE = 40 PSF : DEAD = 12 PSF ROOF LOADS: LIVE = 30 PSF : DEAD = 20 PSF 2. EARTHQUAKE DESIGN DATA:

5-1/4x14 PSL 2.0E

5-1/8x13-1/2 GLB 24F-V4

Continuous Footing Schedule

6s = 150, 61 = 0.835, 6D6 = 1.00, 6D1 = 0.835SEISMIC DESIGN CATEGORY: D BASE SHEAR Y = Co+W = (1+Rho+F+SDS/1.4+R)+W R = 6.5 (LIGHT FRAMED WOOD WALLS SHEATHED WITH WOOD STRUCTURAL PANELS RATED FOR SHEAR RESISTANCE).

## Design Parameters

**Abbreviations** 

AB

ATR

BM

BRG

BET

BLK

BOT

CANT

MAX

MIN

(N)

NTS

O.C.

PSF

REQ'D

SCHED

SAD

SIM

SQ

STD STL THD

T&G

T#B

UNO

WWM

TRIMM

SPEC

MECH

BS(B/S.

APPROX

ALTERNATE

BEAM

BEARING

BETWEEN

BOTH SIDES

CANTILEVER

CENTERLINE

CONTINUOUS

CONTROL JOINT

COUNTERSINK

DEAD LOAD

DETAIL

DIAMETER

DIMENSION

DOUGLAS FIR

DRAG TRUSS

DRAWING

EACH END

LEACH FACE

EACH SIDE

|EACH WAY

EDGE NAIL

ELEVATION

|EMBEDMENT

EQUAL

EXISTING

FLOOR

FOOTING

FIELD NAIL

FOUNDATION

GALYANIZED

HEADER

HEM-FIR

HORIZONTAL

INTERIOR

KING STUD

LIVE LOAD

MAXIMUM

MINIMUM

MECHANICAL

MACHINE BOLT

MANUFACTURER

MISCELLANEOUS

NOT TO SCALE

ON OR OVER

PENETRATION

ON CENTER

PLATE

PLYWOOD

REDWOOD

REFERENCE

REQUIRED

SCHEDULE

SIMILAR

SQUARE

STEEL

STANDARD

THREADED

TOP PLATE

TUBE STEEL

TRIMMERS

YPICAL

VERTICAL

TOP & BOTTOM

TONGUE & GROOVE

WELDED WIRE MESH

UNLESS NOTED OTHERWISE

SHEAR WALL

SPECIFICATION

INFORMATION

HEIGHT

GLUED-LAMINATED BEAM

KING STUD EACH SIDE

LAMINATED VENEER LUMBER

POUND PER SQUARE FOOT

POWDER DRIVEN FASTENER PDF

POUND PER SQUARE INCH

PRESERVATIVE TREATED

SEE ARCHITECTURAL DRAWINGS

EXTERIOR

DOUBLE

CONCRETE MASONRY UNIT

CONCRETE

BLOCK

BOTTOM

ANCHOR BOLT

APPROXIMATE

ALL THREAD ROD

2012 IBC AND LOCAL DESIGN CRITERIA

PROJECT ELEVATION: SITE CLASS:

130 MPH (3 SECOND GUST) WIND SPEED: WIND EXPOSURE:

DESIGN INCLUDES SNOW LOAD FOR DRIFT AND UNBALANCED LOADING.

5090'

CONC CMU		Shearwall Schedule										
CONT CJ CS	Shearwall Symbol	Sheathing Thickness	Nail Size	Shear Na E.N.	il Spacing F.N.	16d Nail Spacing	1/4" SDS Spacing	3x Framing at Adj. Panel Edges	5/8" A.B. Spacing	1/2" A.B. Spacing	**MASA SPACINO	
DL DET 0	6	3/8"	8d	6' O.C.	12" O.C.	6' O.C.	16' O.C.	No	48' O.C.	48" O.C.	48" O.C.	
DBL DBL DF	4	3/8'	8d	4' O.C.	12" O.C.	4' O.C.	12 <b>'</b> O.C.	No	48' O.C.	35' O.C.	48" O.C.	
DT DWG EA	3	3/8"	8d	3" O.C.	12 <b>'</b> O.C.	3 <b>'</b> O.C.	8' O.C.	Y <i>e</i> s.	46' O.C.	30' O.C.	38' O.C.	
EE EF E6	2	3/8'	8d	2" O.C.	12' O.C.	2" O.C.	6' O.C.	Yes	35' O.C.	23 <b>'</b> O.C.	29' O.C.	
EN ELEY	4/2	3/8" B/9	1Ød	4" O.C.	12' O.C.	2" O.C.	5-1/2" O.C.	Yes	32' O.C.	21 <b>'</b> O.C.	26' O.C.	
EMBED EQ (E)	3/2	3/8 <b>'</b> B/9	8d	3" O.C.	12 <b>'</b> O.C.	N/A	4' O.C.	Yes	23 <b>'</b> O.C.	15" O.C.	19" O.C.	
EXT FIN FLR	2/2	3/8" B/6	8d	2 <b>'</b> O.C.	12' O.C.	N/A	3' O.C.	Yes	18" O.C.	11' O.C.	14' O.C.	
FTG FN FDN	A	1/2"	1Ød	2" O.C.	12' O.C.	N/A	6' O.C.	Yes	29" O.C.	19" O.C.	24' O.C.	
GA GALY GLB	B	5/8'	100d	2" O.C.	12' O.C.	N/A	5-1/2" O.C.	Yes	26' O.C.	16" O.C.	21" O.C.	
#DR ## ##	L6	7/16" Smart Panel Siding	8d	6' O.C.	12' O.C.	6' O.C.	16' O.C.	No	48' O.C.	48' O.C.	48" O.C.	
HORIZ INFO INT	L4	7/16" Smart Panel Siding	8d	4' O.C.	12' O.C.	4' O.C.	16' O.C.	No	48' O.C.	39' O.C.	48" O.C.	
JST KS KSES	L3	7/16' Smart Panel Siding	8d	3' O.C.	12' O.C.	3' O.C.	12 <b>'</b> O.C.	Yes:	48' O.C.	36' O.C.	45" O.C.	
	L2	7/16' Smart Panel Siding	8d	2' O.C.	12' O.C.	2" O.C.	8' O.C.	Yes*	42 <b>'</b> O.C.	27' O.C.	35' O.C.	

### USE APA RATED SHEAR PLYWOOD / OSB / TI-11 SHEATHING OR AN APPROVED EQUAL U.N.O. NAILS SHALL BE COMMON OR GALVANIZED BOX. NAIL HEADS ARE NOT TO PENETRATE PLYWOOD.

- ALL FIELD NAILING SHALL BE AT 6" O.C. U.N.O. ALL SHEAR WALL STUDS SHALL BE DOUGLAS FIR LARCH SPACED AT 16' O.C.
- NAIL ALL SHEAR PLY WITH EDGE NAIL SPACING AT TOP PLATES, MUD SILLS, ALL POSTS, ALL KING STUDS, AND ALL STUDS WITH HOLDOWNS, WHERE APPLICABLE, PLYWOOD JOINT AND SILL PLATE NAILING SHALL BE STAGGERED IN ALL CASES.
- FOR ALL SHEAR WALLS EXCEPT TYPES 6, 4, L6, L4 USE: A. 3 INCH NOMINAL OR THICKER SILL PLATES WITH 5/8" X 12" AB's AND (2) 200 BOX NAILS FOR STUD END NAILING.
- B. 3 INCH NOMINAL OR THICKER FRAMING MEMBERS, OR DOUBLE 2x FRAMING MEMBERS STITCHED TOGETHER WITH MINIMUM (2) ROWS OF 16d NAILS @ 12" O.C. AT ALL ADJOINING PANEL EDGES.
- 5. FOR SHEAR WALL TYPE 3, L3, \$ L2, A 2" NOMINAL SILL PLATE MAY BE USED IF ANCHOR BOLTS ARE SPACED AT 1/2 THE SPECIFIED SPACING. PROVIDE BLOCKING OR SOLID FRAMING AT ALL PANEL EDGES.
- 10. DOUBLE SHEAR WALLS TO HAVE SHEAR PLY WITH SPECIFIED NAILING BOTH SIDES. PROVIDE 3" NOMINAL OR THICKER FRAMING MEMBERS ON ALL ABUTTING PANEL EDGES. B/S IS BOTH SIDES.
- USE SIMPSON MSTC48 OR MSTC52 TO STRAP ACROSS BEAMS AT ANY BREAK IN TOP PLATES, UN.O. ?. FOUNDATION SILL ANCHOR BOLTS SHALL BE 5/8"XIØ" SPACED AT 48" O.C. ON ALL EXTERIOR WALL U.N.O. 13. USE 3'X3'X'4' THICK STEEL PLATE WASHERS AT ALL WOOD SILL PLATES FOR SHEAR WALLS. SPACE WASHERS 1/2' FROM
- 14.\*\* ALL MASA MUDSILL ANCHORS TO BE INSTALLED WITH BOTH LEGS FULLY NAILED INTO SIDE AND TOP OF SILL PLATE. 15. ALL NOTES ARE TYPICAL U.N.O.

## Holdown Schedule

FLOOR TO FLOOR HOLDOWN CONNECTIONS (SEE DETAILS FOR APPLICATIONS) Min. Vertical Wood Thickness (2) Additional Comments Symbol Holdown (1) **H**−1 MSTC4Ø (2) - 2x Studs H-2 MSTC52 N/A (2) - 2x Studs H-3 MSTC66 N/A (2) - 2x Studs H-4 CMST14 (2) - 2x Studs Strap to be at Least 80' Long H-5 CMST12 (2) - 2x Studs Strap to be at Least 102" Long

FLOOR TO FOUNDATION HOLDOWN CONNECTIONS (SEE DETAILS FOR APPLICATIONS)

	Holdown Options	Min. Vert Wood Thickness	Foundation Installation Options (10)				2 5 11
Symbol (T)			Threaded Rod w/	Simpson SB	Threaded Rod Retrofit w/	CL (8)	Screws, Bolts, or Nails
		(2)	Dbl. Nutted BP	(4)	SET-XP Epoxy (5)		(6)
	STHD1Ø (9)	(2) - 2x Studs	N/A	N/A	N/A	N/A	(20) 16d
H-1Ø	HTT4	(2) - 2x Studs	5/8" W/ 12" Embed	SB 5/8x24 W/ 18" Embed	5/8" W/ 12" Embed	1-3/8"	(18) 16d
	HDU2-9D92.5	(2) - 2x Studs	5/8" W/ 14" Embed	SB 5/8x24 W/ 18" Embed	5/8" W/ 12" Embed	1-5/16"	(6) SDS
H-11	STHD14 (9)	(2) - 2x Studs	N/A	N/A	N/A	N/A	(24) 16d
	HTT5	(2) - 2x Studs	5/8" W/ 14" Embed	SB 5/8x24 W/ 18" Embed	5/8" W/ 12" Embed	1-3/8"	(26) 16d
	HDU4-5D52.5	(2) - 2x Studs	5/8" W/ 14" Embed	5B 5/8x24 W/ 18" Embed	5/8" W/ 12" Embed	1-5/16"	(1Ø) SDS
H-12	HDU5-SDS2.5	(2) - 2x Studs	5/8' W/ 20' Embed	5B 5/8x24 W/ 18" Embed	5/8" W/ 16" Embed	1-5/16"	(14) SDS
	HDTB	(2) - 2x Studs	7/8" W/ 20" Embed(3)	5B 7/8x24 W/ 18" Embed	7/8' W/ 16' Embed	1-1/4"	(3) 3/4" Bolts
H-13	HDQ8-5D53	3-1/2"	7/8" W/ 20" Embed(3)	PABT W/8" Embed Into FTG. (3)	N/A	1-1/4"	(2Ø) SDS
	HD9B	3-1/2"	7/8" W/ 20" Embed(3)	N/A	N/A	1-1/4"	(3) 7/8" Bolts
H-14	HHDQII	5-1/2"	1' W/8' Embed Into Ftg. (3)	N/A	N/A	1-1/2"	(24) SDS
	HD12	5-1/2"	1' W/8' Embed Into Ftg. (3)	N/A	N/A	2-1/8"	(4)   Bolts
H-15	HHDQ14	5-1/2"	1' W/8' Embed Into Ftg. (3)	N/A	N/A	1-1/2"	(3Ø) SDS
	HDU14-5D52.5	5-1/2"	1' W/8' Embed Into Ftg. (3)	N/A	N/A	1-9/16"	(36) SDS

Embed Into Ftg. (3) INSTALL ALL HOLDOWNS PER SIMPSON'S SPECIFICATIONS, MAINTAINING REQUIRED EDGE CLEARANCES. DOUBLE STUDS TO BE CONNECTED BY (2) ROWS OF 16d NAILS AT 4" O.C. STAGGERED

. USE (1) \*4 BAR VERTICAL EACH SIDE OF 1/8" OR GREATER THREADED ROD (TOTAL OF 2) TO CONNECT STEMWALL TO FOOTING. . SB EMBEDMENTS ARE FOR SINGLE POUR INSTALLATION ONLY. REFER TO LATEST VERSION OF SIMPSON CATALOG FOR DOUBLE POUR APPLICATIONS SPECIAL INSPECTION IS REQUIRED AT ALL EPOXY-SET ANCHORS. CONTACT ENGINEER OF RECORD 2-WORKING

APPLICATIONS & SET EPOXY FOR SOLID GROUTED CMU APPLICATIONS . BOLT HOLES SHALL BE A MINIMUM OF 1/32" AND NO MORE THAN 1/16" LARGER THAN THE SPECIFIED BOLT DIAMETER. . IT IS ACCEPTABLE TO SUBSTITUTE HOLDOWNS SPECIFIED HIGHER IN THE TABLE WITH HOLDOWNS OCCURING LOWER IN THE TABLE.

. "CL" IS IS THE DIMENSION TO THE CENTERLINE OF AB HOLE IN HOLDOWN.

9. USE STHORJ WHEN RIM JOIST IS PRESENT.

DAYS IN ADVANCE PRIOR TO INSTALLATION. USE SIMPSON SET-XP EPOXY FOR CONCRETE DRILL & EPOXY

10. USE ASTM A307 ALL THREADED RODS TYPICAL

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Revisions

BTK Checked

16-261

Structural Notes & Schedules

Project No.

