

9222 Prototype Drive Reno, Nevada 89521 Tel. 775.827.6111 Fax 775.827.6122 www.LumosInc.com

# ARROWCREEK MIDDLE SCHOOL SPECIAL USE PERMIT

April 16, 2018

Prepared by:





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# **Project Request**

Project Summary

| Commissioner District: | 2   |
|------------------------|---|
| Applicant:             | Washoe County School District                                   |
| APN Number:            | 049-010-29  |
| Request:               | This is a request for a Special Use Permit to allow grading per |
|                        | Washoe County Development Code Section 110.438.35(a).           |
| Zoning:                | Low Density Suburban (LDS)                                      |
| Master Plan:           | Suburban Residential  |
| Planning Area:         | Southwest Truckee Meadows Planning Area                         |

### Project Background

In November of 2016, Washoe County voters approved WC-1, which gives the Washoe County School District (WCSD) funding to repair and renovate older schools, and to build new schools to address overcrowding. This approval came at a time when population growth within Washoe County has placed a strain on the School District resources and has led to overcrowding in many local schools. As the community is coming out of one of the worst recessions in history, the WCSD is working to address overcrowding in schools. Construction of a new middle school in south Reno has been identified as a high priority and immediate need for the community.

The WCSD has submitted an application requesting conveyance of 60 acres of U.S. Forest Service land through the Education Land Grant Act, which authorizes the Secretary of Agriculture to convey National Forest System lands to a public school district for use for educational purposes.

The WCSD is in the process of designing a new middle school located in south Reno. The proposed site (APN 049-010-29) is zoned Low Density Suburban (LDS), and has a master plan designation of Suburban Residential. The parcel is located in the Southwest Truckee Meadows Planning Area.

The site is located north of Arrowcreek Parkway and west of Thomas Creek Road. The west edge of the parcel abuts Crossbow Court, and is directly adjacent to Hunsberger Elementary and Sage Ridge Schools. The site is surrounded by single family homes to the north, east and south, with a vacant parcel on the northern third of the parcel's west edge and a vacant parcel at its southeastern edge. The property also abuts a 2.43-acre parcel owned by the Truckee Meadows Water Authority, at its northwestern corner.

The proposed use as a WCSD middle school, is allowed by right without discretionary review, in accordance with Article 440, Public School Facilities Design Standards, of the Washoe County



Development Code. Development of the site requires grading that exceeds the thresholds of "Major Grading" in accordance with Article 438, Grading Standards, of the Washoe County Development Code, specifically:

- 1) Grading on slopes of less than fifteen percent of more than four acres on a parcel of any size.
- Excavation of five thousand cubic yards or more, whether the material is intended to be permanently located on the project site or temporarily stored on a site for relocation to another final site.

The proposed developed area of approximately 28.4 acres will result in  $\pm 247,980$  cubic yards of cut and  $\pm 251,748$  cubic yards of fill. This will result in  $\pm 3,767$  cubic yards of overall fill for the project. The cut/fill slopes have been minimized with the addition of 8-foot tall retaining walls. The walls have been designed to create varying curvilinear contours, which breaks up the appearance of a manufactured slope. This is also in accordance with the Southwest Truckee Meadows Area Plan, which requires grading to complement the original contours of the landscape and minimize disruption of the natural topography.

The grading has been visually minimized through extensive landscaping that includes over  $\pm 5.6$  acres of landscape area. A total of 566 trees and 2,492 shrubs are included in the landscape design, which will help to screen the grading from public view on the adjacent roadways and neighboring properties. The mix of trees will include both deciduous and coniferous varieties measuring 1"-2" caliper shade and ornamental trees and 5'-7' tall evergreens. All disturbed areas will be seeded with a native revegetation seed mix and temporary irrigation.

This special use permit application is for grading only, and does not include specific information on the proposed school use. However, background information has been provided to better explain the uniqueness of the property use and parcel ownership.





Figure 1 - Vicinity Map



# ARROWCREEK MIDDLE SCHOOL SPECIAL USE PERMIT





Figure 5 - Site Photographs





# Figure 6 - Site Photographs



# **Special Use Permit Findings**

Prior to approving an application for a special use permit, the Planning Commission, Board of Adjustment or a hearing examiner shall find that all of the following are true:

# **1.** Consistency – The proposed use is consistent with the action programs, policies, standards and maps of the Master Plan and the applicable area plan;

The proposed project is in conformance with Washoe County Master Plan and the Southwest Truckee Meadows Area Plan. There are no specific Policies or Action Programs included in the Southwest Truckee Meadows Area Plan that are applicable to the proposed grading for a public Middle School. However, the proposed project is consistent with the following Policies related to grading and provision of schools:

SW.5.2 – The Washoe County Departments of Community Development and Public Works will establish and oversee compliance with design standards for grading that minimize the visual impact of all residential and non-residential hillside development, including road cuts and driveways.

SW.5.3 – The grading design standards referred to in Policy SW.5.2 will, at a minimum, ensure that disturbed areas shall be finished and fill slopes will not exceed a 3:1 slope, and that hillside grading will establish an undulating naturalistic appearance by creating varying curvilinear contours.

Goal Eight – The Southwest Truckee Meadows planning area will contain an extensive system of trails that integrates other recreational facilities, the Regional Trail System, public lands, schools and transit facilities. This trail system will contribute to the preservation and implementation of community character.

SW.20.3.h – Proposed amendments shall complement the long range plans of facilities providers for transportation, water resources, schools and parks, as reflected in the policy growth level established in Policy 1.2.

SW.20.3.i – If the proposed intensification results in existing public school facilities exceeding design capacity and compromises the Washoe County School District's ability to implement the neighborhood school philosophy for elementary facilities, then there must be a current capital improvement plan or rezoning plan in place that would enable the District to absorb the additional enrollment. The Washoe County Planning Commission, upon request of the Washoe County School Board of Trustees, may waive this finding.



# 2. Improvements – Adequate utilities, roadway improvements, sanitation, water supply, drainage, and other necessary facilities have been provided, the proposed improvements are properly related to existing and proposed roadways, and an adequate public facilities determination has been made in accordance with Division Seven;

Adequate roadways, sanitation, water supply, drainage and other necessary facilities and utilities will be provided to the site, as the grading is intended to create appropriate access and facilitate construction of a public Middle School.

# 3. Site Suitability – The site is physically suitable for the type of development and for the intensity of development;

The property has some topographic constraints that require grading. However, the grading plan has been designed to minimize visual impacts by means of retaining walls and the additional of a significant amount of landscaping. The proposed developed area of approximately 28.4 acres will result in  $\pm 3,767$  cubic yards of overall fill for the project. The cut/fill slopes have been minimized with the addition of 8-foot tall retaining walls. The walls have been designed to create varying curvilinear contours, which breaks up the appearance of a manufactured slope. This is also in accordance with the Southwest Truckee Meadows Area Plan, which requires grading to complement the original contours of the landscape and minimize disruption of the natural topography. The grading has been visually minimized through extensive landscaping that includes over  $\pm 5.6$  acres of landscape area. A total of 566 trees and 2,492 shrubs have been included in the landscape design, which will help to screen the grading from public view on the adjacent roadways and neighboring properties.

# 4. Issuance Not Detrimental – Issuance of the permit will not be significantly detrimental to the public health, safety or welfare; injurious to the property or improvements of adjacent properties; or detrimental to the character of the surrounding area;

Issuance of the permit will not be significantly detrimental to the public health, safety or welfare of the surrounding area. Consideration has been given to the neighboring property through the overall site design. This includes placement of the buildings and play fields away from the residential properties. The extensive landscaping will help to mitigate the grading impacts and screen the development from public view.

# **5.** Effect on a Military Installation – Issuance of the permit will not have a **detrimental effect on the location, purpose or mission of the military installation.** The proposed project has no effect on the location, purpose or mission of military installation. There are no military installations in the area.





Figure 7 - Zoning Map



# Washoe County Development Application

Your entire application is a public record. If you have a concern about releasing personal information, please contact Planning and Building staff at 775.328.6100.

| Project Information                         | S                       | Staff Assigned Case No.:        |                 |  |  |  |
|---|-------------------------|---------------------------------|-----------------|--|--|--|
| Project Name:                               |                         |                                 |                 |  |  |  |
| Project<br>Description:                     |                         |                                 |                 |  |  |  |
| Project Address:                            |                         |                                 |                 |  |  |  |
| Project Area (acres or square fe            | et):                    |                                 |                 |  |  |  |
| Project Location (with point of re          | eference to major cross | s streets AND area locator):    | -               |  |  |  |
| Assessor's Parcel No.(s):                   | Parcel Acreage:         | Assessor's Parcel No.(s):       | Parcel Acreage: |  |  |  |
|   |                         |                                 |                 |  |  |  |
|   |                         |                                 |                 |  |  |  |
| Section(s)/Township/Range:                  |                         |                                 |                 |  |  |  |
| Indicate any previous Washe<br>Case No.(s). | be County approval      | s associated with this applicat | ion:            |  |  |  |
| Applicant Inf                               | ormation (attach        | additional sheets if necess     | ary)            |  |  |  |
| Property Owner:                             |                         | Professional Consultant:        |                 |  |  |  |
| Name:                                       |                         | Name:                           |                 |  |  |  |
| Address:                                    |                         | Address:                        |                 |  |  |  |
|   | Zip:                    |                                 | Zip:            |  |  |  |
| Phone:                                      | Fax:                    | Phone:                          | Fax:            |  |  |  |
| Email:                                      |                         | Email:                          |                 |  |  |  |
| Cell:                                       | Other:                  | Cell:                           | Other:          |  |  |  |
| Contact Person:                             |                         | Contact Person:                 |                 |  |  |  |
| Applicant/Developer:                        |                         | Other Persons to be Contacted:  |                 |  |  |  |
| Name:                                       |                         | Name:                           |                 |  |  |  |
| Address:                                    |                         | Address:                        |                 |  |  |  |
|   | Zip:                    |                                 | Zip:            |  |  |  |
| Phone:                                      | Fax:                    | Phone:                          | Fax:            |  |  |  |
| Email:                                      |                         | Email:                          |                 |  |  |  |
| Cell:                                       | Other:                  | Cell:                           | Other:          |  |  |  |
| Contact Person:                             |                         | Contact Person:                 |                 |  |  |  |
|   | For Office              | e Use Only                      |                 |  |  |  |
| Date Received:                              | Initial:                | Planning Area:                  |                 |  |  |  |
| County Commission District:                 |                         | Master Plan Designation(s):     |                 |  |  |  |
| CAB(s):                                     |                         | Regulatory Zoning(s):           |                 |  |  |  |

# Refer to attached documentation for owner affidavit information from the U.S. Forest Service.

# Applicant Name: \_\_\_\_\_

The receipt of this application at the time of submittal does not guarantee the application complies with all requirements of the Washoe County Development Code, the Washoe County Master Plan or the applicable area plan, the applicable regulatory zoning, or that the application is deemed complete and will be processed.

STATE OF NEVADA

COUNTY OF WASHOE

)

)

Ι,

(please print name)

being duly sworn, depose and say that I am the owner\* of the property or properties involved in this application as listed below and that the foregoing statements and answers herein contained and the information herewith submitted are in all respects complete, true, and correct to the best of my knowledge and belief. I understand that no assurance or guarantee can be given by members of Planning and Building.

#### (A separate Affidavit must be provided by each property owner named in the title report.)

Assessor Parcel Number(s):

Printed Name\_\_\_\_\_

Signed

Address\_\_\_\_\_

Subscribed and sworn to before me this \_\_\_\_\_ day of \_\_\_\_\_, \_\_\_\_,

(Notary Stamp)

Notary Public in and for said county and state

My commission expires:\_\_\_\_\_

\*Owner refers to the following: (Please mark appropriate box.)

- Owner
- Corporate Officer/Partner (Provide copy of record document indicating authority to sign.)
- Dever of Attorney (Provide copy of Power of Attorney.)
- Owner Agent (Provide notarized letter from property owner giving legal authority to agent.)
- D Property Agent (Provide copy of record document indicating authority to sign.)
- □ Letter from Government Agency with Stewardship

USDA Forest Service

# fully executed Copy 28 NOV.2017

OMB 0596-0217 FS-1500-19

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| 6. NAME/ADDRESS<br>4, county):<br>Mike Boster       | OF RECIPIENT/COOPERATOR (sneet, di  | ty, state, and zip 1              | 7, RECIPIENT/COOPERATOR'S .<br>payment use only):  | HIS SUB ACCOUNT                                 | 'NUMDER (For 1111S                 |
| 14101 Old Vir                                       | ginia Road. Reno NV. 89521  |                                   |  |   |                                    |
| 775-789-3810  | mboster@washoeschools   | .riet.                            |  |   |                                    |
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Jumboldt-Toiyabe National Forest

on Ranger District C. 1536 South Carson Street Carson City, NV 89701 775-882-2766

File Code: 5570 Date:

May 11, 2017

Michael S. Boster Washoe County School District, School Planner 14101 Old Virginia Road Reno, NV 89521

Dear Mr. Boster,

Enclosed is my response to your ELGA application that I received April 27. 2017.

Sincerely,

mand W.The

**IRENE DAVIDSON District Ranger** 



ü



Mr. Michael S. Boster, Washoe County School District Capital Projects 14101 Old Virginia Road Reno, NV. 89521

Dear Mr. Boster:

This letter Acknowledges receipt of your recent Educational Land Grant Act Application (ELGA) to acquire 60 acres of National Forest System (NFS) lands within the Humboldt Toiyabe National Forest.

I received your application on May 27, 2017 and have reviewed it to ensure we have a complete application package.

Following is a list of minimum requirements necessary for an application package to be complete.

- 1. A legal description of the land requested, including appropriate maps that accurately depict the area requested.
- 2. A statement that the conveyed NFS land will be used for a public or publicly funded elementary or secondary school to provide grounds or facilities for that school.
- 3. Documentation that the total acreage requested is the minimum amount necessary for the intended purpose.
- 4. Documentation of other alternatives considered, such as, private, local governmental, or State lands, and the reason(s) why they cannot accommodate this need.
- 5. Documentation that the conveyed land is within the applicant school district and contiguous to an existing school. If either of these two conditions is not met, the applicant school district must:

a. Demonstrate the objective educational benefit which will be served by the conveyance, and b. Provide documentation on how access to the conveyed land will be obtained.

6. Documentation that the school district is financially capable of completing the proposed project.

7. Reasons why the applicant school district feels that the conveyance of NFS lands for educational purposes outweigh the public objectives and values that would be served by keeping the land in the National Forest System.

8. A development plan that describes the proposed public educational use of the conveyed land, the type(s) of facilities that will be constructed and their location, proposed access, utility routes, environmental controls during construction, and estimated construction times. The development plan must be included in the environmental assessment of the proposed conveyance.

After reviewing your application and weighing it against these criteria I have determined it to be complete as it meets the minimum requirements set forth in the Forest Service Handbook (FSH) 5509.11 Chapter 30 sec 34.12 – Application Content. If a decision on the proposal is not made within 120 days, the Forest Service will provide a written explanation as to why a decision has not been made and give a revised timeframe for a final decision on the project.

The next steps will be to set up a cooperative agreement between Washoe County School District and the Forest Service for payment of cost incurred as a result of this project. You will be required to pay a nominal fee of \$10 per acre conveyed, plus all Forest Service costs directly associated with the project, that the Forest Service may incur to evaluate and process the application, including costs associated with National Environmental Policy Act (NEPA) compliance, document preparation, surveys, posting of property monuments, markers, or posts, and so forth. Costs incurred by the Forest Service in the evaluation and processing of an ELGA application are payable by the school district regardless of whether or not the conveyance is approved.

It is my understanding the school district will contract with a third party to perform the necessary NEPA studies and the Forest Service will provide the criteria to be used including what documents or reports will be needed. The Forest Service will review the documents produced by the school districts contractors and approve them when they are sufficient for the Forest Service to make a decision on.

As a part of the public notice and comment the Forest Service requires a name of the proposed school, I understand there is a process required by the school district for this but it is imperative we meet this requirement please take whatever actions are necessary to ensure we can meet this legal requirement.

A couple of other item to be aware of are a reversionary interest in the United States will be retained in all land conveyed under this law which will vest if a school district attempts to convey the land to another party or where the lands are devoted to another use different from the use for which the land is conveyed by the United States, and a conveyance under this law may not convey mineral or water rights. If necessary, the exact acreage and legal description shall be determined by a survey satisfactory to the Secretary at the expense of the applicant.

Sincerely. achand Witter

Irené Davidson DISTRICT RANGER





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#### Text of Ad:

#### 02/07/2018

**Text of Ad:** 02/07/2018 The Carson Ranger District of the Humboldt-Toiyabe National Forest (Forest) is initiating a comment period for a preliminary Environmental Assessment (EA) for the proposed Education Land Grant Act Transfer of National Forest System Land to Washee County School District #32446, displaying the proposed action and potential effects of the project. The Forest is proposing to convey approximately 60 acres of National Forest System land in accordance with the Educational Land Grant Act (ELGA) to the Washee County School District (WCSD). If conveyed, the WCSD would use the land to develop a school campus. The Project Area is located in southwest Reno, Washoe County at the intersection of Thomas Creek Road and Arrowcreek Parkway. Specifically, the project area is located at T. R N., R. 19 E., Sec. 24 S V/SE V/AExcepting that portion conveyed by U.S. Patent No. 27-96-0035. The preliminary EA, including the proposed action, is available for review and can be downloaded from the Humboldt-Toiyabe National Forest website at https://www.fs.usda.gov/project/?project=52446. To obtain a hard copy of the preliminary EA and proposed action please contact I rene Davidson, Carson District Ranger, Humboldt-Toiyabe National Forest, f.efd.us. This preliminary EA is subject to comment pursuant to 36 CFR 218, Subparts A and B. Only those who submit timely project-specific written comments during a public comment period are eligible to file an objection. Furthermore, issues raised in objection, nuless the issue is based on new information that arose after the opportunities for comments must sign the comments or verify identity upon reaust.

ting comments must sign the comments or verify identity upon re-quest. **HOW TO COMMENT AND TIMEFRAMES** The Forest Service will accept comments on the preliminary EA for 30 days following publication of the opportunity to comment legal no-tice in the Reno- Gazette Journal, which is the exclusive means for calculating the comment period. Commenters should not rely upon dates or timeframe information provided by any other source. It is the commenter's responsibility to ensure timely receipt of comments (36 CFR 218.25). Please submit your comments on the proposal website, at https://cara ecosystem-managment.org/Public/CommentInput?project=52446. If your computer is not compatible with the website, try using a differ-ent web browser, or you can email comments to: comments-intermin -humboldt-toiyabe-carson@fs.fed.us. All formal comments on the EA must be submitted in writing. In cases where no identifiable name is attached to a comment, a veri-fication of identity will be required for appeal eligibility. If using an electronic message, a scanned signature is one way to provide verifi-cation. It is the responsibility of persons providing comments to sub-mit them by the close of the comment period. Names of commenters will be part of the public record subject to the Freedom of informa-tion Act. The Errest Service will host an open house forum meeting to discuss

will be part of the public record subject to the Freedom of Informa-tion Act. The Forest Service will host an open house forum meeting to be made. The meeting will be held at 6:30 pm on February 22, 2018 at the South Valley Library Diamond Room, located at 15650-A Wedge Parkway, Reno, NV 89511. The meeting will include a brief introduction of the proposed land conveyance by the Forest Service followed by an op-portunity to discuss with Forest staff members. WILLIAM A. DUNKELBERGER Forest Supervisor

No. 2718638

Feb. 9, 2018

#### Public Notices

#### **Public Notices**

The Carson Ranger District of the Humboldt-Toiyabe National Forest (Forest) is initiating a comment period for a preliminary Environ-mental Assessment (EA) for the proposed Education Land Grant Act Transfer of National Forest System Land to Washee County School District #52446, displaying the proposed action and potential effects of the project. The Forest is proposing to convey approximately 60 acres of National Forest System land in accordance with the Educational Land Grant Act (ELGA) to the Washee County School District (WCSD). If conveyed, the WCSD would use the land to development of a school campus. The Project Area is located in southwest Reno, Washee County at the intersection of Thomas Creek Road and Arrowcreek Parkway. Specifically, the project area is located at T. 18 N., R. 19 E., Sec. 24 S VSE Vazcepting that portion conveyed by U.S. Patent No. 27-96-0035. The preliminary EA, including the proposed action, is available for review and can be downloaded from the Humboldt-Toiyabe National Forest website at https://www.fs.usda.gov/project/?project=52446. To obtain a hard copy of the preliminary EA and proposed action please contact David Drake, Land Use Specialist. Humboldt-Toiyabe National Forest, 1200 Franklin Way, Sparks, NV, 89431, 775 332-1241 d drake@fs.fed.us.

drake@fs.fed.us.

drake@fs.fed.us. This preliminary EA is subject to comment pursuant to 36 CFR 218, Subparts A and B. Only those who submit timely project-specific written comments during a public comment period are eligible to file an objection. Furthermore, issues raised in objections must be based on previously submitted specific written comments regarding the pro-posed conveyance or activity and attributed to the objector, unless the issue is based on new information that arose after the opportuni-ties for comment. Individuals or representatives of an entity submit-ting comments must sign the comments or verify identity upon re-quest.

ting comments must sign the continents of Year and the augest. HOW TO COMMENT AND TIMEFRAMES The Forest Service will accept comments on the preliminary EA for 30 days following publication of the opportunity to comment legal no-tice in the Reno- Gazette Journal, which is the exclusive means for calculating the comment period. Commenters should not rely upon dates or timeframe information provided by any other source. It is the commenter's responsibility to ensure timely receipt of comments (36 CFR 218.25). Please submit your comments on the proposal website, at https://cara .ecosystem-management.org/Public//ReadingRoom?Project=52446. If your computer is not compatible with the website, try using a dif-ferent web browser. All formal comments on the EA must be submit-ted in writing.

refer web prowser. An formal comments of the EA must be subtribu-ted in writing. In cases where no identifiable name is attached to a comment, a veri-fication of identity will be required for appeal eligibility. If using an electronic message, a scanned signature is one way to provide verifi-cation. It is the responsibility of persons providing comments to sub-mit them by the close of the comment period. Names of commenters will be part of the public record subject to the Freedom of Informa-tion Act

will be part of the public record subject to the Freedom of Informa-tion Act. The Forest Service will host an open house forum meeting to discuss the preliminary EA and the Forest Service's decision to be made. The meeting will be held at 6:30 pm on February 22, 2018 at the South Valley Library Diamond Room, located at 15650-A Wedge Parkway, Reno, NV 89511. The meeting will include a brief introduction of the proposed land conveyance by the Forest Service followed by an opportunity to dis-cuss with Forest staff members. WILLIAM A. DUNKELBERGER Forest Supervisor

No. 2733449

Feb. 15, 2018

\* ALL TRANSACTIONS CONSIDERED PAID IN FULL UPON CLEARANCE OF FINANCIAL INSTITUTION

# 2nd notice 15 Feb RGJ Lega The Mason Valley News Notice

"The Only Newspaper in the World that Gives a Damn About Yerington"

#### Order Confirmation for Ad #: 0002733449

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# Special Use Permit Application for Grading Supplemental Information

(All required information may be separately attached)

Chapter 110 of the Washoe County Code is commonly known as the Development Code. Specific references to special use permits may be found in Article 810, Special Use Permits. Article 438, Grading, and Article 418, Significant Hydrologic Resources, are the ordinances specifically involved in this request.

1. What is the purpose of the grading?

- 2. How many cubic yards of material are you proposing to excavate on site?
- 3. How many square feet of surface of the property are you disturbing?
- 4. How many cubic yards of material are you exporting or importing? If none, how are you managing to balance the work on-site?

5. Is it possible to develop your property without surpassing the grading thresholds requiring a Special Use Permit? (Explain fully your answer.)

6. Has any portion of the grading shown on the plan been done previously? (If yes, explain the circumstances, the year the work was done, and who completed the work.)

7. Have you shown all areas on your site plan that are proposed to be disturbed by grading? (If no, explain fully your answer.)

8. Can the disturbed area be seen from off-site? If yes, from which directions, and which properties or roadways?

9. Could neighboring properties also be served by the proposed access/grading requested (i.e. if you are creating a driveway, would it be used for access to additional neighboring properties)?

10. What is the slope (Horizontal/Vertical) of the cut and fill areas proposed to be? What methods will be used to prevent erosion until the revegetation is established?

11. Are you planning any berms?

| Yes | 🛛 No | If yes, how tall is the berm at its highest? |
|-----|------|--|
|     |      |  |

12. If your property slopes and you are leveling a pad for a building, are retaining walls going to be required? If so, how high will the walls be and what is their construction (i.e. rockery, concrete, timber, manufactured block)?

13. What are you proposing for visual mitigation of the work?

14. Will the grading proposed require removal of any trees? If so, what species, how many and of what size?

15. What type of revegetation seed mix are you planning to use and how many pounds per acre do you intend to broadcast? Will you use mulch and, if so, what type?

16. How are you providing temporary irrigation to the disturbed area?

17. Have you reviewed the revegetation plan with the Washoe Storey Conservation District? If yes, have you incorporated their suggestions?

18. Are there any restrictive covenants, recorded conditions, or deed restrictions (CC&Rs) that may prohibit the requested grading?

| Yes | 🗆 No | If yes, please attach a copy. |
|-----|------|-------------------------------|
|     |      |                               |

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| 2017   | \$0.00  | \$0.00   | \$0.00  | \$0.00                           | \$0.00                                     |   |
| 2016   | \$0.00  | \$0.00   | \$0.00  | \$0.00                           | \$0.00                                     | Special Assessment  |
| 2015   | \$0.00  | \$0.00   | \$0.00  | \$0.00                           | \$0.00                                     | District  |
| 2014   | \$0.00  | \$0.00   | \$0.00  | \$0.00                           | \$0.00                                     | Installment Date  |
| 2013   | \$0.00  | \$0.00   | \$0.00  | \$0.00                           | \$0.00                                     | Information   |
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NOTE: This map is prepared for the use of the Washoe County Assessor for assessment and illustrative purposes only, it does not represent survey of the premises. No liability is assumed as to the sufficiency or accuracy of the data delineated hereon.

Appendix A - 19

# PRELIMINARY HYDROLOGIC AND HYDRAULIC ANALYSIS REPORT

# FOR

# **ARROW CREEK MIDDLE SCHOOL**

Prepared for

Washoe County School District Capital Projects and Facilities Management 14101 Old Virginia Road Reno, Nevada 89521-8912

Prepared by



Odyssey Engineering Incorporated 895 Roberta Lane, Suite 104 Reno, Nevada 89431 (775) 359-3303

April 2018



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| 5 | CONCLUSIONS                      | 5 |

# APPENDICES

 $\begin{array}{l} \mbox{Appendix A-Supporting Data} \\ \mbox{Appendix B-Existing Hydrologic Analysis} \\ \mbox{Appendix C-Proposed Hydrologic Analysis} \\ \mbox{Appendix D-Detention Facility Analysis} \\ \end{array}$ 

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FIGURE 2– PROPOSED HYDROLOGY DISPLAY

#### 1 <u>INTRODUCTION</u>

The following report represents the preliminary hydrologic and hydraulic analysis for the Washoe County School District - Arrow Creek Middle School Project (ACMS Project) which is located east of Crossbow Court, West of Thomas Creek Road, and north of Arrow Creek Parkway. The Washoe County School District - Arrow Creek Middle School is a proposed development that includes buildings, athletic facilities, landscaping, and parking.

The ACMS Project is located within the Southwestern 1/4 of Section 24, Township 18 North, and Range 19 East. The site is undeveloped ground with native shrubs and grasses. Site topography consists of slopes from south – southeast to west – northwest, ranging from 0% to 6%. Rainfall runoff from the site flows in a north-northwesterly direction towards an existing ephemeral drainage channels towards an existing housing development and civil improvements.

According to the Federal Emergency Management Agency (FEMA) Flood Insurance Rate Map (FIRM) Community-Panel Number 32031C3245G dated March 16, 2009, the subject property is in Zone X. Zone X is an area determined to be outside the 100-year floodplain. Reference FEMA panel in Appendix A.

The purpose of this preliminary report is to analyze the existing and proposed conditions of the subject property based on the 5-year and 100-year peak flow events. The report contains the following sections: (1) Methodology, (2) Existing Hydrology, (3) Proposed Hydrology, and (4) Conclusion.

#### 2 <u>METHODOLOGY</u>

#### Hydrologic Method

Hydrologic analyses were performed to determine the peak discharge for the 5-year and 100-year peak flow events. *AutoDesk Storm and Sanitary Analysis (SSA)* was used to perform a *Rational Method* analysis to model the hydrologic basins that contribute in the existing and proposed conditions.

#### 2.1 RATIONAL ANALYSIS METHOD

The on-site analysis was performed using the Rational Method. Rational Method peak flows were used to design the storm drain facilities for the proposed project. The hydrology was determined using the Truckee Meadows Regional Drainage Manual (TMRDM) and the Rational Method (Appendix A). The parameters for the Rational Method of analysis are:

- 1. The Drainage Area
- 2. Time of Concentration
- 3. Runoff Coefficient
- 4. Rainfall Intensity

The runoff coefficients were obtained from the TMRDM (Reference Appendix A). The resulting "Rational Method" developed flows determined from the above information was used to determine the proposed storm drain facilities. The rainfall characteristics were modeled using the NOAA database to determine site specific depth of precipitation (Appendix A).

#### 2.2 HYDRAULIC ANALYSIS METHODS

Hydraulic analyses were performed using the associated hydrologic data to provide the estimates of the elevation of floods for the selected recurrence intervals. Water-surface elevations were computed in SSA using hydrodynamic routing.

#### 3 EXISTING CONDITIONS HYDROLOGY

For the existing catchments, a time of concentration ( $T_c$ ) and Rational Method coefficient were selected, based on the Rational Method (Appendix A), taking into consideration the catchment characteristics, which include catchment area, slope and length of the longest channel, watershed boundaries, urbanization, and land cover. Table 1 and Figure 2 summarize the characteristics of the on-site catchment area. Reference Appendix B for the complete Rational Method analysis. Reference Figure 1 (Existing Hydrology) in the map pocket for existing hydrological drainage and the associated 5-year and 100-year peak flow events.

| Sub-Basin | Area  | Rational    | Time of       | Rainfall Intensity                                  | 5-Year     | 100-Year   |
|-----------|-------|-------------|---------------|---|------------|------------|
|           | (Ac.) | Method      | Concentration | ( <b>I</b> <sub>5</sub> / <b>I</b> <sub>100</sub> ) | Peak Flows | Peak Flows |
|           |       | Coefficient | (min)         | (in/hr)   | (cfs)      | (cfs)      |
|           |       | (C5/C100)   |               |   |            |            |
| X-01      | 11.83 | 0.31/0.57   | 17.39         | 1.18/2.85   | 4.3        | 19.2       |
| X-02      | 9.81  | 0.26/0.54   | 16.50         | 1.21/2.93   | 3.1        | 15.5       |
| X-03      | 13.98 | 0.24/0.52   | 28.84         | 0.88/2.13   | 3.0        | 15.5       |
| X-04      | 16.16 | 0.20/0.50   | 17.76         | 1.16/2.81   | 3.8        | 22.7       |
| X-05      | 22.98 | 0.20/0.50   | 13.49         | 1.35/3.26   | 6.2        | 37.4       |
| X-06      | 13.52 | 0.20/0.50   | 14.15         | 1.32/3.19   | 3.6        | 21.5       |
| X-07      | 10.62 | 0.20/0.50   | 13.92         | 1.33/3.21   | 2.8        | 17.0       |
| TOTAL     | 98.90 |             |               |   | 26.8       | 148.8      |

Table 1 – Existing Conditions Rational Method Model Summary for the ACMS Project, Reno, NV.

The 5-year and 100-year peak flows from off-site catchments (X-01 through X-03) in the existing condition are 10.4 cfs and 50.2 cfs, respectively. The 5-year and 100-year peak flows from on-site catchment (X-04 through X-07) in the existing condition are 16.4 cfs and 98.60 cfs, respectively. Therefore, the total existing flows are 26.8 cfs and 148.8 cfs in the 5-year and 100-year peak flow events, respectively (Appendix B). The flows are discharged towards the existing ephemeral drainages and existing civil improvements.

#### 4 PROPOSED CONDITIONS HYDROLOGY AND HYDRAULICS

#### 4.1 **PROPOSED HYDROLOGY**

*SSA*, the hydrologic modeling software has the capacity to route the flows and analyze the attenuation throughout the system. The proposed ACMS project has discharge values of the proposed sub-basins (Table 2) which will be directed through the proposed storm drain system, existing ephemeral drainages, and the proposed detention facility.

There are six on-site proposed development sub-basins with the ACMS development area (Figures 2). The sub-areas took into account the proposed on- and off-site flows that affect the site. The calculated 5-year and 100-year peak flows can be found in Table 2. Weighted run-off coefficients were calculated for each basin (Table 2). Routing was used to determine the intensities for the off- and on-site sub-basins, the proposed storm drain systems will route and attenuate flows to the ACMS Project points of discharge (Figure 2). Figure 2 provides a comparative 100-year peak flow discharge analysis for the existing areas that currently have peak runoff. Refer to Appendix C, Hydrologic Analysis for all data and supporting calculations using the Rational Method. Reference Table 2 below for a summary of the proposed drainage conditions.

| Sub-Basin | Area  | Rational          | Time of       | Rainfall Intensity                  | 5-Year     | 100-Year   |
|-----------|-------|-------------------|---------------|-------------------------------------|------------|------------|
|           | (Ac.) | Method            | Concentration | (I <sub>5</sub> /I <sub>100</sub> ) | Peak Flows | Peak Flows |
|           |       | Coefficient       | (min)         | (in/hr)                             | (cfs)      | (cfs)      |
|           |       | $(C_{5}/C_{100})$ |               |                                     |            |            |
| P-01      | 11.83 | 0.31/0.57         | 17.39         | 1.18/2.85                           | 4.3        | 19.2       |
| P-02      | 9.81  | 0.26/0.54         | 16.50         | 1.21/2.93                           | 3.1        | 15.5       |
| P-03      | 13.98 | 0.24/0.52         | 28.84         | 0.88/2.13                           | 3.0        | 15.5       |
| P-04      | 7.11  | 0.20/0.50         | 12.03         | 1.42/3.43                           | 2.0        | 12.2       |
| P-05      | 9.19  | 0.20/0.50         | 10.00         | 1.55/3.74                           | 2.8        | 17.2       |
| P-06      | 26.94 | 0.55/0.72         | 13.41         | 1.35/3.27                           | 20.0       | 63.3       |
| P-07      | 10.01 | 0.20/0.50         | 13.24         | 1.36/3.28                           | 2.7        | 16.4       |
| P-08      | 2.28  | 0.20/0.50         | 10.00         | 1.55/3.74                           | 0.7        | 4.3        |
| P-09      | 7.75  | 0.20/0.50         | 11.99         | 1.42/3.44                           | 2.2        | 13.3       |
| TOTAL     | 98.90 |                   |               |                                     | 26.8       | 148.8      |

Table 2 – Proposed Rational Method Model Summary for the ACMS Project, Reno, NV.

#### 4.2 **DETENTION**

The proposed detention facility is approximately eight-feet deep with an overall detention capacity of 1.26 ac-ft. During 100-year peak flow event, the proposed detention facility will have a 100-year peak flow of 37.2 cfs and will discharge 20.0 cfs with a freeboard of 1.97 feet.

### 5 <u>CONCLUSIONS</u>

The proposed improvements and the analyses presented herein are in accordance with drainage regulations presented in Chapter II – Storm Drainage, in conjunction with the *Truckee Meadows Regional Drainage Manual (TMRDM, April 30, 2009)*.

This analysis is a preliminary analysis to provide an overview of the proposed development, a comprehensive hydrologic and hydraulic analysis will be completed once the civil improvements have been completed. However, this preliminary analysis determined that the proposed project improvements, roadways, and storm water conveyance facilities, once constructed, will not adversely impact upstream or downstream properties adjacent to this site. Actually, the proposed improvements will decrease overall flows towards the existing residential development. As seen of Figure 2, the three critical discharge sites will all experience decreased runoff from the associated peak flow events.

# APPENDIX A Supporting Data



NOAA Atlas 14, Volume 1, Version 5 Location name: Reno, Nevada, USA\* Latitude: 39.4068°, Longitude: -119.7959° Elevation: 5136.61 ft\*\* \* source: ESRI Maps \*\* source: USGS



#### POINT PRECIPITATION FREQUENCY ESTIMATES

Sanja Perica, Sarah Dietz, Sarah Heim, Lillian Hiner, Kazungu Maitaria, Deborah Martin, Sandra Pavlovic, Ishani Roy, Carl Trypaluk, Dale Unruh, Fenglin Yan, Michael Yekta, Tan Zhao, Geoffrey Bonnin, Daniel Brewer, Li-Chuan Chen, Tye Parzybok, John Yarchoan

NOAA, National Weather Service, Silver Spring, Maryland

PF\_tabular | PF\_graphical | Maps\_& aerials

#### **PF** tabular

| PDS-based point precipitation frequency estimates with 90% confidence intervals (in inches/hour) <sup>1</sup> |               |               |               |               |               |                |               |               |               |               |
|---|---------------|---------------|---------------|---------------|---------------|----------------|---------------|---------------|---------------|---------------|
| Duration  |               |               |               | Avera         | ige recurren  | ce interval (y | years)        |               |               |               |
| Duration  | 1             | 2             | 5             | 10            | 25            | 50             | 100           | 200           | 500           | 1000          |
| 5-min   | <b>1.21</b>   | <b>1.52</b>   | <b>2.03</b>   | <b>2.52</b>   | <b>3.31</b>   | <b>4.04</b>    | <b>4.92</b>   | <b>5.98</b>   | <b>7.68</b>   | <b>9.24</b>   |
|   | (1.04–1.43)   | (1.30–1.79)   | (1.73-2.40)   | (2.12–2.98)   | (2.72–3.96)   | (3.22-4.90)    | (3.78–6.04)   | (4.40-7.50)   | (5.30-9.92)   | (6.08–12.2)   |
| 10-min  | <b>0.924</b>  | <b>1.16</b>   | <b>1.55</b>   | <b>1.91</b>   | <b>2.53</b>   | <b>3.08</b>    | <b>3.74</b>   | <b>4.55</b>   | <b>5.84</b>   | <b>7.03</b>   |
|   | (0.792-1.09)  | (0.990-1.36)  | (1.31–1.83)   | (1.62–2.27)   | (2.08–3.01)   | (2.45-3.73)    | (2.87–4.59)   | (3.35-5.70)   | (4.04-7.56)   | (4.63–9.31)   |
| 15-min  | <b>0.764</b>  | <b>0.956</b>  | <b>1.28</b>   | <b>1.58</b>   | <b>2.09</b>   | <b>2.55</b>    | <b>3.10</b>   | <b>3.76</b>   | <b>4.83</b>   | <b>5.81</b>   |
|   | (0.656-0.900) | (0.820-1.13)  | (1.09–1.51)   | (1.34–1.87)   | (1.72-2.49)   | (2.03–3.08)    | (2.38–3.79)   | (2.77-4.71)   | (3.34–6.24)   | (3.82-7.69)   |
| 30-min  | <b>0.516</b>  | <b>0.644</b>  | <b>0.860</b>  | <b>1.07</b>   | <b>1.40</b>   | <b>1.72</b>    | <b>2.08</b>   | <b>2.53</b>   | <b>3.25</b>   | <b>3.91</b>   |
|   | (0.442-0.606) | (0.552-0.760) | (0.732-1.02)  | (0.900-1.26)  | (1.15–1.68)   | (1.37-2.07)    | (1.60-2.55)   | (1.86-3.17)   | (2.25-4.21)   | (2.58–5.18)   |
| 60-min  | <b>0.319</b>  | <b>0.398</b>  | <b>0.533</b>  | <b>0.659</b>  | <b>0.869</b>  | <b>1.06</b>    | <b>1.29</b>   | <b>1.57</b>   | <b>2.01</b>   | <b>2.42</b>   |
|   | (0.274-0.375) | (0.341-0.470) | (0.453-0.630) | (0.557-0.781) | (0.715-1.04)  | (0.845-1.28)   | (0.991–1.58)  | (1.15-1.96)   | (1.39–2.60)   | (1.59–3.20)   |
| 2-hr  | <b>0.212</b>  | <b>0.264</b>  | <b>0.336</b>  | <b>0.400</b>  | <b>0.496</b>  | <b>0.581</b>   | <b>0.679</b>  | <b>0.804</b>  | <b>1.03</b>   | <b>1.23</b>   |
|   | (0.187-0.243) | (0.233-0.302) | (0.294–0.386) | (0.345-0.459) | (0.416-0.572) | (0.476-0.681)  | (0.542-0.809) | (0.620-0.992) | (0.756-1.31)  | (0.874–1.62)  |
| 3-hr  | <b>0.170</b>  | <b>0.212</b>  | <b>0.265</b>  | <b>0.308</b>  | <b>0.367</b>  | <b>0.419</b>   | <b>0.478</b>  | <b>0.560</b>  | <b>0.703</b>  | <b>0.835</b>  |
|   | (0.152–0.192) | (0.191–0.241) | (0.236-0.300) | (0.272-0.349) | (0.319-0.419) | (0.357-0.483)  | (0.400-0.559) | (0.458-0.667) | (0.558–0.884) | (0.645-1.09)  |
| 6-hr  | <b>0.121</b>  | <b>0.152</b>  | <b>0.187</b>  | <b>0.215</b>  | <b>0.251</b>  | <b>0.278</b>   | <b>0.305</b>  | <b>0.338</b>  | <b>0.389</b>  | <b>0.442</b>  |
|   | (0.108-0.136) | (0.135-0.171) | (0.166-0.211) | (0.190-0.243) | (0.218-0.285) | (0.239-0.318)  | (0.258-0.353) | (0.280-0.396) | (0.315-0.465) | (0.351-0.551) |
| 12-hr   | <b>0.080</b>  | <b>0.101</b>  | <b>0.126</b>  | <b>0.146</b>  | <b>0.173</b>  | <b>0.193</b>   | <b>0.214</b>  | <b>0.235</b>  | <b>0.262</b>  | <b>0.284</b>  |
|   | (0.071-0.090) | (0.090–0.113) | (0.112-0.142) | (0.129–0.165) | (0.151–0.197) | (0.166-0.221)  | (0.181-0.248) | (0.194-0.276) | (0.211-0.314) | (0.224–0.347) |
| 24-hr   | <b>0.052</b>  | <b>0.066</b>  | <b>0.083</b>  | <b>0.097</b>  | <b>0.117</b>  | <b>0.133</b>   | <b>0.149</b>  | <b>0.166</b>  | <b>0.190</b>  | <b>0.209</b>  |
|   | (0.047-0.059) | (0.059-0.074) | (0.075-0.094) | (0.087–0.110) | (0.104–0.132) | (0.117-0.150)  | (0.130-0.170) | (0.143-0.191) | (0.161–0.220) | (0.174-0.245) |
| 2-day   | <b>0.031</b>  | <b>0.039</b>  | <b>0.050</b>  | <b>0.059</b>  | <b>0.072</b>  | <b>0.082</b>   | <b>0.092</b>  | <b>0.103</b>  | <b>0.119</b>  | <b>0.131</b>  |
|   | (0.028-0.036) | (0.035-0.045) | (0.045-0.058) | (0.052-0.068) | (0.062-0.082) | (0.071-0.094)  | (0.079-0.107) | (0.087-0.122) | (0.098-0.142) | (0.106–0.159) |
| 3-day   | <b>0.023</b>  | <b>0.029</b>  | <b>0.038</b>  | <b>0.045</b>  | <b>0.055</b>  | <b>0.063</b>   | <b>0.072</b>  | <b>0.081</b>  | <b>0.095</b>  | <b>0.105</b>  |
|   | (0.021-0.026) | (0.026-0.033) | (0.034-0.043) | (0.040-0.051) | (0.048-0.063) | (0.055-0.072)  | (0.062-0.083) | (0.069-0.095) | (0.078-0.111) | (0.085–0.126) |
| 4-day   | <b>0.019</b>  | <b>0.024</b>  | <b>0.032</b>  | <b>0.038</b>  | <b>0.047</b>  | <b>0.054</b>   | <b>0.062</b>  | <b>0.070</b>  | <b>0.082</b>  | <b>0.092</b>  |
|   | (0.017-0.022) | (0.022-0.028) | (0.028-0.036) | (0.034-0.043) | (0.041-0.053) | (0.047-0.062)  | (0.053-0.071) | (0.059-0.081) | (0.068-0.096) | (0.075-0.109) |
| 7-day   | <b>0.013</b>  | <b>0.017</b>  | <b>0.022</b>  | <b>0.026</b>  | <b>0.032</b>  | <b>0.037</b>   | <b>0.042</b>  | <b>0.048</b>  | <b>0.056</b>  | <b>0.062</b>  |
|   | (0.011-0.015) | (0.015–0.019) | (0.019-0.025) | (0.023-0.030) | (0.028-0.037) | (0.032-0.043)  | (0.036-0.049) | (0.040-0.056) | (0.046-0.066) | (0.051-0.074) |
| 10-day  | <b>0.010</b>  | <b>0.013</b>  | <b>0.017</b>  | <b>0.021</b>  | <b>0.025</b>  | <b>0.029</b>   | <b>0.033</b>  | <b>0.037</b>  | <b>0.043</b>  | <b>0.048</b>  |
|   | (0.009-0.012) | (0.012-0.015) | (0.015-0.020) | (0.018-0.024) | (0.022-0.029) | (0.025-0.033)  | (0.028-0.038) | (0.031-0.043) | (0.036-0.051) | (0.039–0.057) |
| 20-day  | <b>0.006</b>  | <b>0.008</b>  | <b>0.011</b>  | <b>0.013</b>  | <b>0.016</b>  | <b>0.018</b>   | <b>0.020</b>  | <b>0.022</b>  | <b>0.026</b>  | <b>0.028</b>  |
|   | (0.006-0.007) | (0.007–0.009) | (0.010-0.012) | (0.011-0.015) | (0.014-0.018) | (0.016-0.020)  | (0.017-0.023) | (0.019-0.026) | (0.022-0.030) | (0.023-0.033) |
| 30-day  | <b>0.005</b>  | <b>0.007</b>  | <b>0.009</b>  | <b>0.010</b>  | <b>0.012</b>  | <b>0.014</b>   | <b>0.016</b>  | <b>0.018</b>  | <b>0.020</b>  | <b>0.022</b>  |
|   | (0.005-0.006) | (0.006-0.007) | (0.008-0.010) | (0.009-0.012) | (0.011-0.014) | (0.012-0.016)  | (0.014-0.018) | (0.015-0.020) | (0.017-0.023) | (0.018-0.026) |
| 45-day  | <b>0.004</b>  | <b>0.005</b>  | <b>0.007</b>  | <b>0.008</b>  | <b>0.010</b>  | <b>0.011</b>   | <b>0.012</b>  | <b>0.013</b>  | <b>0.015</b>  | <b>0.016</b>  |
|   | (0.004-0.005) | (0.005–0.006) | (0.006-0.008) | (0.007-0.009) | (0.009–0.011) | (0.010-0.012)  | (0.011-0.014) | (0.012-0.015) | (0.013-0.017) | (0.014–0.019) |
| 60-day  | 0.003         | <b>0.005</b>  | 0.006         | <b>0.007</b>  | 0.008         | 0.009          | 0.010         | 0.011         | 0.012         | <b>0.013</b>  |
|   | (0.003-0.004) | (0.004-0.005) | (0.005-0.007) | (0.006-0.008) | (0.007-0.009) | (0.008-0.010)  | (0.009-0.011) | (0.009-0.013) | (0.010-0.014) | (0.011–0.015) |

<sup>1</sup> Precipitation frequency (PF) estimates in this table are based on frequency analysis of partial duration series (PDS).

Numbers in parenthesis are PF estimates at lower and upper bounds of the 90% confidence interval. The probability that precipitation frequency estimates (for a given duration and average recurrence interval) will be greater than the upper bound (or less than the lower bound) is 5%. Estimates at upper bounds are not checked against probable maximum precipitation (PMP) estimates and may be higher than currently valid PMP values. Please refer to NOAA Atlas 14 document for more information.

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Appendix B - 9

# National Flood Hazard Layer FIRMette



# Legend



Engineering Handbook, Section 4 (SCS, 1985). The antecedent moisture condition of the watershed is explained as follows:

The amount of rainfall in a period of 5 to 30 days preceding a particular storm is referred to as antecedent rainfall, and the resulting condition of the watershed in regard to potential runoff is referred to as an antecedent moisture condition. In general, the heavier the antecedent rainfall, the greater the direct runoff that occurs from a given storm. The effects of infiltration and evapotranspiration during the antecedent period are also important, as they may increase or lessen the effect of antecedent rainfall. Because of the difficulties of determining antecedent storm conditions from data normally available, the conditions are reduced to three cases, AMC-I, AMC-II and AMC-III.

#### For the Washoe County area, an AMC-II condition shall be used for determining storm runoff.

Having determined the soil group, land use and treatment class and the antecedent moisture condition, CN values can be determined from Table 702.

There will be areas to which the values in Table 702 do not apply. The percentage of impervious area for the various types of residential areas or the land use condition for the pervious portions may vary from the conditions assumed in Table 702. A curve for each pervious CN can be developed to determine the composite CN for any density of impervious area. Figure 702 has been developed assuming a CN of 98 for the impervious area. The curves in Figure 702 can help in estimating the increase in runoff as more land within a given area is covered with impervious material.

There are a number of methods available for computing the percentage of impervious area in a watershed. Some methods include using U.S. Geological Survey topographic maps, land use maps, aerial photographs, and field reconnaissance. Care must be exercised when using methods based on such parameters as population density, street density, and age of the development as a means of determining the percentage of impervious area. The available data on runoff from urban areas are not yet sufficient to validate widespread use of these methods. Therefore, the CN to be used in the Washoe County area shall be based on Table 702 or Figure 702 in this Manual. A CN computation example is included in Section 711.

## 704 RATIONAL FORMULA METHOD

For drainage basins that are not complex and have small drainage areas, the design storm runoff may be analyzed using the Rational Formula Method in accordance with Section 304.3. This method was introduced in 1889 and is still being used in many engineering offices in the United States. Even though this method has frequently come under academic criticism for its simplicity, no other practical drainage design method has evolved to such a level of general acceptance by practicing engineers. The Rational Formula Method, when properly understood and applied, can produce satisfactory results for determining peak discharge.

#### 704.1 METHODOLOGY

The Rational Formula Method is based on the formula:

$$Q = CIA \tag{708}$$

Q is defined as the maximum rate of runoff in cubic feet per second (actually, Q has units of acre inches per hour, which is approximately equal to the units of cubic feet per second). C is a runoff coefficient and represents the runoff-producing conditions of the subject land area (see Section 704.5).

I is the average intensity of rainfall in inches per hour for a duration equal to the time of concentration. A is the contributing basin area in acres.

#### 704.2 ASSUMPTIONS

The basic assumptions made when applying the Rational Formula Method are as follows:

- 1. The computed maximum rate of runoff to the design point is a function of the average rainfall rate during the time of concentration to that point.
- 2. The maximum rate of rainfall occurs during the time of concentration, and the design rainfall depth during the time of concentration is converted to the average rainfall intensity for the time of concentration.
- 3. The maximum runoff rate occurs when the entire area is contributing flow. However, this assumption has been modified from time to time when local rainfall/runoff data was used to improve calculated results.

#### 704.3 LIMITATIONS ON METHODOLOGY

The Rational Formula Method adequately approximates the peak rate of runoff from a rainstorm in a given basin. The critics of the method usually are unsatisfied with the fact that the answers are only approximations. A shortcoming of the Rational Formula Method is that only one point on the runoff hydrograph is computed (the peak runoff rate).

Another disadvantage of the Rational Formula Method is that with typical design procedures one normally assumes that all of the design flow is collected at the design point and that there is no "carry over water" running overland to the next design point. However, this is not the fault of the Rational Formula Method, but of the design procedure. The problem becomes one of routing the surface and subsurface hydrographs which have been separated by the storm sewer system. In general, this sophistication is not warranted and a conservative assumption is made wherein the entire routing occurs through the storm sewer system when this system is present.

#### 704.4 RAINFALL INTENSITY

The rainfall intensity, I, is the average rainfall rate in inches per hour for the period of maximum rainfall of a given frequency having a duration equal to the time of concentration. After the design storm frequency has been selected, a graph should be prepared showing rainfall intensity versus time. Information on local rainfall data is presented in Section 600 of this Manual.

#### 704.5 RUNOFF COEFFICIENT

The runoff coefficient, C, represents the integrated effects of infiltration, evaporation, retention, flow routing, and interception, all which affect the time distribution and peak rate of runoff. Determination of the coefficient requires judgment and understanding on the part of the engineer. Table 701 presents the recommended values of C for the various recurrence frequency storms. The values are presented for different surface characteristics as well as for different aggregate land uses. Variations to these values are subject to the approval of the Jurisdictional Entity.

A composite runoff coefficient is computed on the basis of the percentage of different types of surfaces in the drainage area. For homogeneous developed areas, this procedure is often applied to a typical "sample" area as a guide to selection of reasonable values of the coefficient for an entire area. Suggested coefficients with respect to surface type are also given in Table 701 under the column

labeled "Percent Impervious". Where land use features are mixed, a composite C analysis will result in more accurate results. The runoff coefficients in Table 701 also vary with recurrence frequency.

#### 704.6 APPLICATION OF THE RATIONAL FORMULA METHOD

The first step in applying the Rational Formula Method is to obtain a topographic map and define the boundaries of all the relevant drainage basins. Basins to be defined include all basins tributary to the area of study and sub-basins within the study area. A field check and possibly field surveys should be made for each basin. At this stage of planning, the possibility for the diversion of transbasin waters should be identified.

The major storm drainage basin does not always coincide with the minor storm drainage basin. This is often the case in urban areas where a low flow will stay next to a curb and follow the lowest grade, but when a large flow occurs the water will be deep enough so that part of the water will overflow street crowns and flow into a new sub-basin. An example of how to apply the Rational Formula Method is presented in Section 711.

#### 704.7 MAJOR STORM ANALYSIS

When analyzing the major runoff occurring within an area that has a storm sewer system sized for the minor storm, care must be used when applying the Rational Formula Method. Normal application of the Rational Method assumes that all of the runoff is collected by the storm sewer. For the minor storm design, the time of concentration is dependent upon the flow time in the sewer. However, during the major storm runoff, the sewers will probably be at capacity and would not carry the additional water flowing to the inlets. This additional water then flows overland past the inlets, generally at a lower velocity than the flow in the storm sewers.

If a separate time of concentration analysis is made for the pipe flow and surface flow, a time lag between the surface flow peak and the pipe flow peak will occur. This lag, in effect, will allow the pipe to carry a larger portion of the major storm runoff than would be predicted using the minor storm time of concentration. The basis for this increased benefit is that the excess water from one inlet will flow to the next inlet downhill, using the overland route. If that inlet is also at capacity, the water will often continue on until capacity is available in the storm sewer. The analysis of this aspect of the interaction between the storm sewer system and the major storm runoff is complex. The simplified approach of using the minor storm time of concentration for all frequency analysis is acceptable for use in Washoe County.

## 705 SCS UNIT HYDROGRAPH METHOD

The SCS Unit Hydrograph method was developed for the SCS by Mr. Victor Mockus. The SCS Unit Hydrograph was derived from a large number of natural unit hydrographs from watersheds varying widely in size and geographic location. The SCS Unit Hydrograph has been in use for many years and has produced satisfactory results for many applications. This method may be used for drainage areas within the Washoe County area in accordance with Section 304.3.

#### 705.1 METHODOLOGY

The SCS Unit Hydrograph method uses the unit hydrograph theory as a basis for runoff computations. The unit hydrograph theory computes rainfall excess hydrographs for a unit amount of rainfall excess applied uniformly over a sub-basin for a given unit of time (or unit duration). The rainfall excess hydrographs are then transformed to a sub-basin hydrograph by superimposing each excess hydrograph lagged by the unit duration.

# SECTION 700

# **STORM RUNOFF**

# 701 INTRODUCTION

For the area within the jurisdiction of this Manual, two deterministic hydrological models can be used to predict storm runoff (Policy Section 304). These models are the Rational Formula Method and the Soil Conservation Service, U.S. Department of Agriculture (SCS) Unit Hydrograph method. The procedures for using these methods are presented in this section. The Rational Formula Method may be employed without the use of computers. Computer modeling using the U.S. Army Corps of Engineers HEC-1 or HEC-HMS Flood Hydrograph Package or other hydrologic computer modeling programs is required for the SCS method. For certain circumstances, where adequate recorded stream flow data are available and the drainage area is large (> 10 square miles), a statistical analysis may be required to predict the storm runoff peaks or for calibration of deterministic models (see Section 708).

#### 701.1 BASIN CHARACTERISTICS

The basin characteristics needed for the subject runoff computation methods include the drainage area, soil type, the various flow path lengths, slopes, and characteristics (i.e., overland, grassed channel, gutter) and land use types. The drainage basin boundary and area may be determined from available topographic maps or site-specific mapping depending upon the level of detail required. A field investigation is recommended to verify drainage boundaries. The land use and flow path characteristics can be obtained from zoning maps, aerial photographs, field investigations, or detailed topographic maps.

## **702 TIME OF CONCENTRATION**

The definition of the time of concentration,  $t_c$ , for the purpose of this Manual, is the time required for water to flow from the hydraulically most distant part of the drainage area to the point under consideration. For the Rational Formula Method, the time of concentration must be estimated so that the average rainfall rate for the corresponding duration can be determined from the rainfall intensity-duration-frequency curves. For the SCS Unit Hydrograph method, the time of concentration is used to determine the time-to-peak,  $t_p$ , of the unit hydrograph and subsequently, the peak runoff.

In the past, several different time of concentration equations have been used with the runoff methods discussed in the following sections. However, as both methods have the same definition of the time of concentration, and to promote consistency between the two runoff methods, the time of concentration equations presented in this section shall be used for all watersheds of total area less than one square mile and whose basin slope is less than ten percent. For larger watersheds and for watersheds with basin slopes equal to or greater than ten percent, the basin lag equation shall be used (see Section 705.3).

For urban areas, the time of concentration consists of an inlet time or overland flow time  $(t_i)$  plus the time of travel  $(t_t)$  in the storm sewer, paved gutter, roadside drainage ditch, or drainage channel. For non-urban areas, the time of concentration consists of an overland flow time  $(t_i)$  plus the time of travel in a combined form, such as a small swale, channel, or wash. The latter portion  $(t_t)$  of the time of concentration can be estimated from the hydraulic properties of the storm sewer, gutter, swale, ditch, or wash. Inlet time, on the other hand, will vary with surface slope, depression storage, surface cover,

antecedent rainfall, and infiltration capacity of the soil, as well as distance of surface flow. Thus, the time of concentration for both urban and non-urban areas shall be calculated as follows:

$$t_c = t_i + t_t \tag{701}$$

In which  $t_c$  = time of concentration (minutes)

 $t_i$  = initial, inlet, or overland flow time (minutes)

 $t_t$  = travel time in the ditch, channel, gutter, storm sewer, etc. (minutes)

To aid in the computation of  $t_c$ , Standard Form 2 (see Section - 1500) has been developed to organize the computation. In all drainage studies, t<sub>c</sub> calculations should be submitted using Standard Form 2.

The initial or overland flow time, t<sub>i</sub>, may be calculated using the following equation:

$$t_{i} = \underline{1.8} \, \underline{(1.1 - R)}_{S^{1/3}} \, \underline{L}_{0}^{\frac{1}{2}}$$
(702)

Where

 $t_i$  = initial or overland flow time (minutes) R =flow runoff coefficient  $L_0 =$ length of overland flow (feet, 500 feet maximum) S = average overland basin slope (percent)

Equation 702 was originally developed by the Federal Aviation Administration (FAA, 1970) for use with the Rational Formula Method. However, the equation is also valid for computation of the initial or overland flow time for the SCS Unit Hydrograph method using the appropriate flow runoff coefficient.

For the Rational Formula Method, the 5-year runoff coefficient,  $C_5$ , presented in Table 701 shall be used as the flow runoff coefficient, R. For the SCS Unit Hydrograph method, R shall be calculated using the following equation:

$$R = .0132 \text{ CN} - 0.39 \tag{703}$$

This equation was developed by converting CN factors to typical C<sub>5</sub> runoff coefficients.

The overland flow length,  $L_0$ , is generally defined as the length of flow over which the flow characteristics appear as sheet flow or very shallow flow in grassed swales. Changes in land slope, surface characteristics, and small drainage ditches or gullies will tend to force the overland flow into a concentrated flow condition. Thus, the initial flow time would generally end at these locations.

For longer basin lengths, initial or overland flow needs to be considered in combination with the travel time,  $t_1$ , which is calculated using the hydraulic properties of the swale, ditch, or channel. For preliminary work, travel time can be estimated with the help of Figure 701 (SCS, 1985). The time of concentration is then the sum of the initial flow time,  $t_i$  and the travel time  $t_t$  (Equation 701). The minimum t<sub>c</sub> in Washoe County for non-urban watersheds shall be 10 minutes.

#### 702.1 **URBANIZED BASINS**

Overland flow in urbanized basins can occur from the back of the lot to the street, in parking lots, in greenbelt areas, or within park areas. It can be calculated using the procedure described in Section 702 except that the travel time, t<sub>t</sub>, to the first design point or inlet is estimated using the "Paved Area (Sheet Flow) & Shallow Gutter Flow" line in Figure 701. The time of concentration for the first design point in an urbanized basin using this procedure should not exceed the time of concentration

| RATION  | AL FORMULA METHOD<br>OFF COEFFICIENTS |                             |                                 |
|---|---------------------------------------|-----------------------------|---------------------------------|
|   |                                       | Runoff C                    | coefficients                    |
| Land Use or Surface<br>Characteristics                | Aver. % Impervious<br>Area            | 5-Year<br>(C <sub>g</sub> ) | 100-Year<br>(C <sub>100</sub> ) |
| Business/Commercial:                                  |                                       |                             | * * * * *                       |
| Downtown Areas  | 85                                    | .82                         | .85                             |
| Neighborhood Areas                                    | 70                                    | .65                         | .80                             |
| Residential:  |                                       |                             |                                 |
| (Average Lot Size)                                    |                                       |                             |                                 |
| <sup>1</sup> / <sub>8</sub> Acre or Less (Multi-Unit) | 65                                    | .60                         | .78                             |
| <sup>1</sup> / <sub>4</sub> Acre                      | 38                                    | .50                         | .65                             |
| <sup>1</sup> / <sub>8</sub> Acre                      | 30                                    | .45                         | .60                             |
| <sup>1</sup> / <sub>2</sub> Acre                      | 25                                    | .40                         | .55                             |
| 1 Acre  | 20                                    | .35                         | .50                             |
| Industrial:   | 72                                    | .68                         | .82                             |
| Open Space:   |                                       |                             |                                 |
| (Lawns, Parks, Golf Courses)                          | 5                                     | .05                         | .30                             |
| Undeveloped Areas:                                    |                                       |                             |                                 |
| Range   | 0                                     | .20                         | .50                             |
| Forest  | 0                                     | .05                         | .30                             |
| Streets/Roads:  |                                       |                             |                                 |
| Paved   | 100                                   | .88                         | .93                             |
| Gravel  | 20                                    | .25                         | .50                             |
| Drives/Walks:   | 95                                    | .87                         | .90                             |
| <u>Roof</u> :   | 90                                    | .85                         | .87                             |
| NT /  |                                       |                             |                                 |

# \_

Notes:

1. Composite runoff coefficients shown for Residential, Industrial, and Business/Commercial Areas assume irrigated grass landscaping for all pervious areas. For development with landscaping other than irrigated grass, the designer must develop project specific composite runoff coefficients from the surface characteristics presented in this table.

| VERSION: April 30, 2009 | REFERENCE:<br>USDCM. DROCOG. 1969 | TABLE |
|-------------------------|-----------------------------------|-------|
| WRC ENGINEERING, INC.   | (with modifications)              | 101   |

# **APPENDIX B**

**EXISTING HYDROLOGIC ANALYSIS** 

# **Project Description**

| File Name<br>Description | WCSD ARROW CREEK EXIST 5YR.SPF  |
|--------------------------|---------------------------------|
|                          | WASHOE COUNTY SCHOOL DISTRICT   |
|                          | ARROW CREEK MIDDLE SCHOOL       |
|                          | PRELIMINARY HYDROLOGIC ANALYSIS |
|                          | 5-YR EXISTING CONDITIONS        |
|                          |                                 |

# **Project Options**

| Flow Units                              | CFS          |
|---|--------------|
| Elevation Type                          | Elevation    |
| Hydrology Method                        | Rational     |
| Time of Concentration (TOC) Method      | SCS TR-55    |
| Link Routing Method                     | Hydrodynamic |
| Enable Overflow Ponding at Nodes        | YES          |
| Skip Steady State Analysis Time Periods | NO           |

# **Analysis Options**

| Start Analysis On Apr 05, 2018            | 00:00:00      |
|---|---------------|
| End Analysis On Apr 06, 2018              | 00:00:00      |
| Start Reporting On Apr 05, 2018           | 00:00:00      |
| Antecedent Dry Days 0                     | days          |
| Runoff (Dry Weather) Time Step 0 01:00:00 | days hh:mm:ss |
| Runoff (Wet Weather) Time Step 0 00:05:00 | days hh:mm:ss |
| Reporting Time Step 0 00:05:00            | days hh:mm:ss |
| Routing Time Step 30                      | seconds       |

#### Number of Elements

|                 | Qty |
|-----------------|-----|
| Rain Gages      | 0   |
| Subbasins       | 7   |
| Nodes           | 9   |
| Junctions       | 4   |
| Outfalls        | 5   |
| Flow Diversions | 0   |
| Inlets          | 0   |
| Storage Nodes   | 0   |
| Links           | 4   |
| Channels        | 2   |
| Pipes           | 2   |
| Pumps           | 0   |
| Orifices        | 0   |
| Weirs           | 0   |
| Outlets         | 0   |
| Pollutants      | 0   |
| Land Uses       | 0   |

#### **Rainfall Details**

Return Period...... 5 year(s)

# Subbasin Summary

| SN Subbasin | Area  | Weighted    | Total    | Total  | Total   | Peak   | Time of         |
|-------------|-------|-------------|----------|--------|---------|--------|-----------------|
| ID          |       | Runoff      | Rainfall | Runoff | Runoff  | Runoff | Concentration   |
|             |       | Coefficient |          |        | Volume  |        |                 |
|             | (ac)  |             | (in)     | (in)   | (ac-in) | (cfs)  | (days hh:mm:ss) |
| 1 X-01      | 11.83 | 0.3100      | 0.34     | 0.11   | 1.24    | 4.31   | 0 00:17:23      |
| 2 X-02      | 9.81  | 0.2600      | 0.33     | 0.09   | 0.85    | 3.09   | 0 00:16:30      |
| 3 X-03      | 13.98 | 0.2400      | 0.42     | 0.10   | 1.41    | 2.95   | 0 00:28:50      |
| 4 X-04      | 16.16 | 0.2000      | 0.35     | 0.07   | 1.12    | 3.76   | 0 00:17:45      |
| 5 X-05      | 22.98 | 0.2000      | 0.30     | 0.06   | 1.40    | 6.19   | 0 00:13:29      |
| 6 X-06      | 13.53 | 0.2000      | 0.31     | 0.06   | 0.84    | 3.56   | 0 00:14:09      |
| 7 X-07      | 10.62 | 0.2000      | 0.31     | 0.06   | 0.66    | 2.82   | 0 00:13:55      |

# **Project Description**

| File Name<br>Description | WCSD ARROW CREEK EXIST 100YR.SPF |
|--------------------------|----------------------------------|
|                          | WASHOE COUNTY SCHOOL DISTRICT    |
|                          | ARROW CREEK MIDDLE SCHOOL        |
|                          | PRELIMINARY HYDROLOGIC ANALYSIS  |
|                          | 100-YR EXISTING CONDITIONS       |
|                          |                                  |

# **Project Options**

| Flow Units                              | CFS          |
|---|--------------|
| Elevation Type                          | Elevation    |
| Hydrology Method                        | Rational     |
| Time of Concentration (TOC) Method      | SCS TR-55    |
| Link Routing Method                     | Hydrodynamic |
| Enable Overflow Ponding at Nodes        | YES          |
| Skip Steady State Analysis Time Periods | NO           |

# **Analysis Options**

| Start Analysis On Apr 05, 2018            | 00:00:00      |
|---|---------------|
| End Analysis On Apr 06, 2018              | 00:00:00      |
| Start Reporting On Apr 05, 2018           | 00:00:00      |
| Antecedent Dry Days 0                     | days          |
| Runoff (Dry Weather) Time Step 0 01:00:00 | days hh:mm:ss |
| Runoff (Wet Weather) Time Step 0 00:05:00 | days hh:mm:ss |
| Reporting Time Step 0 00:05:00            | days hh:mm:ss |
| Routing Time Step 30                      | seconds       |

#### Number of Elements

|                 | Qty |
|-----------------|-----|
| Rain Gages      | 0   |
| Subbasins       | 7   |
| Nodes           | 9   |
| Junctions       | 4   |
| Outfalls        | 5   |
| Flow Diversions | 0   |
| Inlets          | 0   |
| Storage Nodes   | 0   |
| Links           | 4   |
| Channels        | 2   |
| Pipes           | 2   |
| Pumps           | 0   |
| Orifices        | 0   |
| Weirs           | 0   |
| Outlets         | 0   |
| Pollutants      | 0   |
| Land Uses       | 0   |

#### **Rainfall Details**

Return Period...... 100 year(s)

# Subbasin Summary

| SN Subbasin | Area  | Weighted    | Total    | Total  | Total   | Peak   | Time of         |
|-------------|-------|-------------|----------|--------|---------|--------|-----------------|
| ID          |       | Runoff      | Rainfall | Runoff | Runoff  | Runoff | Concentration   |
|             |       | Coefficient |          |        | Volume  |        |                 |
|             | (ac)  |             | (in)     | (in)   | (ac-in) | (cfs)  | (days hh:mm:ss) |
| 1 X-01      | 11.83 | 0.5700      | 0.82     | 0.47   | 5.55    | 19.20  | 0 00:17:23      |
| 2 X-02      | 9.81  | 0.5400      | 0.81     | 0.44   | 4.28    | 15.55  | 0 00:16:30      |
| 3 X-03      | 13.98 | 0.5200      | 1.02     | 0.53   | 7.44    | 15.47  | 0 00:28:50      |
| 4 X-04      | 16.16 | 0.5000      | 0.84     | 0.42   | 6.76    | 22.73  | 0 00:17:45      |
| 5 X-05      | 22.98 | 0.5000      | 0.73     | 0.37   | 8.41    | 37.42  | 0 00:13:29      |
| 6 X-06      | 13.53 | 0.5000      | 0.75     | 0.38   | 5.09    | 21.55  | 0 00:14:09      |
| 7 X-07      | 10.62 | 0.5000      | 0.75     | 0.37   | 3.97    | 17.04  | 0 00:13:55      |

# APPENDIX C

**PROPOSED HYDROLOGIC ANALYSIS** 

# **Project Description**

| File Name<br>Description | WCSD ARROW CREEK PROP 5YR.SPF   |
|--------------------------|---------------------------------|
|                          | WASHOE COUNTY SCHOOL DISTRICT   |
|                          | ARROW CREEK MIDDLE SCHOOL       |
|                          | PRELIMINARY HYDROLOGIC ANALYSIS |
|                          | 5-YR PROPOSED CONDITIONS        |
|                          |                                 |

# **Project Options**

| Flow Units                              | CFS          |
|---|--------------|
| Elevation Type                          | Elevation    |
| Hydrology Method                        | Rational     |
| Time of Concentration (TOC) Method      | SCS TR-55    |
| Link Routing Method                     | Hydrodynamic |
| Enable Overflow Ponding at Nodes        | YES          |
| Skip Steady State Analysis Time Periods | NO           |

# **Analysis Options**

| Start Analysis On Apr 05, 2018            | 00:00:00      |
|---|---------------|
| End Analysis On Apr 06, 2018              | 00:00:00      |
| Start Reporting On Apr 05, 2018           | 00:00:00      |
| Antecedent Dry Days 0                     | days          |
| Runoff (Dry Weather) Time Step 0 01:00:00 | days hh:mm:ss |
| Runoff (Wet Weather) Time Step 0 00:05:00 | days hh:mm:ss |
| Reporting Time Step 0 00:05:00            | days hh:mm:ss |
| Routing Time Step 30                      | seconds       |

#### Number of Elements

|                 | Qty |
|-----------------|-----|
| Rain Gages      | 0   |
| Subbasins       | 9   |
| Nodes           | 17  |
| Junctions       | 11  |
| Outfalls        | 5   |
| Flow Diversions | 0   |
| Inlets          | 0   |
| Storage Nodes   | 1   |
| Links           | 12  |
| Channels        | 7   |
| Pipes           | 5   |
| Pumps           | 0   |
| Orifices        | 0   |
| Weirs           | 0   |
| Outlets         | 0   |
| Pollutants      | 0   |
| Land Uses       | 0   |

#### **Rainfall Details**

Return Period...... 5 year(s)

## Subbasin Summary

| 5 | SN Subbasin | Area  | Weighted    | Total    | Total  | Total   | Peak   | Time of         |
|---|-------------|-------|-------------|----------|--------|---------|--------|-----------------|
|   | ID          |       | Runoff      | Rainfall | Runoff | Runoff  | Runoff | Concentration   |
|   |             |       | Coefficient |          |        | Volume  |        |                 |
|   |             | (ac)  |             | (in)     | (in)   | (ac-in) | (cfs)  | (days hh:mm:ss) |
|   | 1 P-01      | 11.83 | 0.3100      | 0.34     | 0.11   | 1.24    | 4.31   | 0 00:17:23      |
|   | 2 P-02      | 9.81  | 0.2600      | 0.33     | 0.09   | 0.85    | 3.09   | 0 00:16:30      |
|   | 3 P-03      | 13.98 | 0.2400      | 0.42     | 0.10   | 1.41    | 2.95   | 0 00:28:50      |
|   | 4 P-04      | 7.11  | 0.2000      | 0.28     | 0.06   | 0.41    | 2.02   | 0 00:12:01      |
|   | 5 P-05      | 9.19  | 0.2000      | 0.26     | 0.05   | 0.48    | 2.85   | 0 00:10:00      |
|   | 6 P-06      | 26.94 | 0.5500      | 0.30     | 0.17   | 4.45    | 20.00  | 0 00:13:24      |
|   | 7 P-07      | 10.01 | 0.2000      | 0.30     | 0.06   | 0.60    | 2.72   | 0 00:13:14      |
|   | 8 P-08      | 2.28  | 0.2000      | 0.26     | 0.05   | 0.12    | 0.71   | 0 00:10:00      |
|   | 9 P-09      | 7.76  | 0.2000      | 0.29     | 0.06   | 0.44    | 2.21   | 0 00:11:59      |

# **Project Description**

| File Name<br>Description | WCSD ARROW CREEK PROP 100YR.SPF |
|--------------------------|---------------------------------|
|                          | WASHOE COUNTY SCHOOL DISTRICT   |
|                          | ARROW CREEK MIDDLE SCHOOL       |
|                          | PRELIMINARY HYDROLOGIC ANALYSIS |
|                          | 100-YR PROPOSED CONDITIONS      |
|                          |                                 |

# **Project Options**

| Flow Units                              | CFS          |
|---|--------------|
| Elevation Type                          | Elevation    |
| Hydrology Method                        | Rational     |
| Time of Concentration (TOC) Method      | SCS TR-55    |
| Link Routing Method                     | Hydrodynamic |
| Enable Overflow Ponding at Nodes        | YES          |
| Skip Steady State Analysis Time Periods | NO           |

# **Analysis Options**

| Start Analysis On Apr 05, 2018            | 00:00:00      |
|---|---------------|
| End Analysis On Apr 06, 2018              | 00:00:00      |
| Start Reporting On Apr 05, 2018           | 00:00:00      |
| Antecedent Dry Days 0                     | days          |
| Runoff (Dry Weather) Time Step 0 01:00:00 | days hh:mm:ss |
| Runoff (Wet Weather) Time Step 0 00:05:00 | days hh:mm:ss |
| Reporting Time Step 0 00:05:00            | days hh:mm:ss |
| Routing Time Step 30                      | seconds       |

#### Number of Elements

|                 | Qty |
|-----------------|-----|
| Rain Gages      | 0   |
| Subbasins       | 9   |
| Nodes           | 17  |
| Junctions       | 11  |
| Outfalls        | 5   |
| Flow Diversions | 0   |
| Inlets          | 0   |
| Storage Nodes   | 1   |
| Links           | 12  |
| Channels        | 7   |
| Pipes           | 5   |
| Pumps           | 0   |
| Orifices        | 0   |
| Weirs           | 0   |
| Outlets         | 0   |
| Pollutants      | 0   |
| Land Uses       | 0   |

#### **Rainfall Details**

Return Period...... 100 year(s)

## Subbasin Summary

| SN | Subbasin | Area  | Weighted    | Total    | Total  | Total   | Peak   | Time of         |
|----|----------|-------|-------------|----------|--------|---------|--------|-----------------|
|    | ID       |       | Runoff      | Rainfall | Runoff | Runoff  | Runoff | Concentration   |
|    |          |       | Coefficient |          |        | Volume  |        |                 |
|    |          | (ac)  |             | (in)     | (in)   | (ac-in) | (cfs)  | (days hh:mm:ss) |
| 1  | P-01     | 11.83 | 0.5700      | 0.82     | 0.47   | 5.55    | 19.20  | 0 00:17:23      |
| 2  | P-02     | 9.81  | 0.5400      | 0.81     | 0.44   | 4.28    | 15.55  | 0 00:16:30      |
| 3  | P-03     | 13.98 | 0.5200      | 1.02     | 0.53   | 7.44    | 15.47  | 0 00:28:50      |
| 4  | P-04     | 7.11  | 0.5000      | 0.69     | 0.34   | 2.44    | 12.21  | 0 00:12:01      |
| 5  | P-05     | 9.19  | 0.5000      | 0.62     | 0.31   | 2.87    | 17.19  | 0 00:10:00      |
| 6  | P-06     | 26.94 | 0.7200      | 0.73     | 0.52   | 14.06   | 63.33  | 0 00:13:24      |
| 7  | P-07     | 10.01 | 0.5000      | 0.72     | 0.36   | 3.60    | 16.44  | 0 00:13:14      |
| 8  | P-08     | 2.28  | 0.5000      | 0.62     | 0.31   | 0.71    | 4.26   | 0 00:10:00      |
| 9  | P-09     | 7.76  | 0.5000      | 0.69     | 0.34   | 2.67    | 13.34  | 0 00:11:59      |

# **APPENDIX D**

**DETENTION FACILITY ANALYSIS** 

# **Project Description**

| File Name   | WCSD ARROW CREEK PROP 100YR.SPF |  |
|-------------|---------------------------------|--|
| Description | WASHOE COUNTY SCHOOL DISTRICT   |  |
|             | ARROW CREEK MIDDLE SCHOOL       |  |
|             | PRELIMINARY HYDROLOGIC ANALYSIS |  |
|             | 100-YR PROPOSED CONDITIONS      |  |
|             |                                 |  |

# **Project Options**

| Flow Units                              | CFS          |
|---|--------------|
| Elevation Type                          | Elevation    |
| Hydrology Method                        | Rational     |
| Time of Concentration (TOC) Method      | SCS TR-55    |
| Link Routing Method                     | Hydrodynamic |
| Enable Overflow Ponding at Nodes        | YES          |
| Skip Steady State Analysis Time Periods | NO           |

# **Analysis Options**

| Start Analysis On Apr 05               | , 2018 00:00:00    |
|--|--------------------|
| End Analysis On Apr 06                 | , 2018 00:00:00    |
| Start Reporting On Apr 05              | , 2018 00:00:00    |
| Antecedent Dry Days 0                  | days               |
| Runoff (Dry Weather) Time Step 0 01:00 | 0:00 days hh:mm:ss |
| Runoff (Wet Weather) Time Step 0 00:05 | 5:00 days hh:mm:ss |
| Reporting Time Step 0 00:05            | 5:00 days hh:mm:ss |
| Routing Time Step 30                   | seconds            |

#### Number of Elements

|                 | Qty |
|-----------------|-----|
| Rain Gages      | 0   |
| Subbasins       | 9   |
| Nodes           | 17  |
| Junctions       | 11  |
| Outfalls        | 5   |
| Flow Diversions | 0   |
| Inlets          | 0   |
| Storage Nodes   | 1   |
| Links           | 12  |
| Channels        | 7   |
| Pipes           | 5   |
| Pumps           | 0   |
| Orifices        | 0   |
| Weirs           | 0   |
| Outlets         | 0   |
| Pollutants      | 0   |
| Land Uses       | 0   |

#### **Rainfall Details**

Return Period...... 100 year(s)

# Storage Nodes

#### Storage Node : Stor-01

#### Input Data

| Invert Elevation (ft)          | 5064.00  |
|--------------------------------|----------|
| Max (Rim) Elevation (ft)       | 5072.00  |
| Max (Rim) Offset (ft)          | 8.00     |
| Initial Water Elevation (ft)   | 5064.00  |
| Initial Water Depth (ft)       | 0.00     |
| Ponded Area (ft <sup>2</sup> ) | 17602.00 |
| Evaporation Loss               | 0.00     |

# Storage Area Volume Curves Storage Curve : Storage-01

| Stage | Storage            | Storage            |
|-------|--------------------|--------------------|
|       | Area               | Volume             |
| (ft)  | (ft <sup>2</sup> ) | (ft <sup>3</sup> ) |
| 0     | 109                | 0.000              |
| 1     | 843                | 476.00             |
| 2     | 2269               | 2032.00            |
| 3     | 3928               | 5130.50            |
| 4     | 5917               | 10053.00           |
| 5     | 8322               | 17172.50           |
| 6     | 10931              | 26799.00           |
| 7     | 13963              | 39246.00           |
| 8     | 17404              | 54929.50           |



### Storage Node : Stor-01 (continued)

#### **Output Summary Results**

| Peak Inflow (cfs)                                 | 37.20   |
|---|---------|
| Peak Lateral Inflow (cfs)                         | 0.00    |
| Peak Outflow (cfs)                                | 19.95   |
| Peak Exfiltration Flow Rate (cfm)                 | 0.00    |
| Max HGL Elevation Attained (ft)                   | 5070.03 |
| Max HGL Depth Attained (ft)                       | 6.03    |
| Average HGL Elevation Attained (ft)               | 5064.24 |
| Average HGL Depth Attained (ft)                   | 0.24    |
| Time of Max HGL Occurrence (days hh:mm)           | 0 00:39 |
| Total Exfiltration Volume (1000-ft <sup>3</sup> ) | 0.000   |
| Total Flooded Volume (ac-in)                      | 0       |
| Total Time Flooded (min)                          | 0       |
| Total Retention Time (sec)                        | 0.00    |

