# Community Services Department Planning and Building DETATCHED ACCESSORY DWELLING ADMINISTRATIVE REVIEW APPLICATION



Community Services Department Planning and Building 1001 E. Ninth St., Bldg. A Reno, NV 89512-2845

Telephone: 775.328.6100

#### **Administrative Review Permit**

Washoe County Code (WCC) Chapter 110, Article 809 Administrative Review Permits authorizes and specifies procedures for the Administrative Review process. This provides a method for reviewing proposed uses identified in Article 302 which have the potential to adversely impact other land uses, transportation or services and facilities in the vicinity. The Director of the Planning and Building Division, or their designee, shall review the application request for compliance with the Development Code taking into consideration any testimony offered by affected property owners and the applicant. The Director, or their designee, may approve, approve with conditions, modify, modify with conditions, or deny the request. See WCC 110.809, for further information.

#### **Development Application Submittal Requirements**

Applications are accepted on the 8<sup>th</sup> of each month. If the 8<sup>th</sup> falls on a non-business day, applications will be accepted on the next business day. Applications are accepted in person or online.

#### If you are submitting your application online, you may do so at OneNV.us

- 1. Fees: See Master Fee Schedule. Most payments can be made directly through the OneNV.us portal. If you would like to pay by check, please make the check payable to Washoe County and bring your application and payment to the Community Services Department (CSD).
- 2. **Development Application:** A completed Washoe County Development Application form and all supplemental questions.
- 3. **Owner Affidavit:** The Owner Affidavit must be signed and notarized by all owners of the property subject to the application request.
- 4. **Proof of Property Tax Payment:** The applicant must provide a written statement from the Washoe County Treasurer's Office indicating all property taxes for the current quarter of the fiscal year on the land have been paid.
- 5. **Application Materials:** The completed Administrative Review Permit Application materials.
- 6. Site Plan Specifications:
  - a. Lot size with dimensions drawn using standard engineering scales (e.g. scale 1" = 100', 1" = 200', or 1" = 500') showing all streets and ingress/egress to the property.
  - b. Show the location and configuration of all proposed buildings (with distances from the property lines and from each other), all existing buildings that will remain (with distances from the property lines and from each other), all existing buildings that will be removed, and site improvements on a base map with topography expressed in intervals of no more than five (5) feet IF there is a difference in elevation of the two dwellings of five (5) feet or more
  - c. Show all required yard setbacks, location and configuration of wells and well house, septic systems and leach fields, overhead utilities, water and sewer lines, and all easements.
  - d. Date, north arrow, scale and number of each sheet in relation to the total number of sheets, and the name of person preparing the plans.
- 7. Building Elevations: Elevations of the main dwelling unit and the detached accessory dwelling, existing or proposed for construction, shall be clearly depicted in vertical architectural drawings provided in accurate architectural scale. All architectural elevations from all building faces shall be presented. Photographs of existing buildings (all sides) may be submitted in place of elevations of existing building.

- 8. **Floor Plans:** Floor plans to scale with all rooms and spaces labeled for both the main dwelling unit and the detached accessory dwelling.
- 9. **Submission Packets:** One (1) packet and a flash drive. Any digital documents need to have a resolution of 300 dpi. If materials are unreadable, you will be asked to provide a higher quality copy. The packet shall include one (1) 8.5" x 11" reduction of any applicable site plan, development plan, and/or application map. Labeling on these reproductions should be no smaller than 8 point on the 8.5" x 11" display. Large format sheets should be included in a slide pocket(s). Any specialized reports identified above shall be included as attachments or appendices and be annotated as such.

#### Notes:

- (i) Application and map submittals must comply with all specific criteria as established in the Washoe County Development Code and/or the Nevada Revised Statutes.
- (ii) Based on the specific nature of the development request, Washoe County reserves the right to specify additional submittal packets, additional information and/or specialized studies that clarify the potential impacts and potential conditions of development in order to minimize or mitigate impacts resulting from the project. No application shall be processed until the information necessary to review and evaluate the proposed project is deemed complete by the Director of Planning and Building.
- (iii) **Labels:** If the assigned planner determines the abandonment will affect the access to a mobile home park, the applicant will be required to submit a list of mailing addresses for every tenant residing in the mobile home park.
- (iv) Please be advised that the Washoe County Director of the Planning and Building Division or their designee has the ability to determine if the application is incomplete if it cannot be ascertained what the applicant is requesting, or if there is insufficient information to determine a favorable outcome.

#### **Washoe County Development Application**

Your entire application is a public record. If you have a concern about releasing personal information, please contact Planning and Building staff at 775.328.6100.

Project Information	s	Staff Assigned Case No.:				
Project Name:						
Project Description:						
Project Address:						
Project Area (acres or square fe	et):					
Project Location (with point of re	eference to major cross	streets AND area locator):				
Assessor's Parcel No.(s):	Parcel Acreage:	Assessor's Parcel No.(s):	Parcel Acreage:			
Indicate any previous Washo Case No.(s).	e County approval	s associated with this applicat	tion:			
Applicant Inf	ormation (attach	additional sheets if necess	sary)			
Property Owner:		Professional Consultant:				
Name:		Name:				
Address:		Address:				
	Zip:		Zip:			
Phone:	Fax:	Phone:	Fax:			
Email:		Email:				
Cell:	Other:	Cell:	Other:			
Contact Person:		Contact Person:				
Applicant/Developer:		Other Persons to be Contact	ted:			
Name:		Name:				
Address:		Address:				
	Zip:		Zip:			
Phone:	Fax:	Phone:	Fax:			
Email:		Email:				
Cell:	Other:	Cell:	Other:			
Contact Person:		Contact Person:				
	For Office	Use Only				
Date Received:	Initial:	Planning Area:				
County Commission District:		Master Plan Designation(s):				
CAB(s):		Regulatory Zoning(s):				

#### **Administrative Review Permit Application** for a Detached Accessory Dwelling **Supplemental Information**

(All required information may be separately attached)

1.	What is the garage)?	size (square	footage) of the main dwelling or proposed main dwelling (exclude size of
2.			e proposed detached accessory dwelling (exclude size of garage)? If a home is the secondary dwelling, list the age and size of the unit.
3.	How are you compatibility		integrate the main dwelling and secondary dwelling to provide architectural ructures?
5.			ing spaces are available? Parking spaces must be shown on site plan. Are ay, or access improvements be required?
6.			nimize any potential negative impacts (e.g. increased lighting, removal of your project may have on adjacent properties?
7.	Is the subject Committee?	ct property p	art of an active Home Owners Association (HOA) or Architectural Control
	☐ Yes	□ No	If yes, please list the HOA name.
8.			covenants, recorded conditions, or deed restrictions (CC&Rs) that may ssory dwelling on your property?
	☐ Yes	□ No	If yes, please attach a copy.
9.		nent, mother	elling unit, whether attached or detached, is allowed per parcel. Is there a -in-law unit, next-gen addition with kitchen or any other type of secondary coperty?
	☐ Yes	□ No	If yes, please provide information on the secondary unit.

10. List who the service providers are for the main dwelling and accessory dwelling:

	Main Dwelling	Accessory Dwelling
Sewer Service		
Electrical Service		
Solid Waste Disposal Service		
Water Service		

19596 TYNER WAY INCLINE VILLAGE IN WASHOE COUNTY IN NEVADA IN APN: 125-511-10 IN INCLINE VILLAGE.

## CUSTOM RESIDENCE FOR WHISPER HOMES, LLC



#### VICINITY MAP

## Lak INCLINE VILLAGE Tahoe

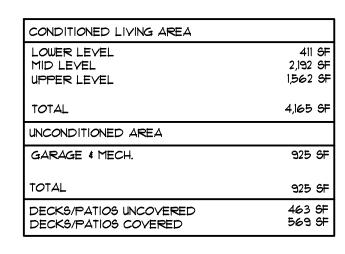
#### PROJECT SUMMARY

#### DESIGN CRITERIA:

CLASS: SINGLE FAMILY RESIDENCE (R3) THREE STORY WOOD FRAMED BUILDING  $\mbox{W}/\mbox{ ATTACHED 3 CAR GARAGE}$ WUI RATED IRI

#### PROJECT DESCRIPTION:

PROJECT PROPOSES TO DEMOLISH EXISTING RESIDENCE AND CONSTRUCT NEW SINGLE FAMILY DWELLING CONSISTING OF 4 BEDROOMS AND 4 AND  $\frac{1}{2}$  BATHROOMS WITH ATTACHED 3 CAR GARAGE. PROJECT INCLUDES THE CONSTRUCTION OF 3 DECKS AND 2 PATIOS.



## **GILANFARR** architecture

785 SOUTHWOOD BLVD SUITE 3 P.O. BOX 6987 INCLINE VILLAGE, NV 89451 VOICE.(775-831-8001)

#### GILANFARRARCHITECTURE.COM

#### NOTES:

THIS PROJECT IS DESIGNED UNDER THE 2018 EDITION OF THE INTERNATIONAL BUILDING CODE (IBC) OF THE INTERNATIONAL CONFERENCE OF BUILDING OFFICIALS AND THE 2018 IFC ALONG WITH THE 2018 WILDLAND URBAN INTERFACE CODE (IUWIC) WITH AMENDMENTS IN NLTFPD RESOLUTIONS 18-1 AND 18-2.

CONTRACTOR SHALL VERIFY ALL DIMENSIONS AND CONDITIONS SHOWN ON THESE DRAWINGS WITH THOSE AT THE SITE. ANY VARIATION WHICH REQUIRES PHYSICAL CHANGE SHALL BE BROUGHT TO THE ATTENTION OF THE ARCHITECT.

THESE DRAWINGS REMAIN THE PROPERTY OF GILANFARR + ASSOCIATES, ARCHITECTURE, A PROFESSIONAL CORPORATION. COPIES OF THESE DRAWINGS RETAINED BY THE CLIENT MAY BE UTILIZED ONLY FOR USE ON THE PARCEL FOR WHICH THEY WERE PREPARED AND ARE NOT FOR THE CONSTRUCTION OF ANY OTHER

### SHEET INDEX

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EXISTING SITE PLAN PROPOSED SITE PLAN PROPOSED BMP & GRADING PLAN LOWER LEVEL FLOOR PLAN LOWER LEVEL ELECTRICAL PLAN

LOWER LEVEL MECHANICAL PLAN MID LEVEL FLOOR PLAN MID LEVEL ELECTRICAL PLAN MID LEVEL MECHANICAL PLAN UPPER LEVEL FLOOR PLAN

UPPER LEVEL ELECTRICAL PLAN UPPER LEVEL MECHANICAL PLAN ROOF PLAN EXTERIOR ELEVATIONS

EXTERIOR ELEVATIONS INTERIOR ELEVATIONS BUILDING SECTIONS

BUILDING SECTIONS & ARCHITECTURAL DETAILS STRUCTURAL GENERAL NOTES SPECIAL INSPECTIONS TYPICAL DETAILS LOW FOUNDATION PLAN

FOUNDATION DIMENSION PLAN UPPER FOUNDATION/ ENTRY ROOF/ MID FLOOR FRAMING PLAN LOW ROOF/ UPPER FLOOR FRAMING PLAN UPPER ROOF FRAMING PLAN

ENTRY ROOF/ MID FLOOR DIAPHRAGM PLAN LOW ROOF/ UPPER FLOOR DIAPHRAGM PLAN UPPER ROOF DIAPHRAGM PLAN STRUCTURAL DETAILS

#### OWNER:

WHISPER HOMES, LLC 110 MAYS BLVD # 5101 INCLINE VILLAGE, NV 89451

STRUCTURAL ENGINEER:

STRUCTURED ENGINEERING, LLC.

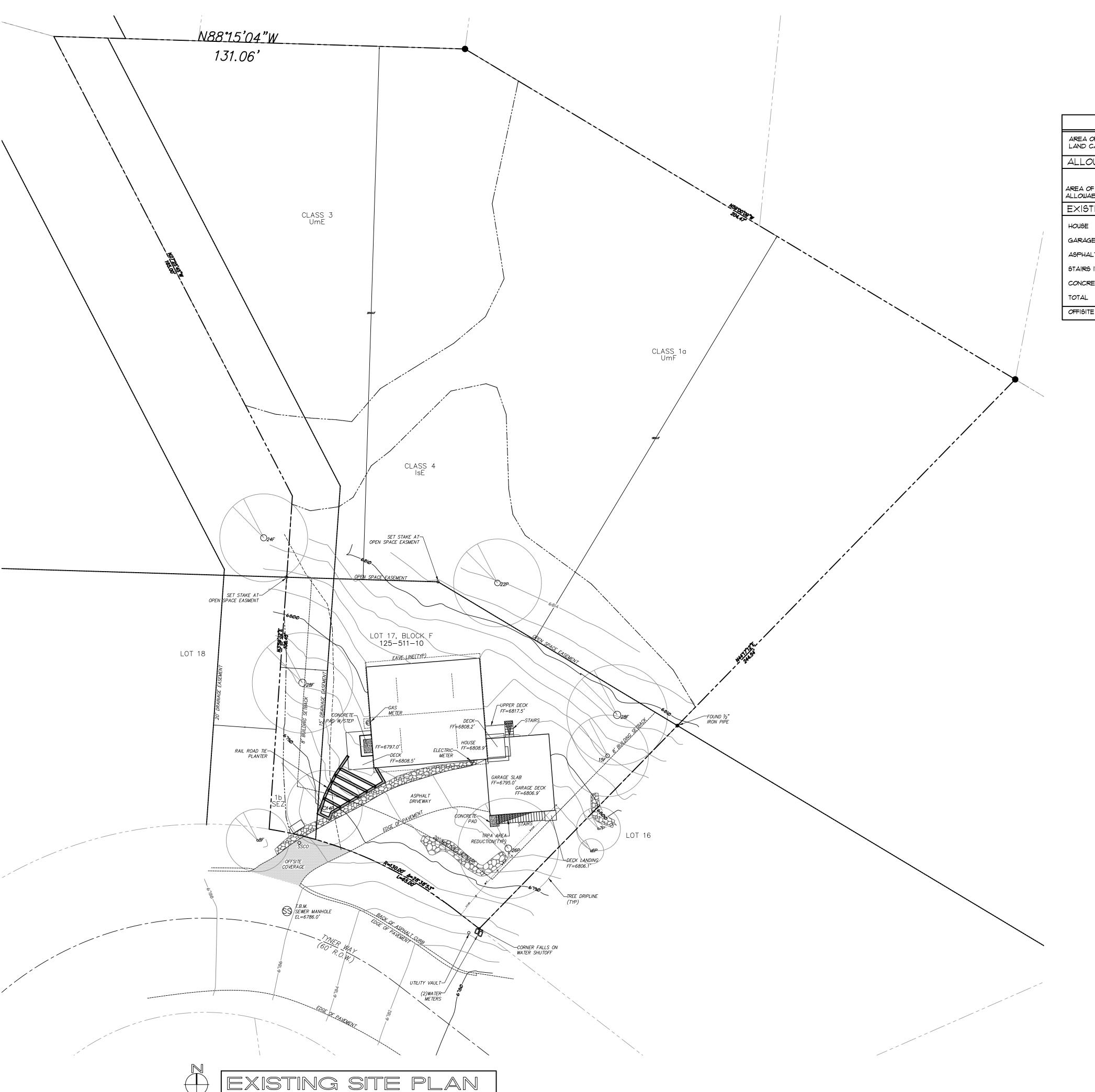
JEFF HARREL, P.E., PRINCIPAL 972 JENNIFER STREET

INCLINE VILLAGE, NEVADA

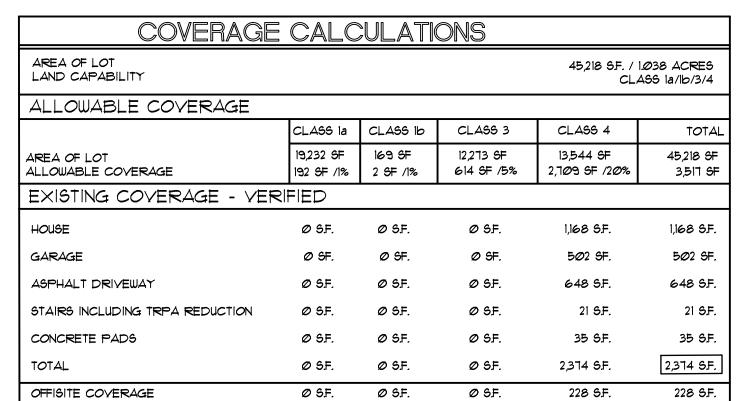
775-813-4915

09-10-24 PLAN CHECK 10-24-24 FIELD CHANGES Ø5-21-25 FIELD CHANGES

06-25-25 FIELD CHANGES 08-07-25 FIELD CHANGES



SCALE: 1/16" = 1'-0"







GILANFARR architecture

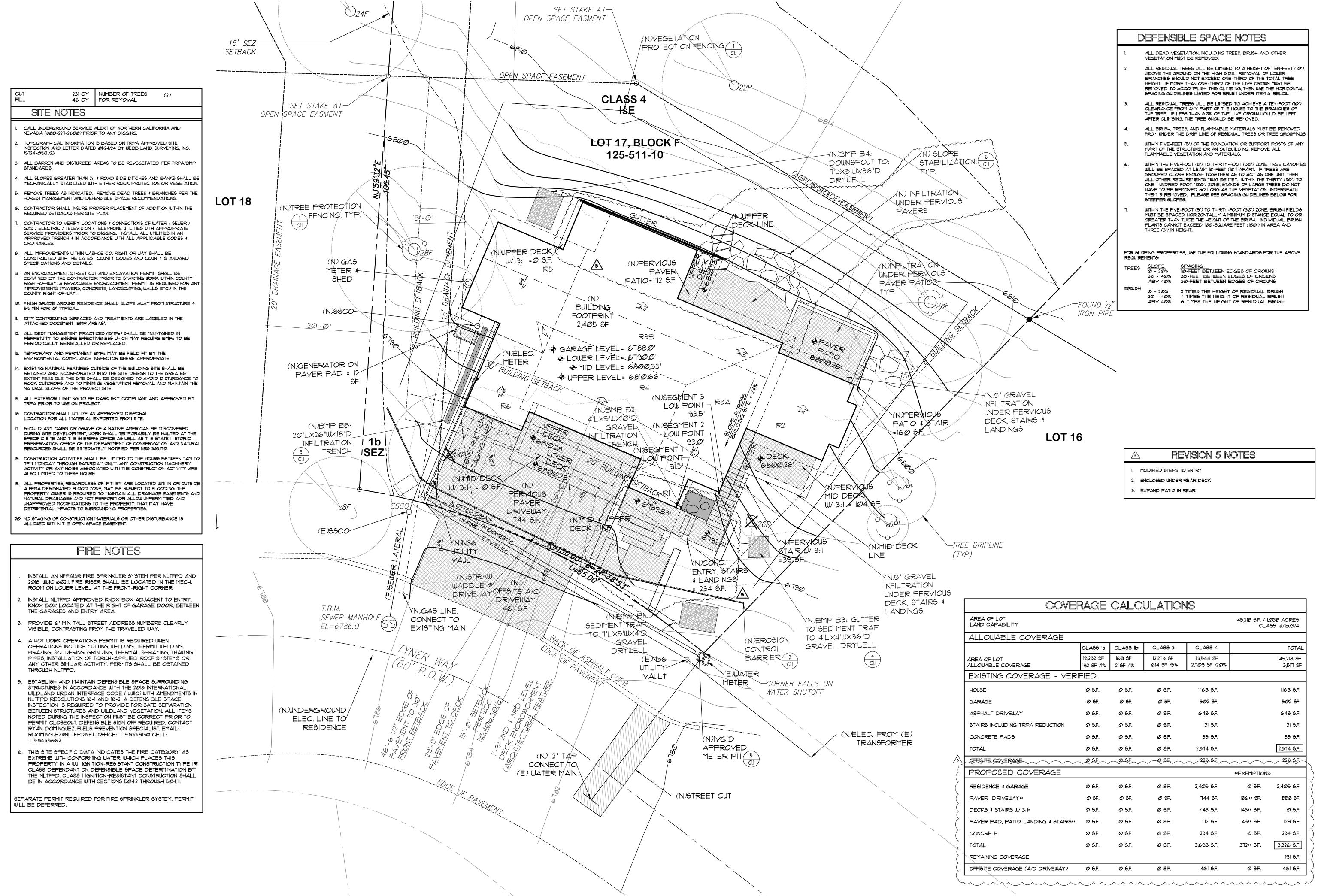
785 SOUTHWOOD BLVD SUITE 3 P.O. BOX 6987 INCLINE VILLAGE, NV 89451 VOICE.(775-831-8001)

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REVISI	ONS	

DATE: 06/03/24

AS NOTED



PROPOSED SITE PLAN

SCALE: 1/8" = 1'-0"





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10-24-24 FIELD CHANG

REVISIONS

596 TYNER

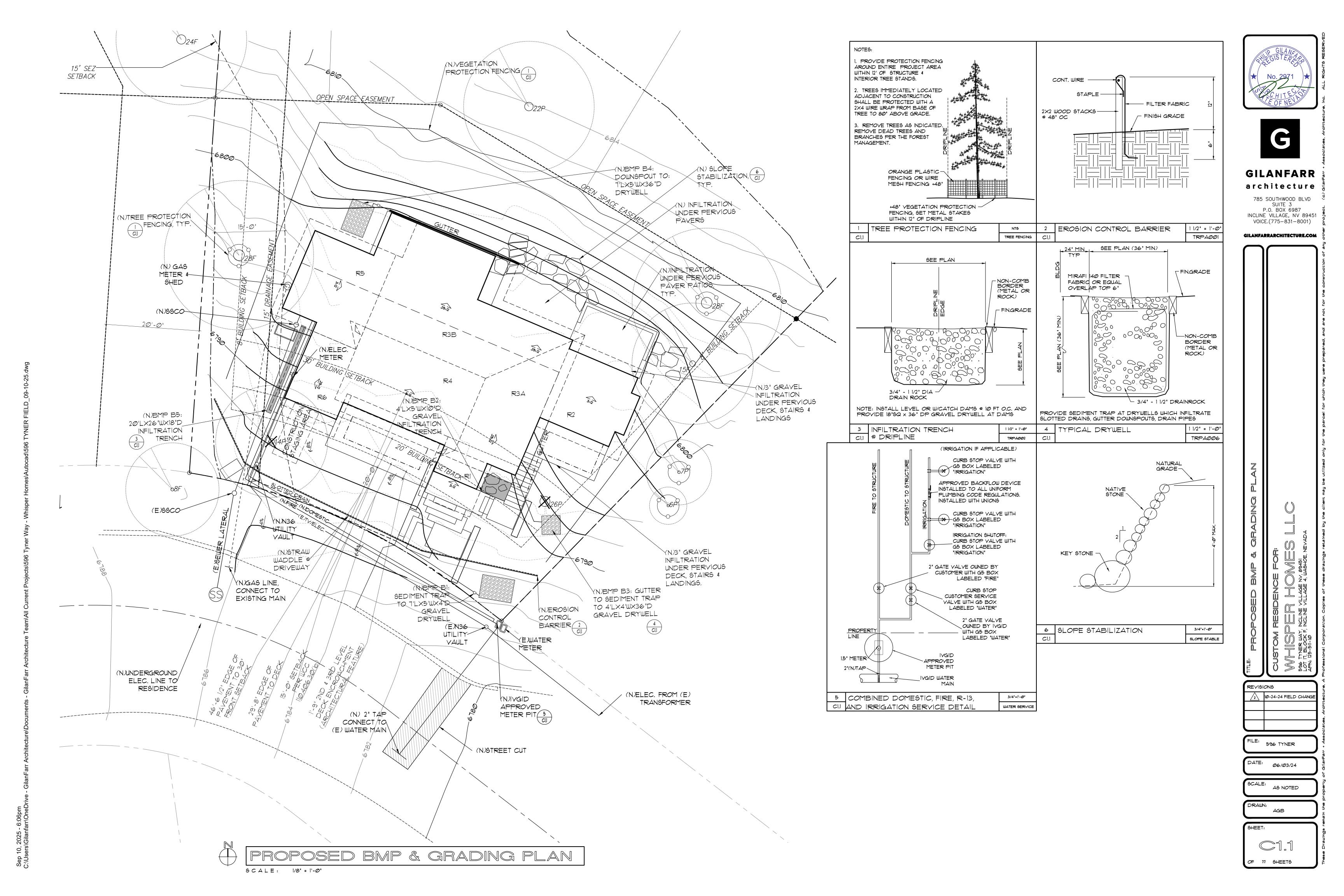
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S C A L E : 1/4" = 1' - 0"





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LOWER LEVEL FINISH PLA STOM RESIDENCE FOR: HSPER HOMES LLC

REVISIONS

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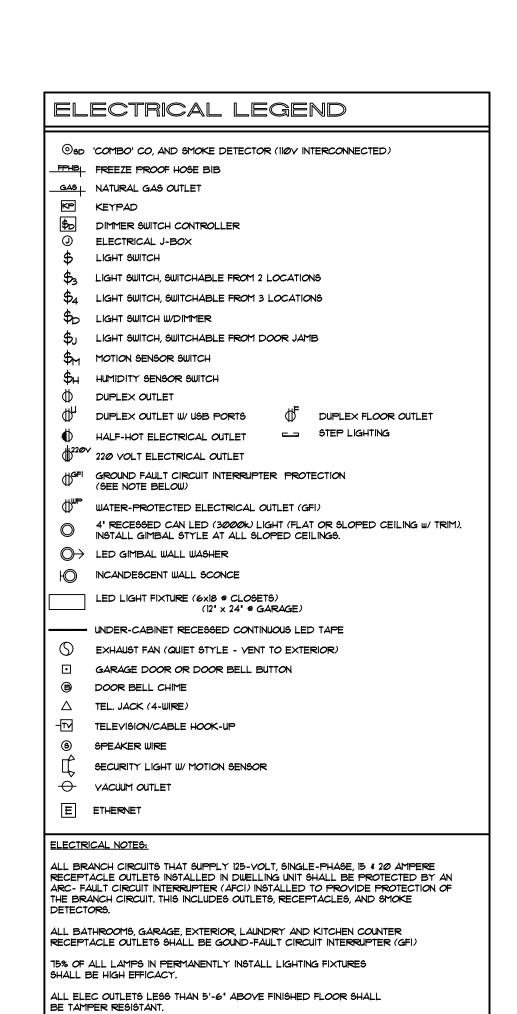
08-07-25 FIELD CHAN

FILE: 596 TYNER

DATE: 06-25-25

SCALE: AS NOTED

DRAWN:



ELEVATOR POWER: 220V/30amp AND 110V/15amp SERVICE EACH IN FUSIBLE LOCKABLE BOXES WITH 2-2" FLEX CONDUIT TO TOP OF ELEVATOR. CONFIRM WITH ELEVATOR MANUFACTURER.

ALL LED'S SHALL BE 3000k

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220V/30 AMP FOR CHAIN HEAT @ EAVE

+6 FOR LANDSCAPE

LIGHTING

VUNDER STAIR

TYP. STEP LIGHTS @

EVERY-OTHER STEP

UNDER LANDING





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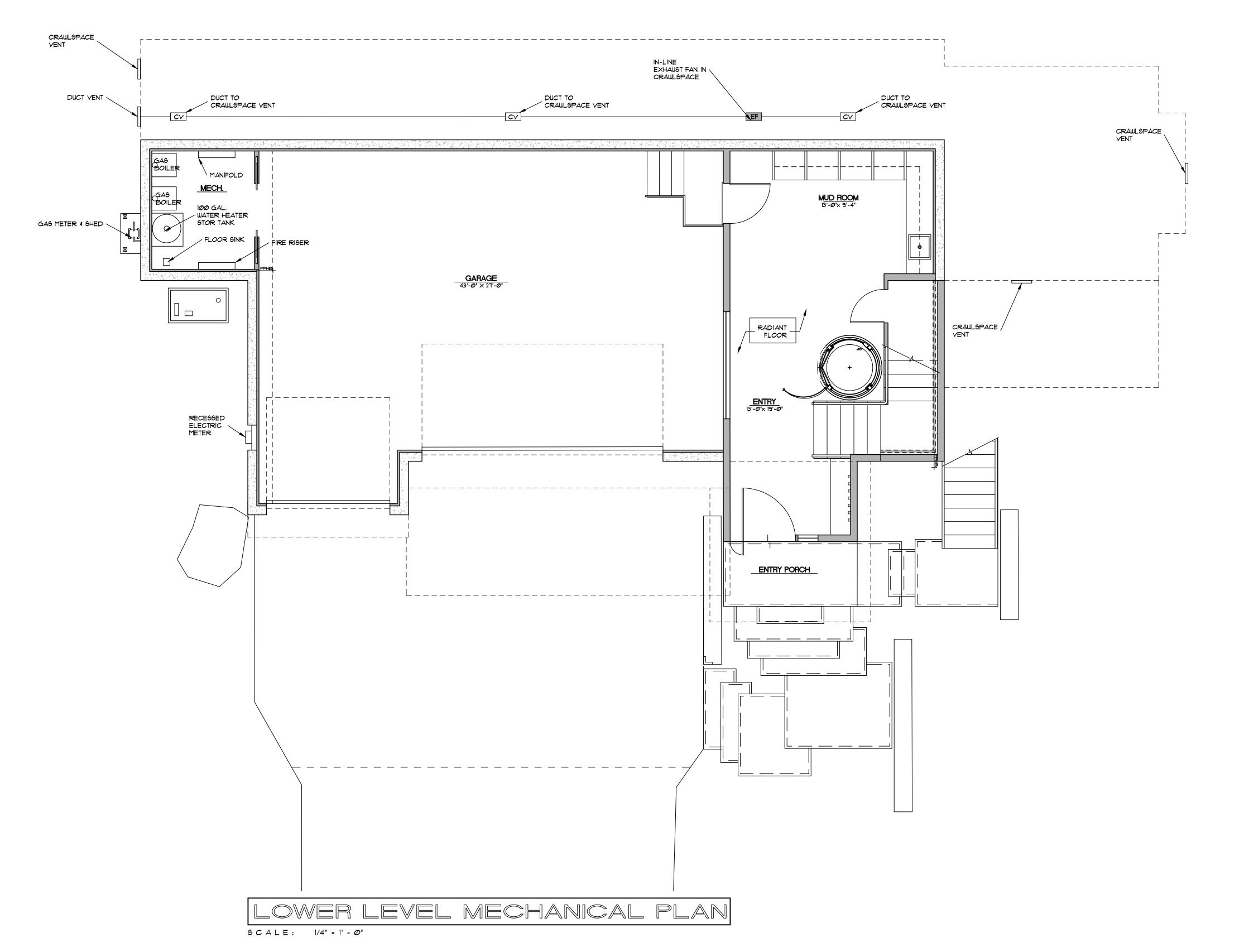
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DATE: Ø6-25-25

SCALE: AS NOTED

DRAWN:





MECH NOTES

VENT MECH. EQUIPMENT PER CODE

PROVIDE 8" X 14" SHT MTL LINED VENT TO EXTERIOR AT TOP & BTM OF WALL FOR COMBUSTION AIR.

PROVIDE 18" MIN. CLR AT BOILER OR INSTALL PER MANUFACTURERS SPECS.

PROVIDE SEISMIC STRAPS AT WATER HEATER PER CODE. PROVIDE RECIRCULATION PUMP AT DOMESTIC WATER SERVICE

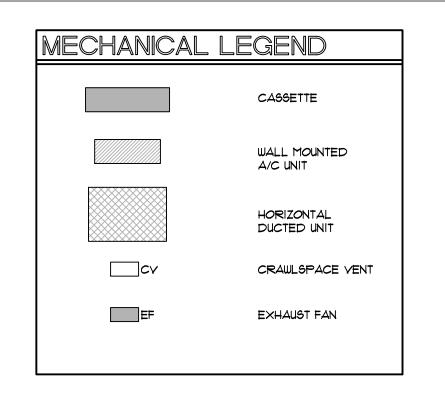
PROVIDE MIN R-2 PIPE WRAP INSULATION ON ALL HOT WATER SUPPLY

PEX AND ABS, CAST IRON DROP PIPES INSTALL WATER, SOIL, AND WASTE PIPES ON THE INSIDE FACE OF THE STUD WALL WITH A MINIMUM OF R-21 RIGID INSULATION TO THE OUTSIDE.

WATER SERVICE PIPE SHALL BE INSTALLED NOT LESS THAN 12" DEEP AND NOT LESS THAN 6" BLW FROST LINE.

HYDRONIC RADIANT IN-FLOOR HEATING.

MOUNT MECH. EQUIP. IN ATTIC/CRAWL SPACE W/ ISOLATION VIBRATION MOUNTS & CONDENSATION PAN W/ DRAIN TO EXTERIOR.

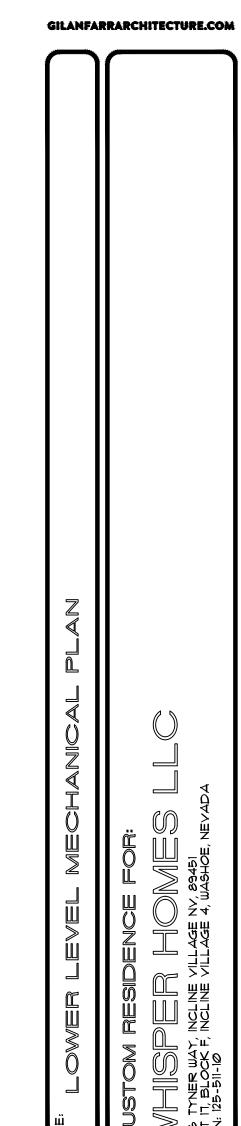






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2 10-24-24 FIELD CHANG

MID LEVEL FLOOR PLAN SCALE: 1/4" = 1' - Ø"

8'-0"





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REVISIONS 10-24-24 FIELD CHANGE /3 Ø5-21-25 FIELD CHANG Ø6-25-25 FIELD CHA

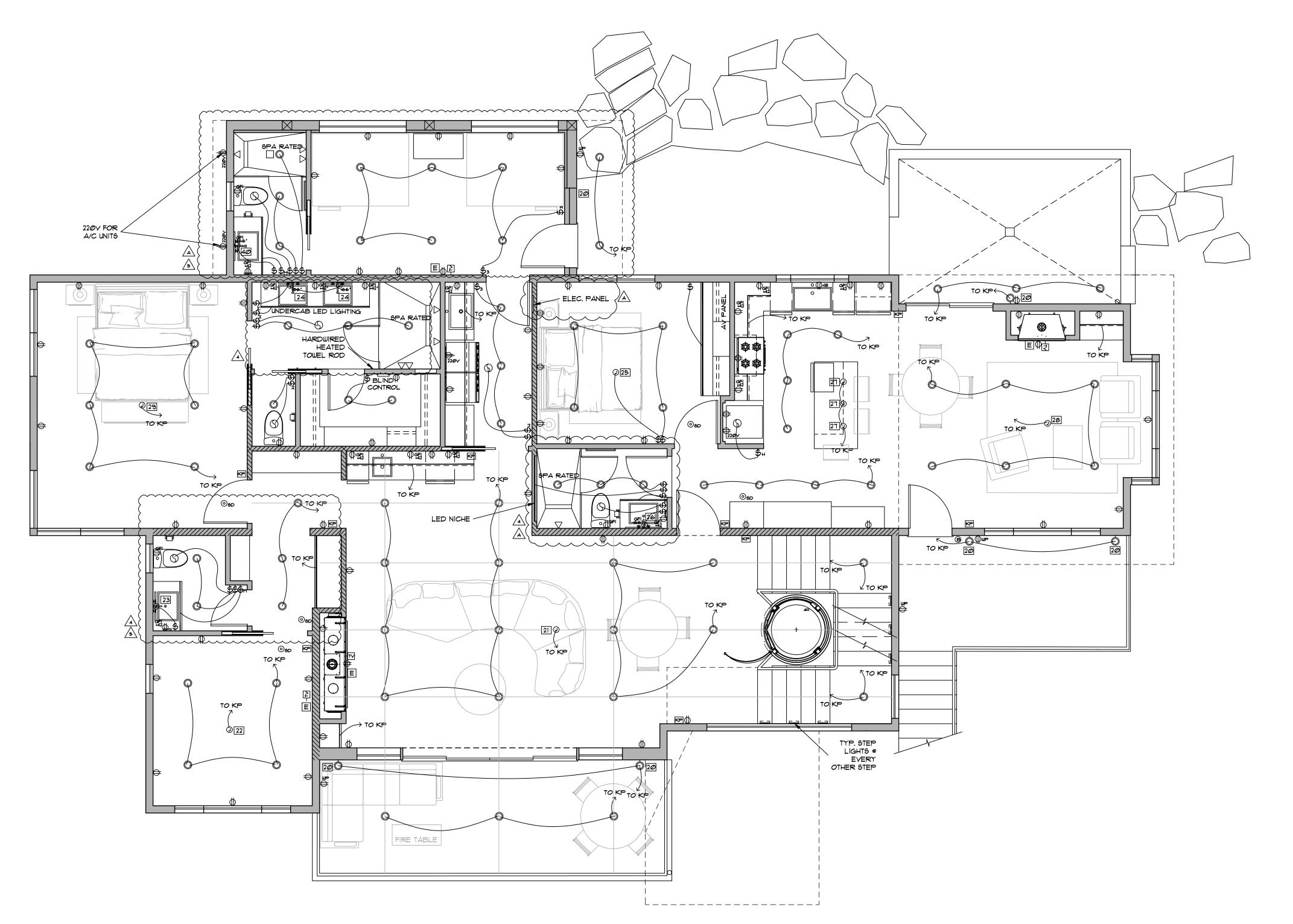
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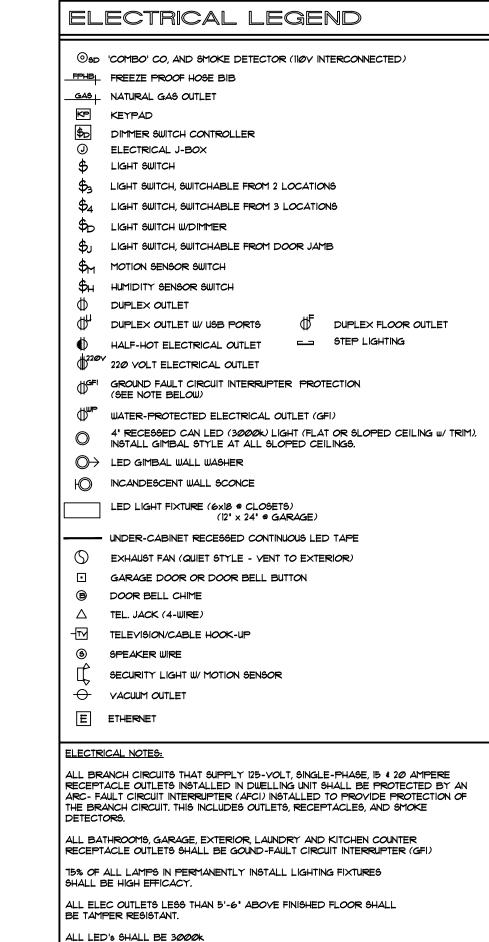
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SCALE: AS NOTED

DRAWN:

A2.0





ELEVATOR POWER: 220V/30amp AND 110V/15amp SERVICE EACH IN FUSIBLE LOCKABLE BOXES WITH 2-2" FLEX CONDUIT TO TOP OF ELEVATOR, CONFIRM WITH ELEVATOR MANUFACTURER.

MID LEVEL ELECTRICAL PLAN

S C A L E : 1/4" = 1' - ∅"

STOM RESIDENCE

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SUITE 3 P.O. BOX 6987

INCLINE VILLAGE, NV 89451 VOICE.(775–831–8001)

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REVISIONS

A 06-25-25 FIELD CHANGE
B 08-01-25 FIELD CHANGE

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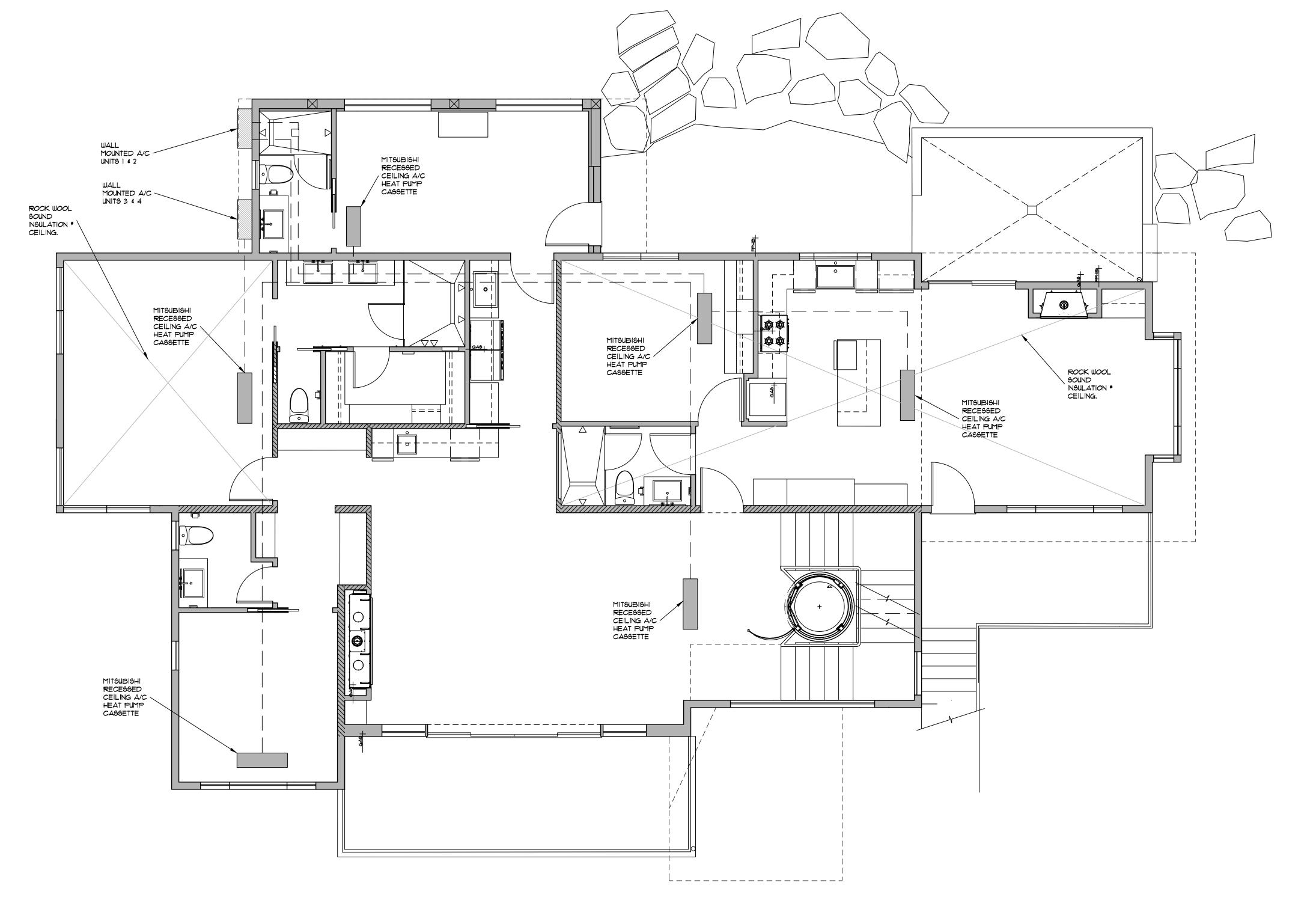
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#### VENT MECH. EQUIPMENT PER CODE

PROVIDE 8" X 14" SHT MTL LINED VENT TO EXTERIOR AT TOP & BTM OF WALL FOR COMBUSTION AIR.

PROVIDE 18" MIN. CLR AT BOILER OR INSTALL PER MANUFACTURERS SPECS.

PROVIDE SEISMIC STRAPS AT WATER HEATER PER CODE.

PROVIDE RECIRCULATION PUMP AT DOMESTIC WATER SERVICE

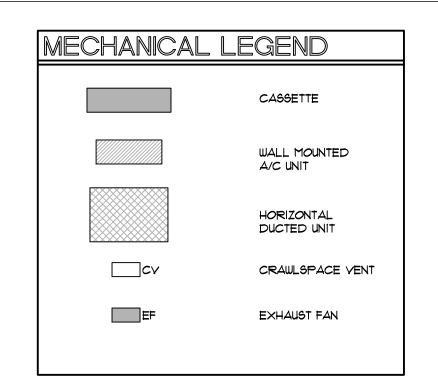
PROVIDE MIN R-2 PIPE WRAP INSULATION ON ALL HOT WATER SUPPLY
PEX AND ABS, CAST IRON DROP PIPES

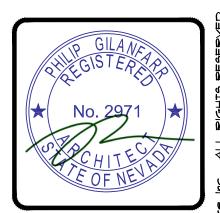
INSTALL WATER, SOIL, AND WASTE PIPES ON THE INSIDE FACE OF THE STUD WALL WITH A MINIMUM OF R-21 RIGID INSULATION TO THE OUTSIDE.

WATER SERVICE PIPE SHALL BE INSTALLED NOT LESS THAN 12" DEEP AND NOT LESS THAN 6" BLW FROST LINE.

HYDRONIC RADIANT IN-FLOOR HEATING.

MOUNT MECH. EQUIP. IN ATTIC/CRAWL SPACE W/ ISOLATION VIBRATION MOUNTS & CONDENSATION PAN W/ DRAIN TO EXTERIOR.







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EVEL MECHANICAL PLAN

CUSTOM RESIDER

WHISPERS

REVISIONS

10-24-24 FIELD CHANGE

FILE: 596 TYNER

DATE: 06/03/24

SCALE: AS NOTED

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A2.2

MID LEVEL MECHANICAL PLAN

SCALE: 1/4" = 1' - 0"

UPPER LEVEL FLOOR PLAN

SCALE: 1/4" = 1' - 0"





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4 06-25-25 FIELD CHAN Ø8-Ø7-25 FIELD CHA

2 10-24-24 FIELD CHANGE

FILE: 596 TYNER

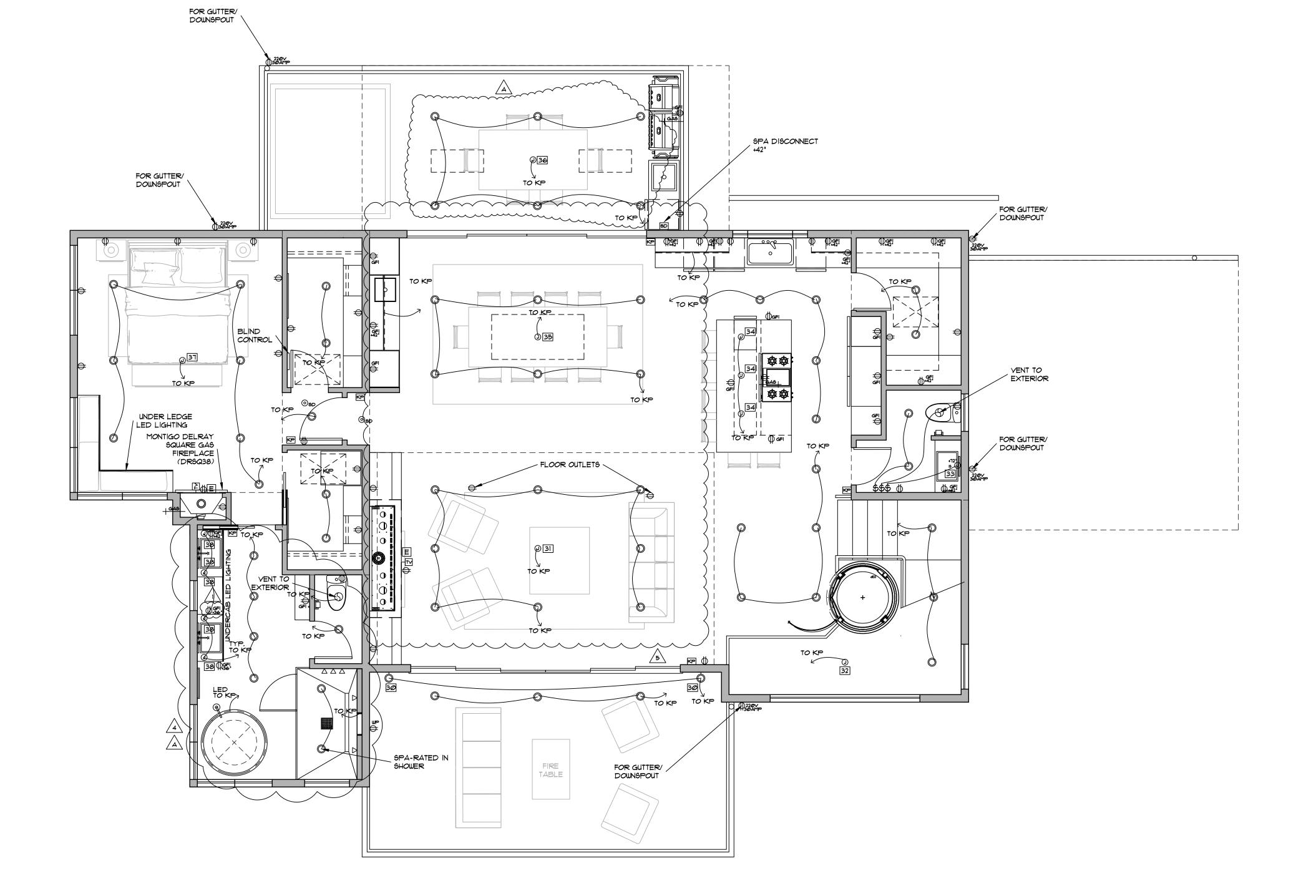
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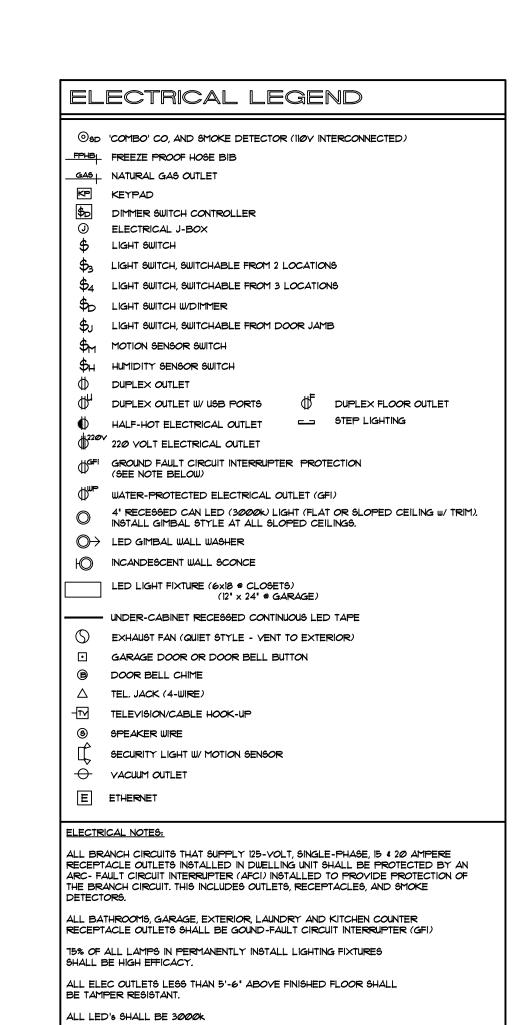
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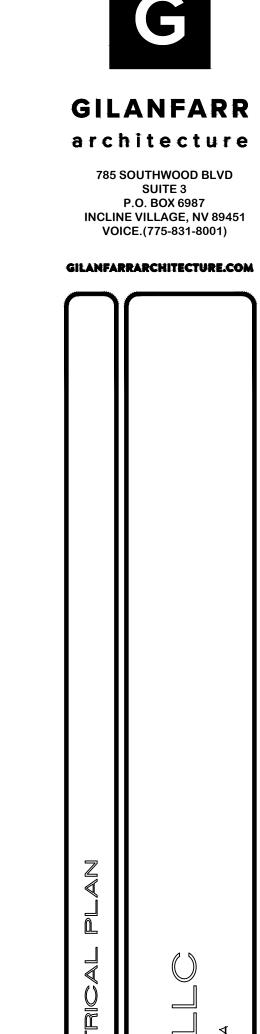
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A RESIDENCE FOR:
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T, INCLINE VILLAGE A, WASHOE, NEVADA

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1 09-10-24 PLAN CHECK
4 06-25-25 FIELD CHAN
5 08-01-25 FIELD CHAN

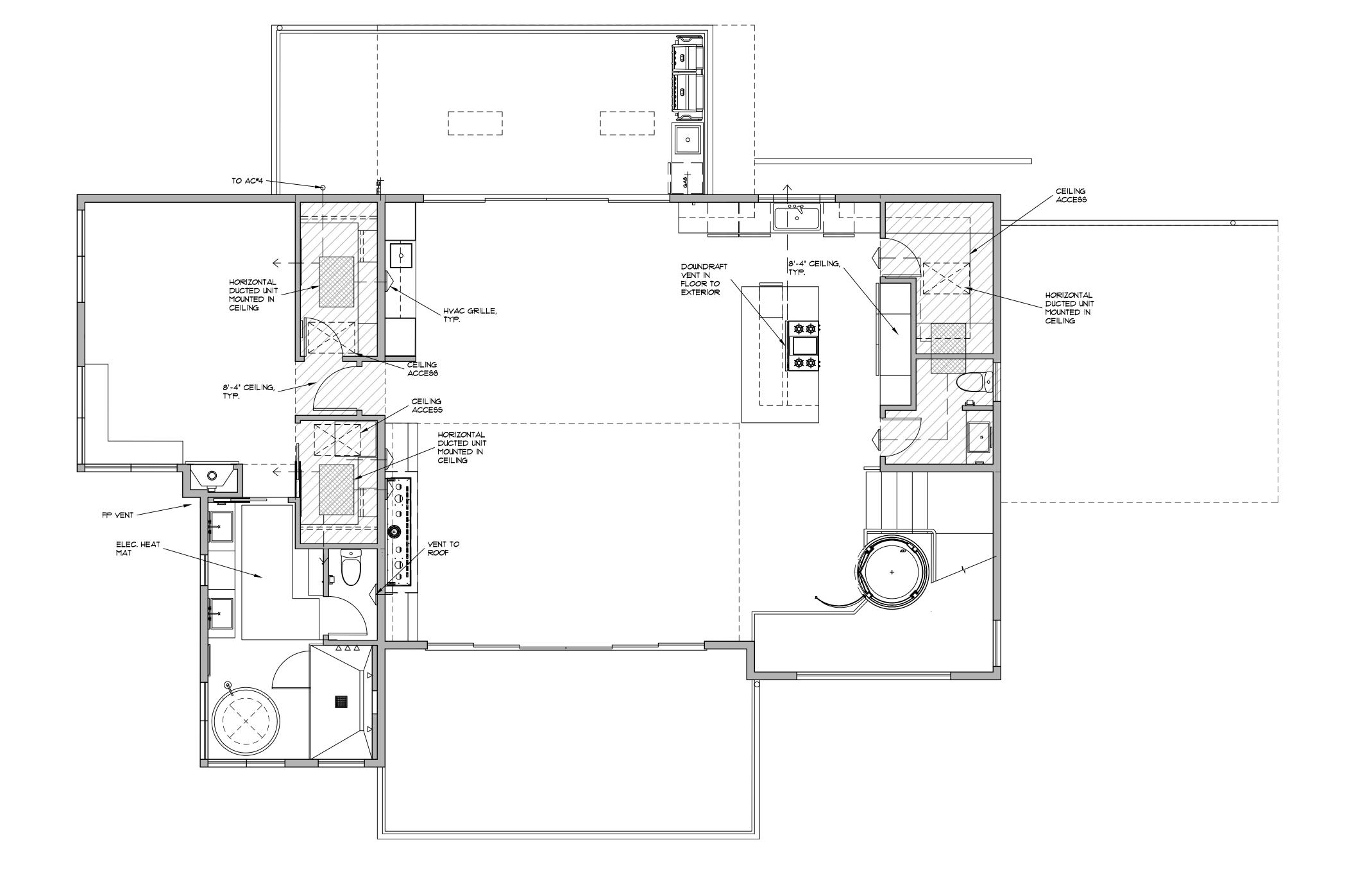
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MECHANICAL LEGEND CASSETTE WALL MOUNTED A/C UNIT HORIZONTAL DUCTED UNIT CRAWLSPACE VENT EXHAUST FAN

PROVIDE 8" X 14" SHT MTL LINED VENT TO EXTERIOR AT TOP 4 BTM OF WALL FOR COMBUSTION AIR.

PROVIDE 18' MIN. CLR AT BOILER OR INSTALL PER MANUFACTURERS SPECS.

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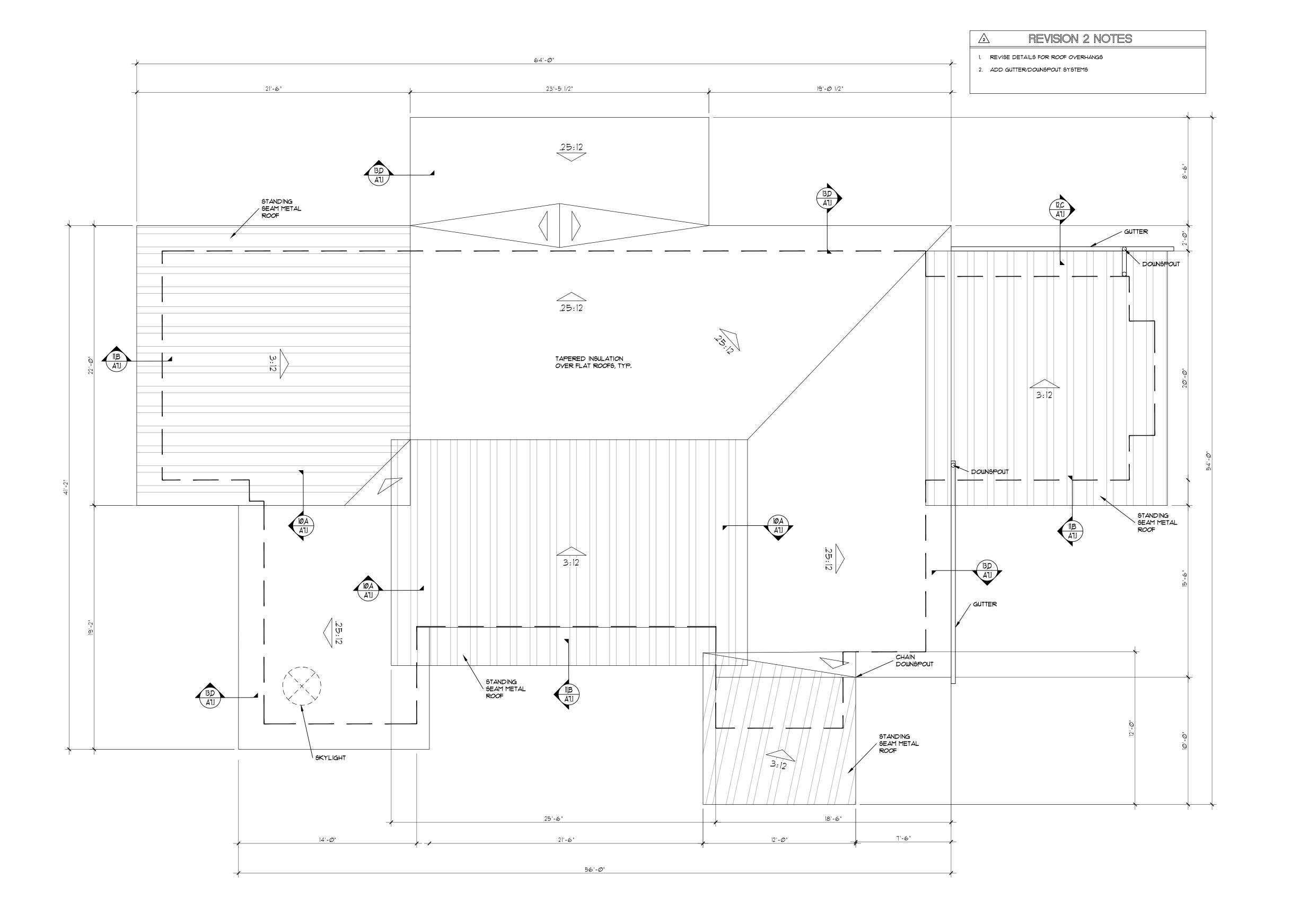
MECH NOTES

VENT MECH. EQUIPMENT PER CODE

PEX AND ABS, CAST IRON DROP PIPES

HYDRONIC RADIANT IN-FLOOR HEATING.

2 10-24-24 FIELD CHANG









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	2	10-24-24 FIELD CHANG
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FILE: 596 TYN

DATE

SCALE: AS NOTED

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TITLE: NEW ELEVATIONS

CUSTOM RESIDENCE FOR:

WHISPER WAY, INCLINE VILLAGE NV, 89451

LOT IT, BLOCK F, INCLINE VILLAGE 4, WASHOE, NEVADA

FILE: 596 TYNER

DATE: 06/03/24

10-24-24 FIELD CHANGE

\$ 08-Ø7-25 FIELD CHAN

SCALE: AS NOTED

DRAWN:

A5.0



#### EXTERIOR FINISH NOTES:

SLOPE ROOF: CLASS A STANDING SEAM, METAL ROOF OVER ICE & WATER SHIELD. COLOR: BLACK.

FLAT ROOF (3:12 OR LEGG): PROVIDE "POLYGLAGG" ELAGTOGHEILD TG4 45MM GRANULAR NON-WOVEN POLYEGTER MATT W/ STONE BALLAGT. COLOR: DARK GRAY.

FASCIA: 2x CEDAR FASCIA PAINTED BLACK.

SOFFITS: 1x4 RESAUN CEDAR, T&G SQ-GROOVE. MACHINE APPLY FX LUMBER GUARD XT FIRE RETARDANT PRIOR TO STAIN. INSTALL OVER \$ SOFFIT GYP. BOARD. SEMI-TRANSPARENT. SOFFIT ALL EXPOSED LYL OR ML RAFTERS. STAIN COLOR: NATURAL.

SIDING: IX8 RESAUN WESTERN RED CEDAR HORIZONTAL T&G SQ. GROOVE LAP SIDING. MACHINE APPLY FX LUMBER GUARD XT FIRE RETARDANT PRIOR TO STAIN. INSTALL OVER 1 SOFFIT GYP. BOARD. OVER LP 1 FLAMEBLOCK OR EQUAL. STAIN COLOR: NATURAL

METAL TRIM: 22GA BLACK ANODIZED SHEET METAL AT CORNERS AND COUNTER FLASHING AT WINDOWS AND DOORS.

PAINT ALL EXPOSED METAL FLAT TO MATCH ROOF.

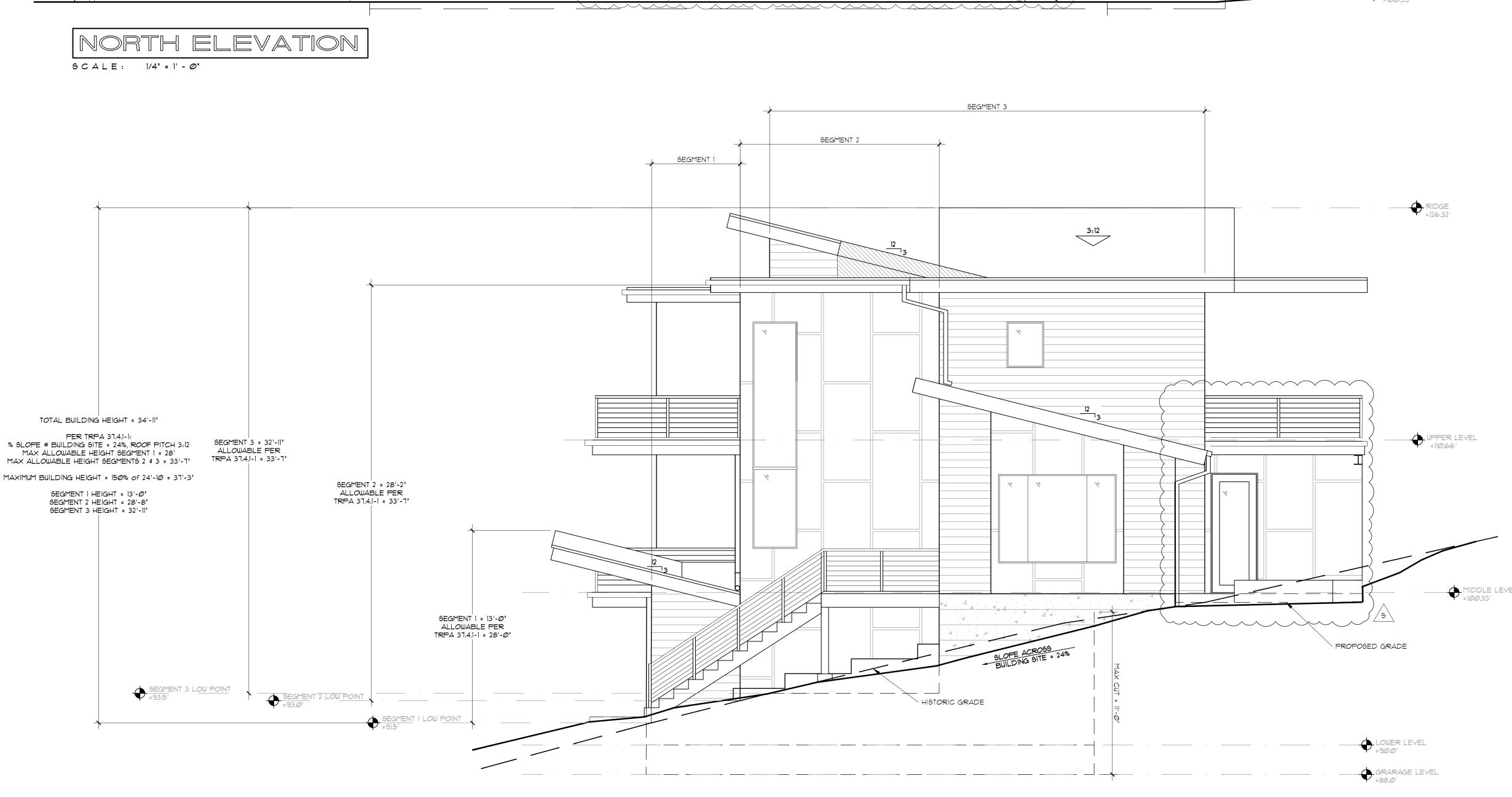
PAINT ALL EXPOSED BEAMS AND POSTS. COLOR: BLACK

ALL EXTERIOR WOODWORK TO BE PRE-FINISHED. USE STAINLESS STEEL NAILS. WINDOWS: TEMPERED GLASS, DUAL GLAZE, LOW E2 IN MTL CLAD FRAMES COLOR: BLACK.

GARAGE DOORS: OVERHEAD, CUSTOM ROLL-UP, INSULATED (WHITE GLASS W/ METAL DOORS. COLOR: BLACK.

RAILINGS: 2X2 METAL POSTS W/ TEMPERED CLEAR GLASS PANELS, PROVIDE THERMORY ASH RAIL CAP.

DECKING: 2X DECKING MATERIAL ON EITHER A PEDESTAL OR TREATED WOOD SYSTEM OVER POLYGLASS ELASTOSHIELD TS4 45MM GRANULAR NON-WOVEN POLYESTER MATT. MINIMUM  $\frac{1}{4}$ 1:12" SLOPE AWAY FROM STRUCTURE. FINAL STAIN COLORS PER CONTRACTOR OR OWNER.



SCALE: 1/4" = 1' - 0"



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REVISIONS /2 10-24-24 FIELD CHANG 5 08-01-25 FIELD CHAN

596 TYNER

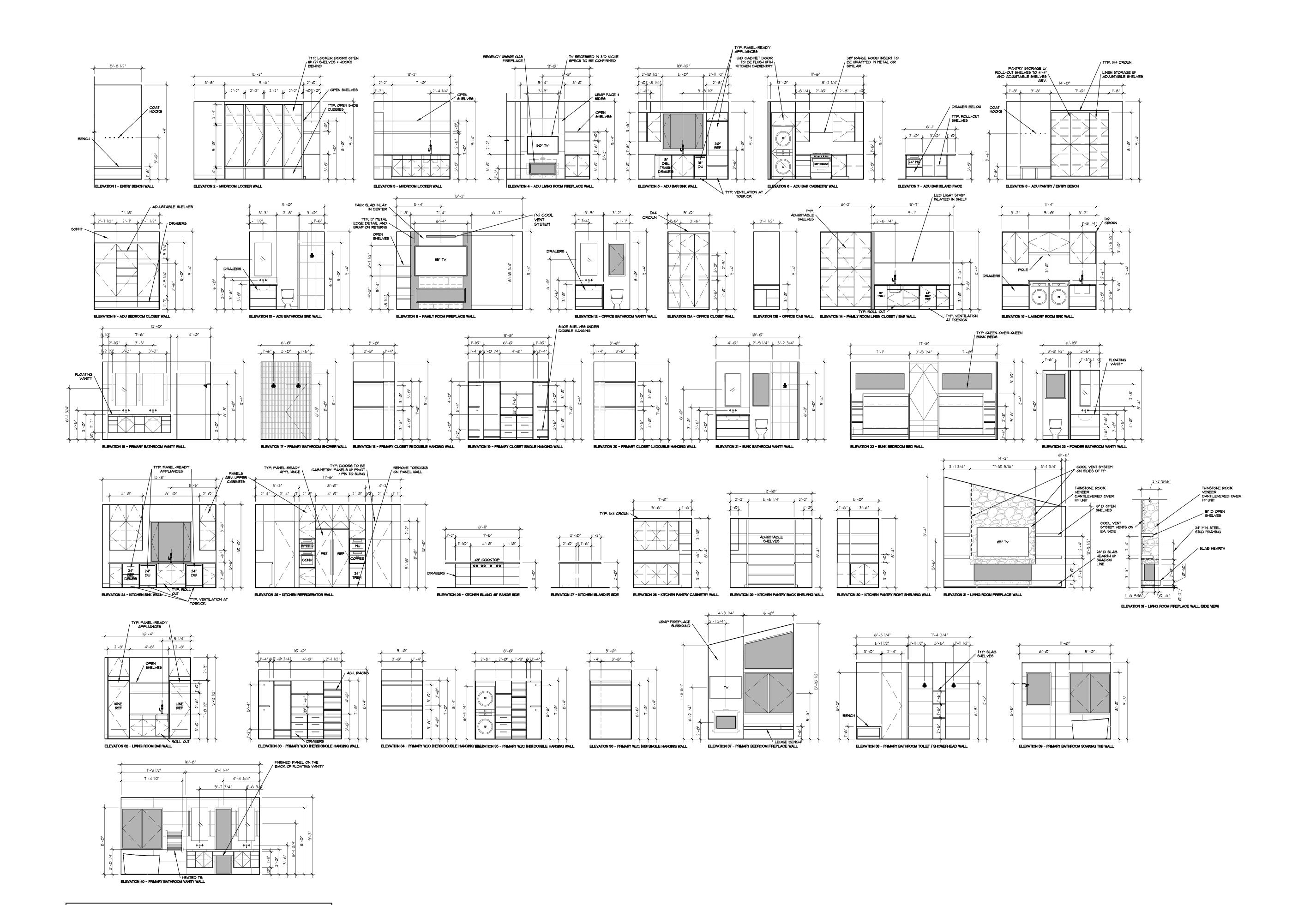
DATE:

SCALE:

SHEET:

INTERIOR ELEVATIONS

SCALE: 3/16" = 1' - 0"







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FILE: 596 TYNER DATE: 06-25-25

SCALE:

AS NOTED AGB / EAC

A6.0 OF ?? SHEETS

#### SECTION NOTES:

FOUNDATION WALL: SEE PLAN FOR FOOTING SIZE AND REINFORCEMENTS, 8" CONC. STEMWALL W/#4 BARS VERT @ 24" OC (ALTERNATE HOOKS) AND #4 BARS HORIZ @ 24" OC, UON

MINIMUM CONCRETE STRENGTH SHALL BE 2500 PSI AT 28 DAYS,

3x6 PRESSURE - TREATED (OR REDWOOD) MUD SILL w/  $\frac{5}{6}$ " DIA x10' DEEP ANCHOR BOLTS AT 48' OC (TYP UNO).

R-11 POLYSTYRENE INSULATION ON INTERIOR FACE OF STEMWALL (w/ AN APPROVED THERMAL BARRIER HAVING AN INDEX OF 15 OR GREATER) AND UNDER ENTIRE SLAB. COVER ENTIRE CRAWL W/ CONTINUOUS CLASS I VAPOR RETARDER BARRIER W/ 6' LAP AT ALL SPLICES, SEALED OR TAPED. EXTEND BARRIER UP WALLS A MIN. OF 6". ATTACH AND SEAL TO STEM WALL. STEGOCRAWL OR EQUAL.

TYPICAL EXTERIOR PONY WALL - 2x6 STUDS @ 16' OC R-21 BATT

TYPICAL EXTERIOR WALL - 2x6 STUDS @ 16" OC, 1/2" GYP BD THROUGHOUT W/R-21 BATT INSULATION, EXTERIOR SIDING OVER TYVEK HOUSE WRAP OR APPROVED WEATHER - RESISTIVE BARRIER - SEE EXTERIOR ELEVATIONS FOR SIDING TYPES.

TYPICAL INTERIOR WALL - 2x4 STUDS @ 16" OC, UNO. 1/2" GYP BD THROUGHOUT (WATER PROOF AT BATHS).

TYPICAL FLOOR CONSTRUCTION - 11  $\frac{1}{8}$ " TJI @ 16" OC  $\frac{1}{9}$ " T&G SUB-FLOOR, GLUE AND NAIL  $\frac{1}{9}$ " OC EDGES & 10" OC FIELD, UON. (SEE STRUCTURAL)

FLOORS OVER UNHEATED SPACE OR EXTERIOR PROVIDE R-24 BATT INSULATION. PROVIDE 11/2" GYPCRETE OVER PLYWOOD SUBFLOOR FOR

DIFFER FROM PLANS.

SCALE: 1/4" = 1' - 0"

HYDRONIC HEAT. FINISH FLOOR ELEVATIONS & VERTICAL DIMENSIONS ARE BASED ON NOMINAL SIZES. FIELD VERIFICATION OF GRADES, WINDOW & DOOR RO. REQUIREMENTS IS REQUIRED PRIOR TO FRAMING.

ALERT ARCHITECT OF ANY SIZES OR HEADER HEIGHTS THAT

TYPICAL ROOF CONST. RAFTERS @ 16' O.C. W/ 40/20 APA RATED SHTG (PSI - 83) NAIL w/8d @ 6' OC EDGES & 12' OC FIELD. R-38 CLOSED CELL SPRAY FOAM ROOF INSULATION. ROOFING OVER AN APPROVED WEATHER - RESISTIVE BARRIER - SEE EXTERIOR ELEVATIONS FOR ROOFING TYPES. PROVIDE ICE AND WATER SHIELD FIRST 6FT OF EAVES AND 3' EACH SIDE OF VALLEYS & RISING WALLS. (SEE STRUCTURAL)

PROTECT EXPOSED BEAM TAILS W/24GA BLACK ANODIZED SHT METAL CAPS W/1' TURN DOWN EDGES.

ALL OPENINGS IN BEARING WALLS SHALL HAVE A 6×12 HEADER w/(2) 2×6 TRIMMERS @ OPENINGS 4FT OR GREATER UON.

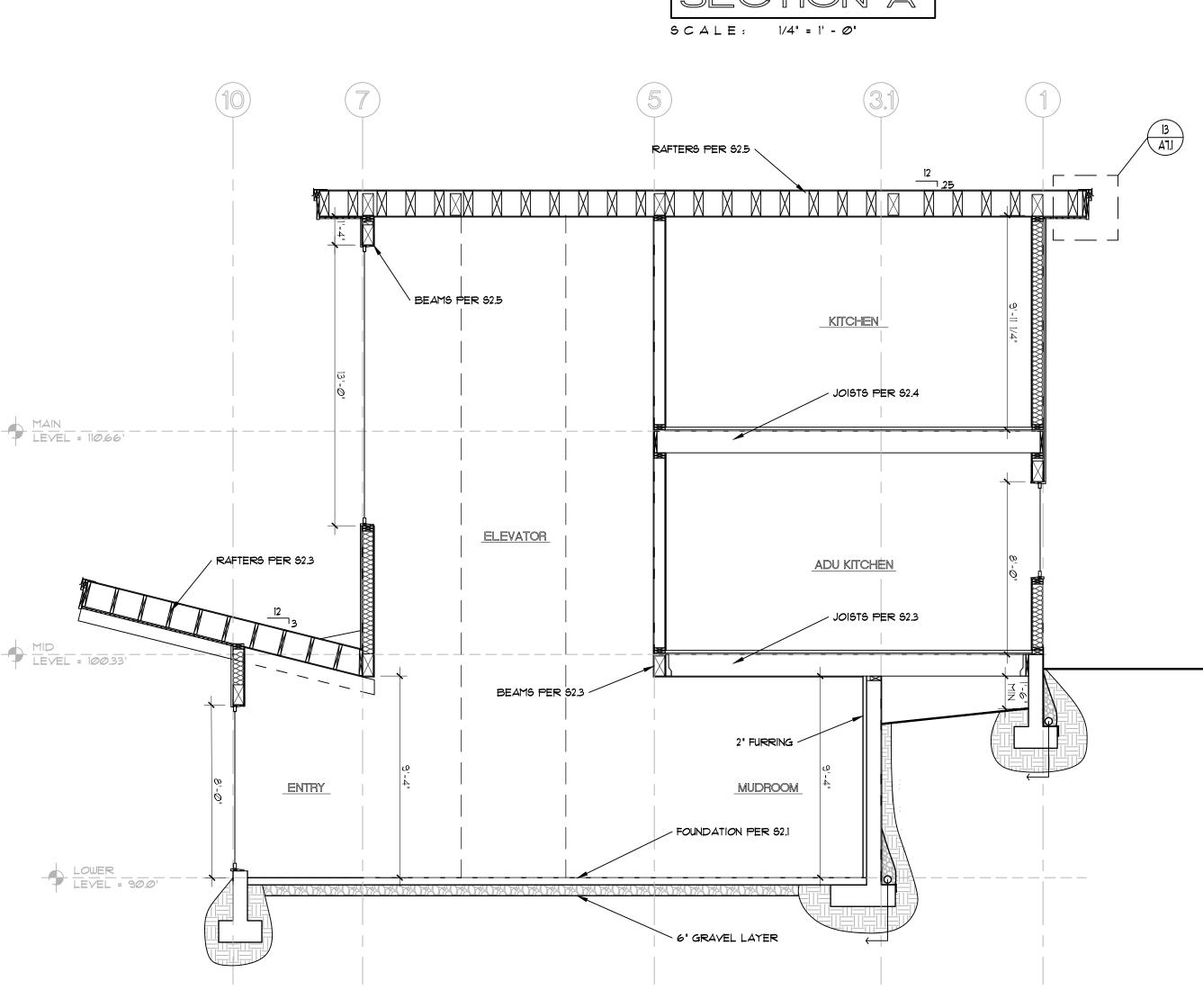
ALL FRAMING LUMBER SHALL BE DOUGLAS FIR - LARCH \*2 OR BETTER 2" TO 4" WIDE AND DF-L #I OR BETTER 5" AND WIDER,

PROVIDE FOUNDATION VENTILATION. NET OPENINGS SHALL BE NOT LESS THAN 6'x14", PLACED NOT TO EXCEED 25 LINEAR FEET OF EXTERIOR WALL. VAPOR BARRIERS SHALL BE INSTALLED WITHIN ALL CRAWL SPACES. VENTS SHALL BE LOCATED CLOSE TO CORNERS ON OPPOSITE SIDES & COVERED W/ FIRE RESISTIVE CORROSION PROOF METAL MESH (OPENINGS NOT GREATER THAN 3"). NET OPENINGS SHALL BE NOT LESS THAN 1SF FOR EACH 150 SF OF AREA WITH CROSS VENTILATION. (VULCAN VENT OR EQUAL)

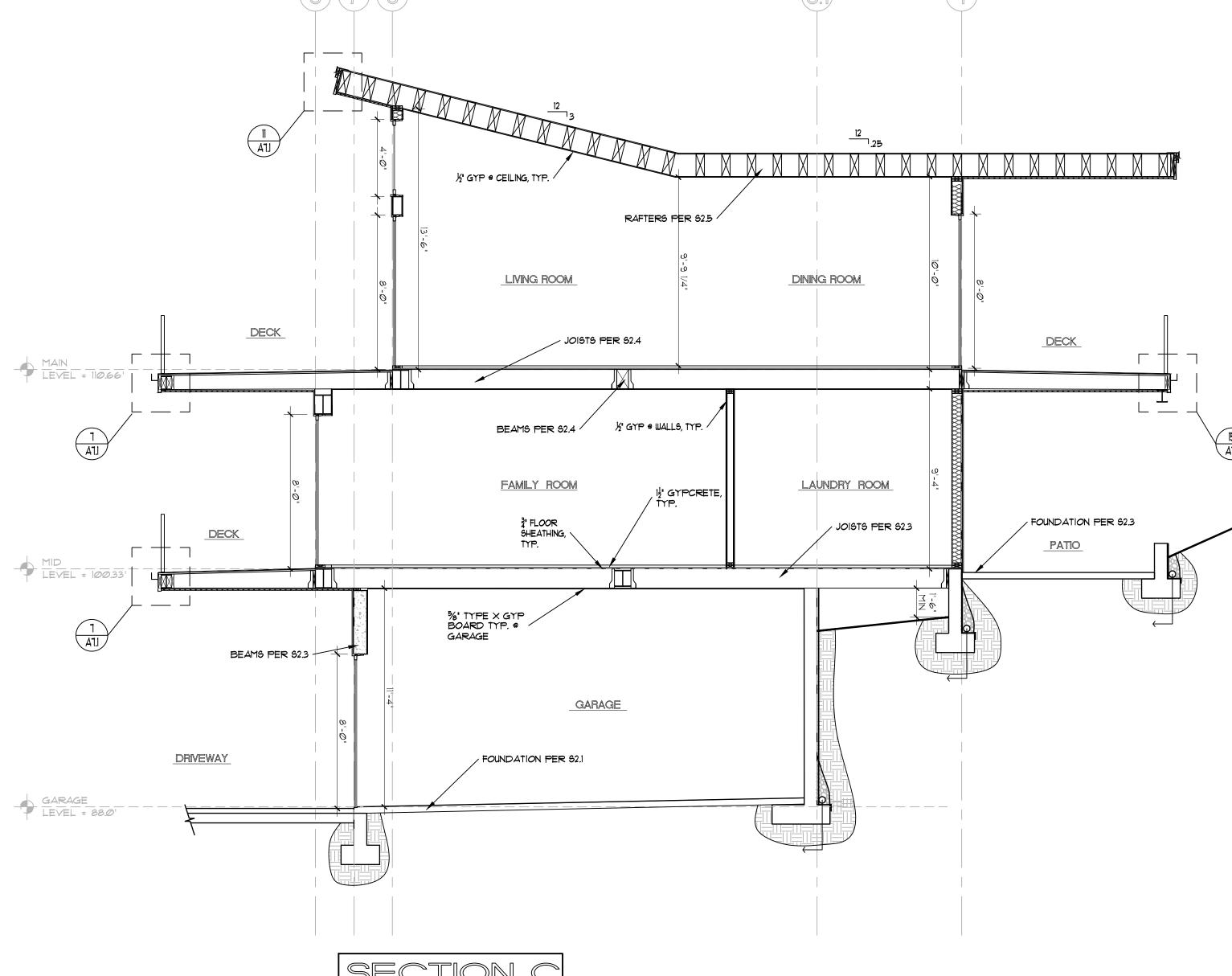
POSTS SHALL BE PROTECTED 6" ABOVE GROUND WITH A CONCRETE PEDESTAL.

ALL CONCRETE FOUNDATION WALLS BELOW GRADE: INSTALL GCP BITUTHENE 4000 MEMBRANE OVER SURFACE CONDITIONER OVER FOOTINGS, UP WALLS TO 6' ABOVE GRADE. INSTALL PER MANUF. SPEC.

INSTALL GCP HYDRO DUCT 220 OVER MEMBRANE & BACKFILL W/DRAIN ROCK, INSTALL 4" PERF. PIPE @ BASE OF WALL. SLOPE TO



S C A L E : 1/4" = 1' - 0"







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2 10-24-24 FIELD CHANG

596 TYNER

DATE: 06/03/24

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CUSTOM RESIDENCE FOR:

WHISPER HON, 89451
LOT 17, BLOCK F, INCLINE VILLAGE 4, WASHOE, NEVADA

REVISIONS

10-24-24 FIELD CHANGE

FILE: 596 TYNER

DATE: 06/03/24

SCALE: AS NOTED

DRAWN: AGB

SHEET:

OF 77 SHEETS

S C A L E: 1/4" = 1' - 0"

## STRUCTURAL GENERAL NOTES

#### 1. GENERAL.

- 1. Structural general notes apply to the structural documents for the Whisper Homes residence at 596 Tyner Way and may not be used on any other project.
- 2. Structural documents apply to the Whisper Homes residence at 596 Tyner Way and may not be used on any other project without consent in writing from Structured Engineering, LLC.
- 3. All workmanship, design and materials shall conform to the 2018 edition of the International Building Code.
- 4. Building geometry has been provided by others. Contractor shall review drawings prior to construction for discrepancies and notify architect and Structured Engineering prior to construction for clarifications or revisions. Any work performed by contractor after discovery of discrepencies without notice to architect and Structured Engineering shall be at builder's own risk.
- 5. Refer to Architectural Drawings for any architectural elements such as flashing, door and window details, finishes, waterproofing, drainage, ventilation, site preparation, and insulation. Refer to the Mechanical and Electrical drawings for piping, ductwork, conduit, fixtures and general layouts of systems.
- 6. Builder shall verify Structural Details match Architectural Elevations and Sections.
- 7. The builder shall be responsible for all job site safety including bracing and shoring during construction.
- 8. The builder shall verify Structural Dimensions match the Architectural Drawings prior to construction. The builder shall notify the architect and Structured Engineering for clarifications and/or revisions. Any work performed by contractor after discovery of discrepancies without notice to architect and Structured Engineering shall be a builder's own risk.

#### 2. Design Criteria

Ground S.L. = 247 psfRoof S.L. = 190 psfThermal Coefficient = 1.1 Exposure Coefficient = 1.0Snow importance factor = 1.0Eave S.L. = 285 psfRoof D.L. = 15 psfFloor LL = 40 psfFloor DL = 25 psfSoil Bearing Capacity = 3000 psf Wind Speed Vult = 120 mph Wind Exposure Factor = CWind Importance Factor = 1.0

Site Elevation < 6900 feet.

Seismic Ss = 1.695Seismic S1 = .577Seismic Sds = 1.13Seismic Sd1 = null Seismic Importance Factor = 1.0Seismic Site Class = D Seismic Design Category = D Occupancy Category = II

Basic Seismic force resistance system = Bearing Wall System Response Modification Factor = 6.5Seismic Governs Lateral

#### 3. SOILS (FOUNDATION).

- 1. Allowable soil bearing capacity = 3000 psf.
- 2. All foundations shall be placed in native undisturbed soil or approved engineered backfill. Backfill supporting structural foundations shall be continually inspected for placement of lift and compaction.
- 3. All backfill supporting footings and slabs shall be compacted to a minimum of 95% relative density.
- 4. Underlayment of slabs shall be designed by others.
- 5. Drainage shall be designed by others. Waterproofing against concrete retaining walls shall be designed by
- 6. Builder shall not backfill against retaining walls until concrete has reached 28 day strength and any required
- 7. Any over-excavation of soil for the placement of foundation shall be filled with concrete.
- Stepped footings shall be placed with a maximum vertical step of 2 feet and a minimum horizontal run of 4 feet. Contact Structured Engineering if an alternate is required for the project.

#### 4. CONCRETE

- 1. All new concrete construction and design shall conform to the 2018 edition of the International Building Code (IBC) and the most current ACI edition.
- 2. Concrete shall be regular weight concrete with a 28 day compressive strength of no less than 3000 psi. Design based on 28 day compressive strength of 2500 psi (special inspection is not required).
- 3. Construction joints shall be determined by others, however the recommended spacing of joints shall be no greater than 12'-0" o.c.
- 4. Additives containing Calcium Chloride shall not be used.
- 5. Concrete shall be inspected as required according to Table 1704.4 of the IBC. See page S1.2 for tables of special inspections.

#### 5. REINFORCING STEEL

- 1. Reinforcement bars #5 or less shall be grade 40 steel, ASTM A615 and shall be fabricated and placed in accordance with the most current ACI standards and these structural documents. For hooked bars, the straight extension must be at least 12 bar diameters. Minimum lap splice must be 40 bar diameters.
- 2. Reinforcement bars #6 or greater shall be grade 60 steel, ASTM A615 and shall be fabricated and placed in accordance with the most current ACI standards and these structural documents. For hooked bars, the straight extension must be at least 20 bar diameters. Minimum lap splice must be 50 bar diameters.
- 3. Reinforcement shall be placed as shown in the structural documents and shall be tied and supported per the most current ACI standards.
- 4. Welded reinforcement shall conform to ASTM A706.
- 5. Unless noted in the structural documents, minimum horizontal and vertical reinforcement must satisfy the minimum requirements of ACI 318.

#### 6. STRUCTURAL STEEL

- 1. Structural W-sections shall be grade 50 steel and conform to ASTM A572.
- HSS sections (square and rectangle) shall be 46 ksi steel and conform to ASTM A500, grade B.
- Structural steel channels, plates, and angles shall conform to ASTM A36.
- 4. Steel pipe shall conform to ASTM A53, grade B.
- 5. All new structural steel exposed to weather shall be hot-dipped galvanized per ASTM A153.
- 6. Fabrication and erection of structural steel shall conform to the latest AISC specifications.
- 7. All welding shall conform to the most current American Welding Society specifications by certified welders and verified by an approved special inspector.
- Welds shall have special inspection as required by IBC 1704.3.1.
- 9. Electrodes shall have a minimum tensile strength of 70 ksi (Innershield NR-211 not allowed).
- 10. Unless noted otherwise, bolts shall be A307 Machine Bolts (M.B.), special inspection not required.
- 11. High strength bolts shall be A325 and shall conform to ASTM A325 and shall be installed per AISC Specifications for Structural Joints. Special inspection is required for High Strength bolts per IBC 1704.3.3.
- 12. Threaded rods shall conform to ASTM A36.
- 13. Unless noted in details, provide  $\frac{1}{2}$ " diameter stud bolts at 32" o.c. at wood studs to steel columns, where
- 14. See sheet S1.2 for steel special inspection table 1704.3.

#### 7. WOOD FRAMING / CARPENTRY

- 1. All wood framing and design shall conform to the most current NDS (National Design Standards) for wood
- 2. Sawn lumber for beams 6x or greater shall be Douglas Fir #1 (UNO)
- Sawn lumber for rafters, joist, ledgers, and beams 4x or less shall be Douglas Fir #2 (UNO). Studs and plates may be construction grade.
- Sawn lumber attached to concrete or concrete masonry at exterior or wet conditions shall be pressure treated with ACQ, CA-B or CCA applications. Stainless steel fasteners are required for all nails, bolts, hangers and straps that attach to P.T. wood at these locations.
- Sawn lumber attached to concrete at interior dry conditions such as mudsills or interior ledgers are permitted to be Zinc Borate treated wood. Hot dipped galvanized fasteners are permitted at these locations.
- 6. LVL (laminated veneer lumber) shall be rated at 2600 psi bending, 285 psi shear, 1.8E and 750 psi compression perpendicular to grain. At exterior conditions, LVL material must be protected by soffits.
- 7. PSL (parallel strand lumber) shall be rated at 2900 psi bending, 290 psi shear, 2.0E and 750 psi compression perpendicular to grain. At exterior conditions, PSL material must be protected by soffits or rated for exterior
- 8. LSL (laminated strand lumber) shall be rated at 1700 psi bending, 285 psi shear, and 1.3E. LSL is not permitted to be used for rafters, studs and beams unless otherwise noted in structural drawings. At exterior conditions, LSL material just be protected by soffits.
- GLB (glulam beam) shall be 24F-V4 and rated at 2400 psi bending, 240 psi shear, 1.8E, and 650 psi compression perpendicular to grain (UNO). At exterior conditions, GLB shall be Western Cedars and exterior rated or it shall be protected by moisture in a soffit or other waterproofing per architect.
- 10. See framing and diaphragm plans for sheathing callouts. Sheathing shall be manufactured in accordance with APA PS 1-95. Exterior shear ply shall be minimum \( \frac{3}{8} \)" LP FlameBlock Fire-Rated OSB sheathing.
- 11. All metal hardware shall be Simpson Strong Tie or equivalent.
- 12. All trusses and rafters shall align with stud below where they bear on walls.
- 13. All nailing not referenced in structural documents shall be in accordance with IBC table 2304.9.1.
- 14. All nail holes in hangers must be used to achieve "Max" rating called out in Simpson Strong Tie.
- 15. Anchor bolts at mudsill must be a minimum of  $\frac{5}{8}$ " dia. x 12" A.B. at 48" o.c. with 3"x3"x $\frac{1}{4}$ " plate washers
- (UNO). A minimum of (2) anchor bolts must be installed at each mudsill.
- 16. All mudsills shall be a minimum of 3x pressure treated (UNO).
- 17. All machine bolts shall be A307 bolts (UNO).
- 18. Beams may bear directly on posts with pocket connection or toenails per structural drawings without steel plates/buckets unless noted in structural drawings. If "CPG" is noted on drawings, see referenced details for
- 19. Posts may bear directly on wood framing unless "CPG" is noted in structural drawings. See referenced details for attachment of post to support below if "CPG" is noted. At corner walls where no mudsill occurs and post bears directly on concrete wall, use a minimum of PB hardware to attach post to foundation.
- 20. Posts may splice in floor cavities per structural drawings and do not need to be full height unless required in
- 21. All wood sizes called out in structural drawings are the minimum sizes required. All wood members can be upsized as required per architectural. Some revisions to details may be required. Builder shall notify Structured Engineering if alternate sizes are requested.
- 22. All nuts at connections shall be re-tightened prior to covering the connection. This includes but is not limited to holdown bolts, anchor bolts, wood to steel connections, and custom wood to wood connections.
- 23. Nails at shear walls shall be "common" or "box" nails per the shear wall chart on Sheet S1.2.
- 24. Nails at diaphragms shall be "common" nails per the structural drawings.
- 25. Nails specified in details shall be "common" nails.
- 26. .128" dia. nails shall have a minimum penetration of  $1-\frac{3}{8}$ " (UNO) .148" dia. nails shall have a minimum penetration of  $1-\frac{1}{2}$ " (UNO) .162" dia. nails shall have a minimum penetration of 1-5/8" (UNO)

#### 8. EPOXY

- 1. Epoxy must be Simpson XP epoxy. Special inspection is required.
- 2. Follow Simpson Strong Tie specifications to prepare hole for epoxy.

## STRUCTURAL ABBREVIATIONS

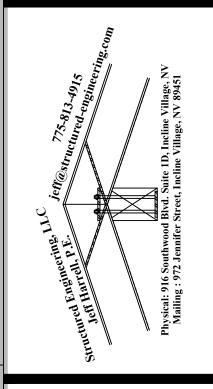
<u>@</u>	AT	JST	JOIST
/	PER (NAIL/FT = NAIL PER FOOT)	JT	JOINT
AB	ANCHOR BOLT	KB	KNEE BRACE
ABV	ABOVE	KP	KING POST
ACI	AMERICAN CONCRETE	KS	KING STUD
	INSTITUTE		
AFF	ABOVE FINISH FLOOR	LL	LIVE LOAD
AISC	AMERICAN INSTITUTE OF STEEL	LONG	LONGITUDINAL
	CONSTRUCTION	LSL	LAMINATED STRAND LUMBER
ALT	ALTERNATE	LVL	LAMINATED VENEER LUMBER
AWS	AMERICAN WELDING SOCIETY		
		MAX	MAXIMUM
BD	BOARD	MB	MACHINE BOLT
BLK	BLOCK	MIN	MINIMUM
BLK'G	BLOCKING	MTL	METAL
BLW	BELOW		
BM	BEAM	(N)	NEW
B.O.	BOTTOM OF	NÓ	NUMBER
BRG	BEARING	N/A	NOT APPLICABLE
BTM	BOTTOM	NS	NEAR SIDE
		NTS	NOT TO SCALE
CJ	CONTROL JOINT		
CL	CENTERLINE	O/	OVER
CLR	CLEAR	OC	ON CENTER
CMU	CONCRETE MASONRY UNIT	OF	OUTSIDE FACE
COL	COLUMN	OH	OVERHANG
CONC.	CONCRETE	OL	OUTLOOKER
CONT'D	CONTINUED		
CPG	CONTINUOUS PARALLEL TO GRAIN	PB	PITCH BREAK
	(POST MAY NOT BEAR DIRECTLY	PKT	POCKET
	ON WOOD MEMBER WITHOUT	PLF	POUND PER FOOT
	BEARING PLATE)	PLWD	PLYWOOD
		PSF	POUND PER SQUARE FOOT
d	NAIL SIZE	PSI	POUND PER SQUARE INCH
DBL	DOUBLE	PSL	PARALLEL STRAND LUMBER
DIA	DIAMETER	PT	PRESSURE TREATED
DIAG	DIAGONAL		
DIM	DIMENSION	RDWD	REDWOOD
DECK'G	DECKING	REINF	REINFORCEMENT
DTL	DETAIL	REF	REFERENCE
212	222	REQ'D	REQUIRED
(E)	EXISTING	REV	REVISION
EA	EACH	RFTR	RAFTER
EL	END LENGTH		
ELEV	ELEVATION	SEC	SECTION
EMBED	EMBEDMENT	SHT	SHEATHING
ES	EACH SIDE	SIM	SIMILAR
EXT	EXTERIOR	SOG	SLAB ON GRADE
		STAG'D	STAGGERD
FDN	FOUNDATION	STD	STANDARD
FF	FINISH FLOOR	STL	STEEL
FLR	FLOOR		
FP	FULL PENETRATION	T&G	TOUNGE AND GROOVE
FS	FAR SIDE	T&B	TOP AND BOTTOM
FT	FOOT	THRD	THREADED
FTG	FOOTING	TO	TOP OF
110	1001110	TRMR	TRIMMER
GA	GAGE	TRANS	TRANSVERSE
GLB	GLULAM BEAM	TYP	TYPICAL
GLD	0202.11.1 22.11.11		
HD	HOLDOWN	UNO	UNLESS NOTED OTHERWISE
HDR	HEADER		
HF	HARDY FRAME	VERT	VERTICAL
HGR	HANGER		<del>-</del>
HORIZ	HORIZONTAL	W/	WITH
HSS	HOLLOW STRUCTURAL TUBE	WP	WATERPROOF
IBC	INTERNATIONAL BUILDING CODE		
IDC IE	INTERNATIONAL BUILDING CODE		

INSIDE FACE

INTERIOR



r Homes LLC Fyner Way Village, NV



REVISIONS:

/1\ 11-6-2024 REVISIONS 2 5-7-2025 REVISIONS 3 8-18-2025 REVISIONS

PROJECT NO: 240601 ISSUE DATE: August 23, 2024

> DRAWING TITLE:

> > GENERAL NOTES & ABBREVIATIONS

DRAWING NO:

## SPECIAL INSPECTION SCHEDULES

#### Table 1705.3 (Required Verification and Inspection of Concrete Construction)

Verification & Inspection	Continuous	Periodic	Referenced Standard	IBC/CBC Reference	Req'd for Project?
1. Inspection of reinforcing steel including prestressing tendons, and placement.		×	ACI 318 : 3.5, 7.1-7.7	1913.4	NO
2. Inspection of reinforcing steel welding in accordance with Table 1704.3, Item 5b.			AWS D1.4, ACI 318 : 3.5.2		NO
3. Inspect bolts to be installed in concrete prior to and during placement of concrete where allowable loads have been increased.	×			1911.5	NO
4. Verify use of required design mix		×	ACI 318 : ch. 4, 5.2 - 5.4	1904.2.2, 1913.2, 1913.3	NO
5. At the time fresh concrete is sampled to fabricate specimens for strength tests, perform slump and air content tests and determine the temperature of the concrete.	*		ASTM C 172 ASTM C 31 ACI 318 : 5.6, 5.8	1913.10	NO
6. Inspection of concrete and shotcrete placement for proper application techniques.	×		ACI 318 : 5.9, 5.10	1913.6, 1913.7, 1913.8	NO
7. Inspection for maintenance of specified curing temperature and technique.		×	ACI 318 : 5.11-5.13	1913.9	NO
8. Inspection of prestressed concrete: a: Application of prestressing forces b: Grouting of bonded prestressing tendons in the seismic-force-resisting system.	×		ACI 318 : 18.20 ACI 318 : 18.18.4		NO
9. Erection of precast concrete members		*	ACI 318 : Ch. 16		NO
10. Verification of in-situ concrete strength, prior to stressing of tendons in posttensioned concrete and prior to removal of shores and forms from beams and structural slabs.		×	ACI 318 : 6.2		NO
11. Inspect formwork for shape, location and dimensions of the concrete member being formed.		×	ACI 318 : 6.11		NO

#### Verification and Inspection of Epoxy Anchors & Titan Bolts

Verification & Inspection Task	Continuous During Task Listed	Periodically During Task Listed	Required for this project
1. Depth		×	YES
2. Placement		×	YES
3. Cleanout	<del></del>	×	YES
4. Temp of Concrete or CMU		×	YES
5. Temperature of Epoxy		×	YES

#### 1. DEFINITIONS OF SPECIAL INSPECTIONS AND OBSERVATIONS

- 1. Special inspection is defined in section 1702 of the IBC/CBC as "inspections as herein required of the materials, installation, fabrication, erection or placement of components and connections requiring special expertise to ensure compliance with approved construction documents and referenced standards". Requirements for this project are listed below.
- 2. Structural observations is defined in section 1702 of the IBC/CBC as "The visual observation of the structural system by a registered design professional for general conformance to the approved construction documents at significant construction stages and at completion of the structural system. Structural observation does not include or waive the responsibility for the inspection required by Section 109, 1704 or other sections of this code". Structural observation IS required for this project.
- Seismic inspection for wood construction is defined in section 1707.3 and states "Continuous special inspection is required during field gluing operations of elements of the seismic-force-resisting system. Periodic special inspection is required for nailing, bolting, anchoring and other fastening of components within the seismic-force-resisting system, including wood shear walls, wood diaphragms, drag struts, braces, shear panels and hold-downs. Seismic inspection is not required for this project because it qualifies as exempt per section 1705. Where inspection is required the owner shall employ Structured Engineering, LLC in a separate contract to perform the special
- inspection for the seismic-force-resisting system.

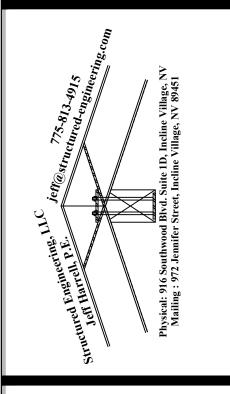
#### Required Verification and Inspection of Steel Construction

Verification & Inspection	Continuous	Periodic	Referenced Standard	IBC/CBC Reference	Req'd for Project?
Material verification of high-strength botls, nuts and washers:					-
a. Identification markings to conform to ASTM standards specified in the approved construction documents.		×	Applicable ASTM material specifications: AISC 360, section A3.3		NO
b. Manufacturer's certificate of compliance required.		×			NO
2. Inspection of high-strength bolting:					
a. Bearing-type connections		×	AISC 360, Section M2.5	1704.3.3	YES
b. Slip-critical connections	×	×	Alse 500, section W.2.5	1704.3.3	NO
3. Material verification of structural steel:					
a. Identification markings to conform to ASTM standards specified in the approved construction documents.			ASTM A6 or ASTM A 568	1708.4	NO
b. Manufacturer's certified mill test reports					NO
4. Material verification of weld filler materials:					
a. Identification markings to conform to AWS specification in the approved construction documents.			ASTM A6 or ASTM A 568	1708.4	NO
b. Manufacturers' certificate of compliance required.					NO
5. Inspection of welding:					
a. Structural Steel:					NO
1: Complete and partial penetration groove welds.	×				NO
2: Multipass fillet welds:	×		AWG D1.1	1704.2.1	YES
3: Single-pass fillet welds $> \frac{5}{16}$ "	*		AWS D1.1	1704.3.1	YES
4: Single-pass fillet welds $<$ or $=\frac{5}{16}$ "		×			YES
5: Floor and deck welds		×	AWS D1.3		NO
b. Reinforcing Steel:					NO
1: Verification of weldability of reinforcing steel other than ASTM A 706.		×			NO
2: Reinforcing steel-resisting flexural and axial forces in intermediate and special moment frames, and boundary elements of special reinforced concrete shear walls and shear reinforcement.	×		AWS D1.4 ACI 318: 3.5.2		NO
3: Shear reinforcement	×				NO
4: Other reinforcing steel		×			NO
6. Inspection of steel frame joint details for compliance with approved construction documents:  a. Details such as bracing and stiffening b. Member locations c. Application of joint details at each connection		*		1704.3.2	NO

#### Table 1705.6 (Required Verification and Inspection of Soils)

Verification & Inspection Task	Continuous During Task Listed	Periodically During Task Listed	Required for this project
Verify materials below footings are adequate to achieve the design bearing capacity.		*	YES
2. Verify excavations are extended to proper depth and have reached proper material.		×	YES
3. Perform classification and testing of controlled fill materials.		*	YES
4. Verify use of proper materials, densities and lift thicknesses during placement of compaction of controlled fill.	*		YES
5. Prior to placement of controlled fill, observe subgrade and verify that site has been prepared properly.		*	YES





REVISIONS: 11-6-2024 REVISIONS 2 5-7-2025 REVISIONS

3 8-18-2025 REVISIONS

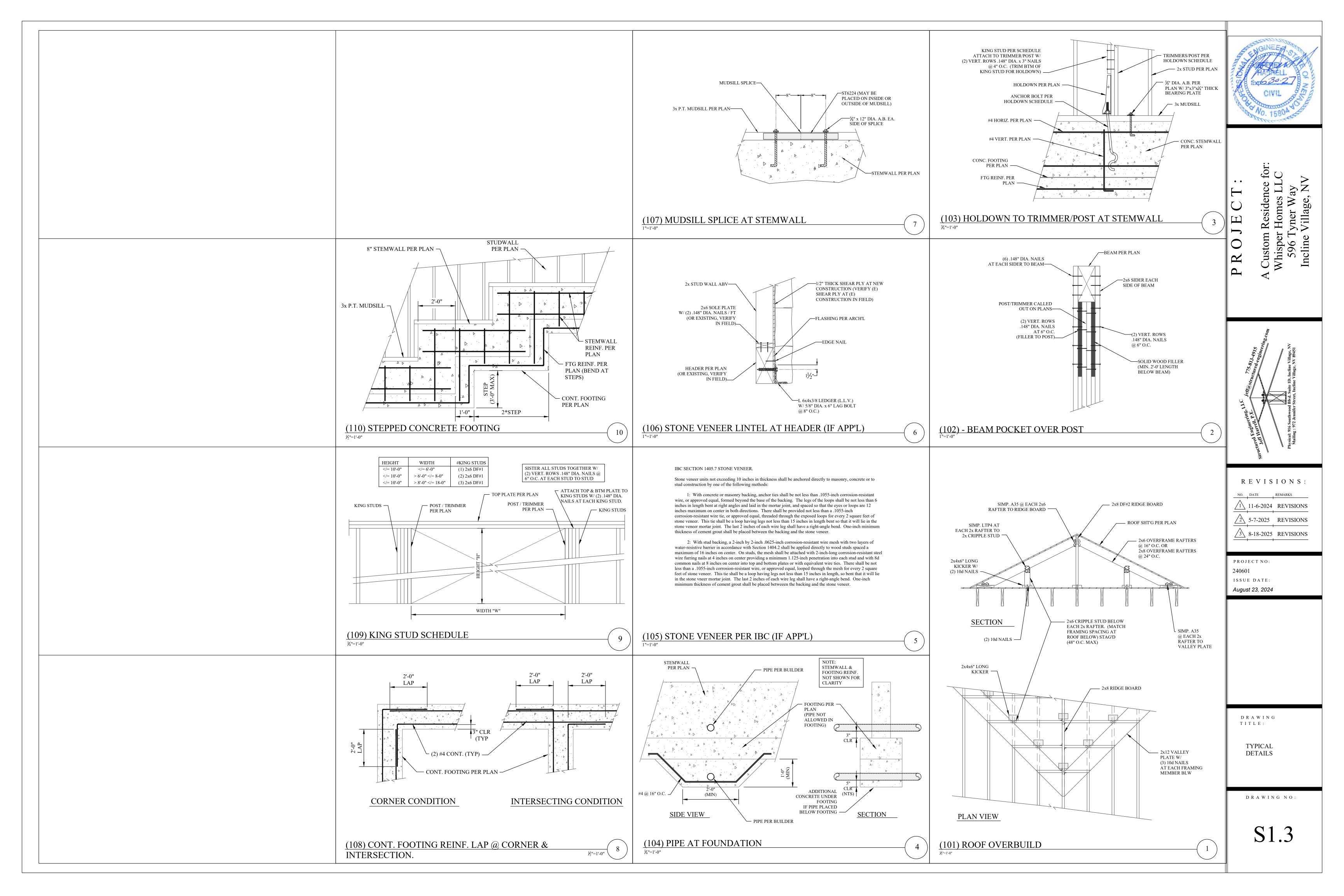
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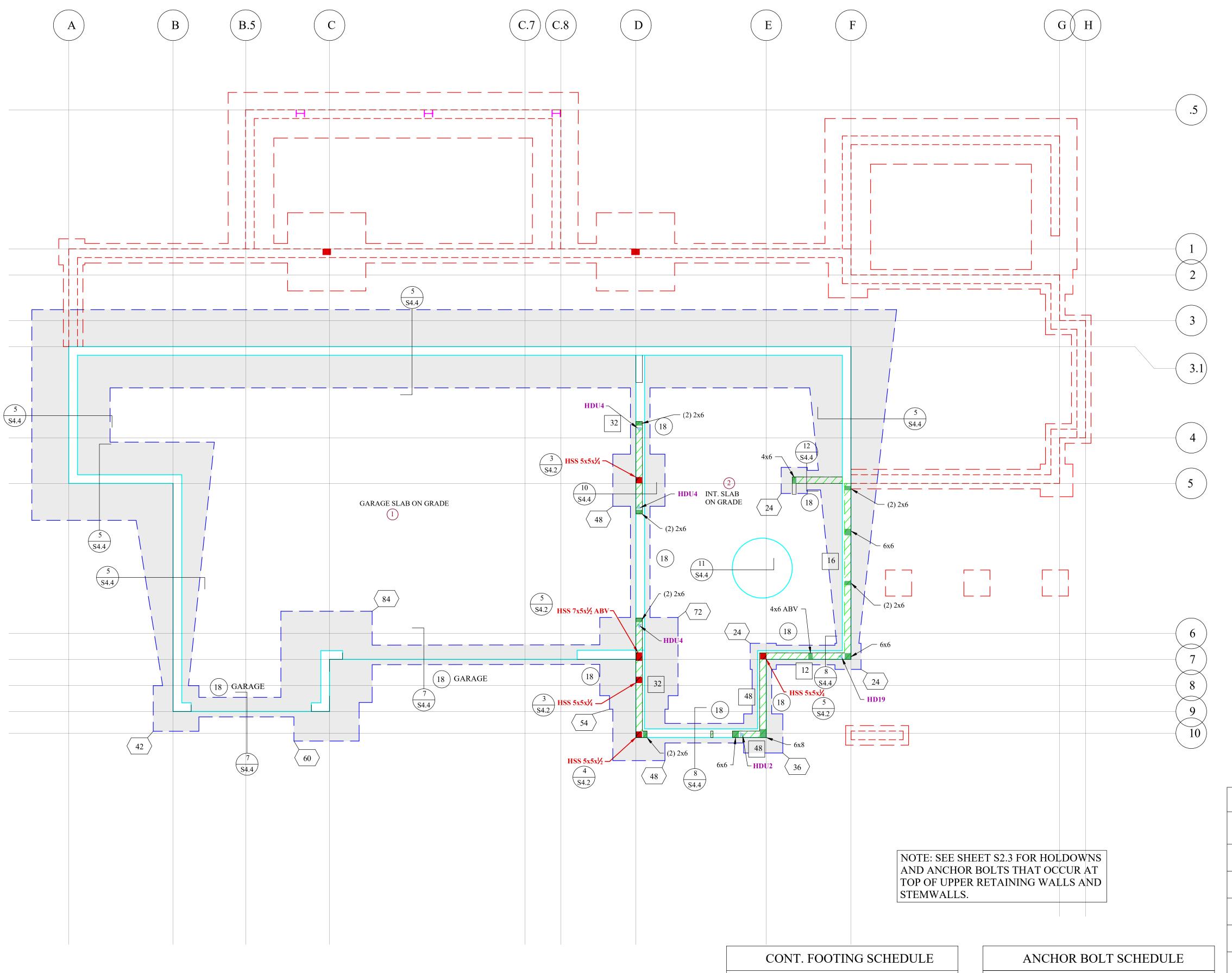
> DRAWING TITLE:

> > **SPECIAL** INSPECTION **SCHEDULES**

DRAWING NO:

S1.2





18

(18) GARAGE

18"x9" THICK CONT. FOOTING

W/ (2) #4 BARS CONTINUOUS.

18"x24" DEEP CONT. FOOTING AT

W/ (2) #4 BARS CONTINUOUS TOP &

GARAGE STEMWALL OPENING

BTM

#### FOUNDATION NOTES

- 1. FOOTINGS SHALL BE PLACED IN NATIVE NON-DISTURBED
- 2. MINIMUM FOOTING DEPTH FOR FROST PROTECTION IS 24
- 3. CONTINUOUS FOOTING SHALL BE CAST IN PLACE CONCRETE AND SHALL BE 18" WIDE x 9" THICK WITH (2) #4 CONTINUOUS,
- 4. STEMWALLS SHALL BE CAST IN PLACE CONCRETE AND SHALL BE 8" WIDE WITH #4 VERTICAL BARS AT 24" O.C. AND #4 HORIZONTAL BARS AT 24" O.C. (MINIMUM (2) HORIZONTAL
- 5. ANCHOR BOLTS SHALL BE  $\frac{5}{8}$ " DIA. x 12" A.B. AT 48" O.C. (U.N.O.) ALL ANCHOR BOLTS SHALL HAVE 3"x3"x1/4" PLATE
- 6. MUDSILL SHALL BE (MIN) 3x6 P.T. (ZINC BORATE).
- 7. POSTS MAY BEAR DIRECTLY ON MUDSILL WITHOUT HARDWARE UNLESS NOTED BY "CPG".
- 8. PROVIDE SIMP. PB66 AT ALL CORNER 6x6 POSTS WHERE NO MUDSILL OCCURS.
- 9. ALL PERIMETER FOOTINGS MUST BE "KEYED" INTO NATIVE UNDISTURBED SOIL. BUILDER SHALL CONTACT STRUCTURED ENGINEERING, LLC IF OPTION IS REQUIRED.
- 10. WIDEN FOOTINGS AS REQUIRED FOR STONE VENEER.

#### S2.1 KEYNOTES

- GARAGE SLAB ON GRADE SHALL BE 4" THICK CONCRETE SLAB WITH #3 BARS AT 18" O.C. EACH WAY. ALT: W 10-10 6x6 WELDED WIRE MESH. UNDERLAYMENT DESIGNED BY
- INTERIOR SLAB ON GRADE SHALL BE 4" THICK WITH #3 BARS AT 18" O.C. EACH WAY. ALT: W 10-10 6x6 WELDED WIRE MESH. UNDERLAYMENT DESIGNED BY OTHERS.

PAD	FOOTING SCHEDULE
24	24"x24"x10" THICK PAD FOOTING
24	W/(3)#4 BARS EACH WAY.
725	36"x36"x12" THICK PAD FOOTING
$\langle 36 \rangle$	W/(5) #4 BARS EACH WAY.
	42"x42"x12" THICK PAD FOOTING
42	W/ (6) #4 BARS EACH WAY.
	48"x48"x15" THICK PAD FOOTING
<u>\ 48</u>	W/ (7) #4 BARS EACH WAY.
<u></u>	54"x54"x15" THICK PAD FOOTING
54	W/ (8) #4 BARS EACH WAY.
	60"x60"x18" THICK PAD FOOTING
$\langle$ 60 $\rangle$	W/ (9) #4 BARS EACH WAY.
	TOP & BTM
	72"x72"x21" THICK PAD FOOTING
/ 72 \	W/(11)#ADADSEACHWAV

W/ (11) #4 BARS EACH WAY.

W/ (13) #4 BARS EACH WAY.

84"x84"x24" THICK PAD FOOTING

TOP & BTM

TOP & BTM

HOLDOWN SCHEDULE						
HOLDOWN CALLOUT	MINIMUM POST REQUIREMENT	SIMPSON STAB BOLT	THREADED ROD W/ DBL. NUT + WASHER	THREADED ROD W/ EPOXY		
HDU2	(2) 2x6	SSTB16	%" DIA. W/ 10" EMBED	%" DIA. W/ 10" EMBED		
HDU4	(2) 2x6	SSTB20	%" DIA. W/ 10" EMBED	%" DIA. W/ 12" EMBED		
HDU5	(2) 2x6	SSTB20	%" DIA. W/ 10" EMBED	%" DIA. W/ 12" EMBED		
HDU8	4x6	SSTB28	%" DIA. W/ 12" EMBED	$\frac{7}{8}$ " DIA. W/ 16" EMBED		
HDU11	6x6	N/A	1" DIA. W/ 14" EMBED	1" DIA. W/ 20" EMBED		
HDU14	6x6	N/A	1" DIA. W/ 18" EMBED	1" DIA. W/ 20" EMBED		
HD19	6x6	N/A	1-¼ " DIA. W/ 20" EMBED	N/A		

#### NOTES:

<sup>5</sup>/<sub>8</sub>" DIA. x 12" A.B. @ 12" O.C.

5/8" DIA. x 12" A.B. @ 16" O.C.

5⁄8" DIA. x 12" A.B. @ 24" O.C.

<sup>5</sup>/<sub>8</sub>" DIA. x 12" A.B. @ 32" O.C.

<sup>5</sup>/<sub>8</sub>" DIA. x 12" A.B. @ 48" O.C.

16

24

48

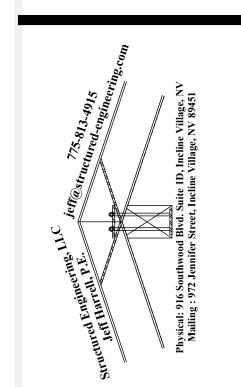
- Edge nail to both studs at (2) 2x6 callouts.
- Spike (2) 2x6 together with (2) vert. rows .148" dia. x3" nails @ 6" o.c.

 $\langle 72 \rangle$ 

 $\langle 84 \rangle$ 

- Install Simpson SSTB bolts per Simpson Specifications. Ensure proper orientation of bolts. Contractor shall ensure holdown bolts are attached to formwork prior to placement of concrete.
- Contact Engineer for inspection of Epoxy applications. Epoxy shall be Simpson Set-XP epoxy. Use threaded rod with double nut option at locations where embedment of SSTB bolts cannot be obtained.
- Embedment for threaded rods do not need to extend to footing.





REVISIONS:

NO. DATE /1\ 11-6-2024 REVISIONS

/2 5-7-2025 REVISIONS /3\ 8-18-2025 REVISIONS

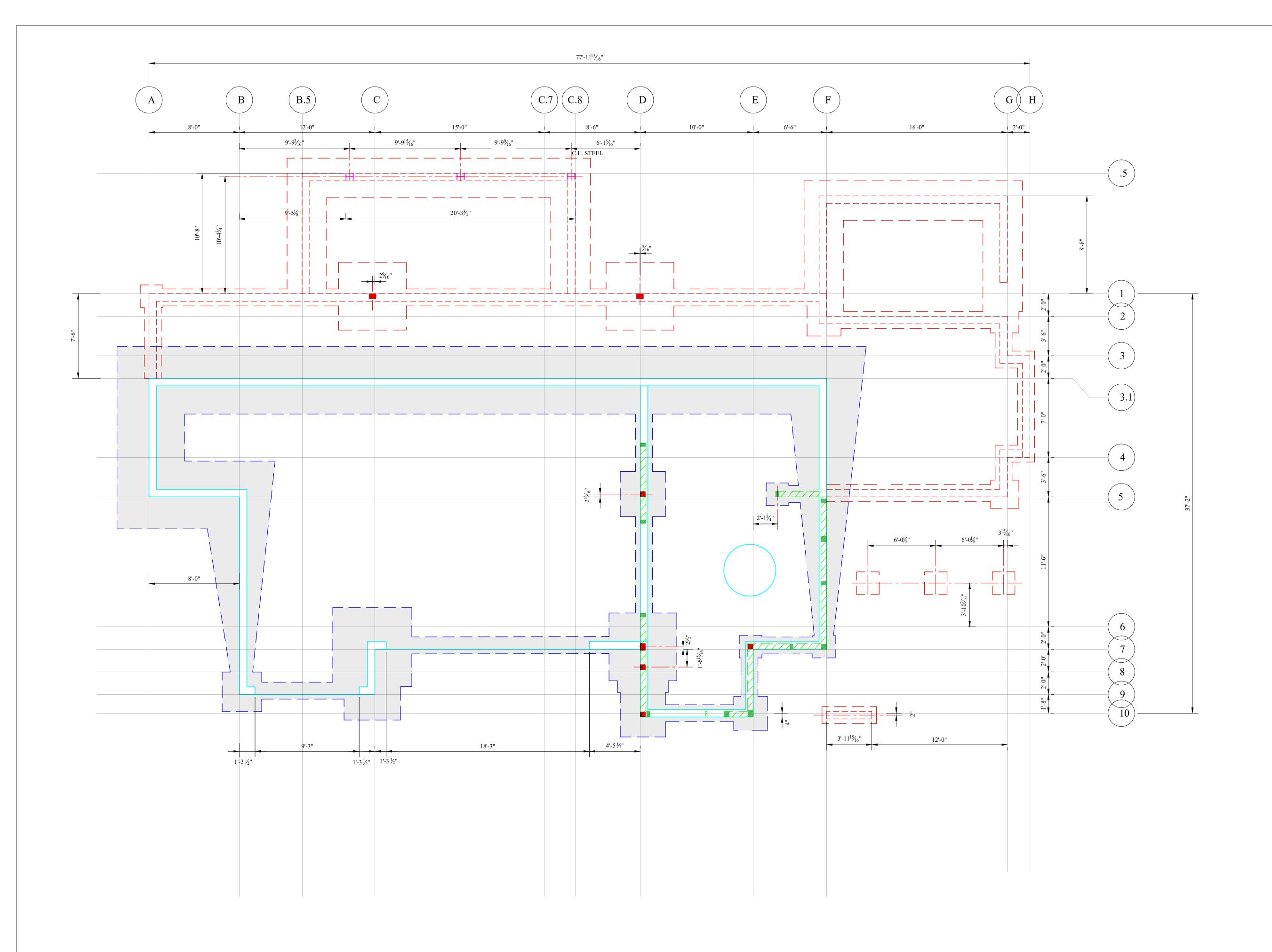
PROJECT NO: 240601 ISSUE DATE:

August 23, 2024

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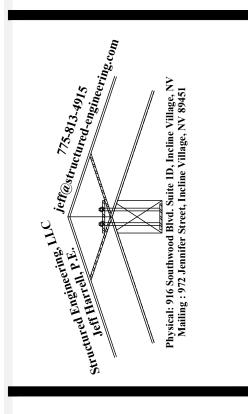
LOW FOUNDATION **PLAN** 

DRAWING NO:





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A Custom Residence for



REVISIONS:

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2 5-7-2025 REVISIONS
3 8-18-2025 REVISIONS

PROJECT NO:

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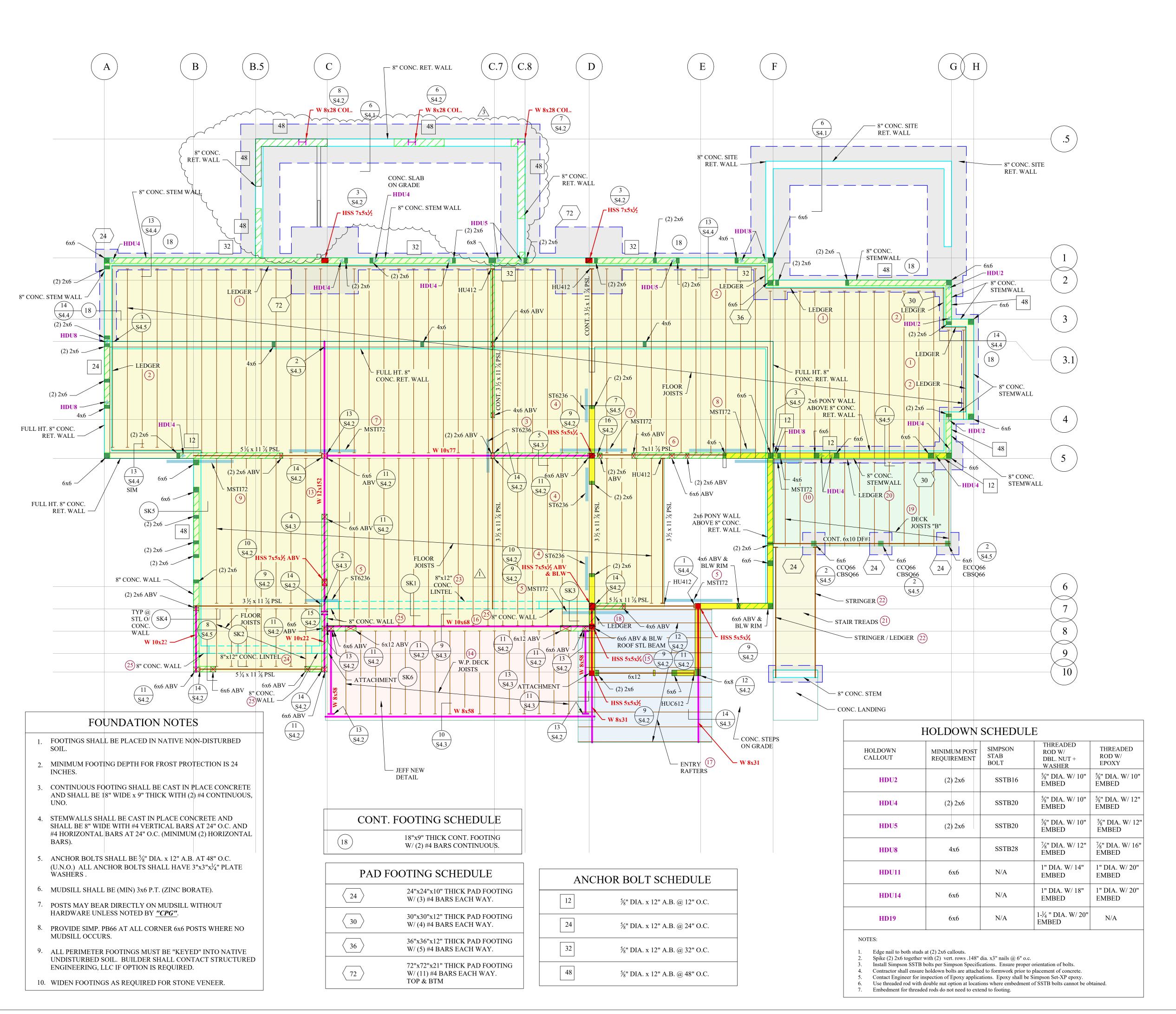
August 23, 2024

DRAWING TITLE:

FOUNDATION DIMENSION PLAN

DRAWING NO:

S2.2

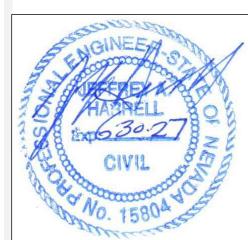


#### FLOOR FRAMING NOTES

- 1. FLOOR SHT'G SHALL BE ¾" APA RATED T & G SHT'G. GLUE & NAIL WITH 8d NAILS AT 6",10" O.C.
- 2. FLOOR JOISTS SHALL BE 11 ½" TJI 360 @ 16" O.C.
- JOIST HANGERS PERPENDICULAR TO BEAMS SHALL BE SIMP. IUS2.37 / 11.88 AT FACE MOUNT. AT TOP FLANGE CONDITIONS USE SIMP. ITS HANGERS (MAX).
- 4. PROVIDE STUD BELOW ALL JOISTS AT SUPPORTING WALLS.
- 5. EDGE NAIL FLOOR SHT'G TO ALL BLOCKING AND LATERAL COLLECTORS.
- 6. PROVIDE TJI JOIST OR BLK'G BELOW ALL PARTITION WALLS
- 7. PROVIDE DOUBLE LVL JOISTS BELOW ALL FIREPLACE HEARTHS.
- 8. MAXIMUM JOIST SPAN WITHOUT INTERMEDIATE TJI BLK'G IS

#### S2.3 KEYNOTES

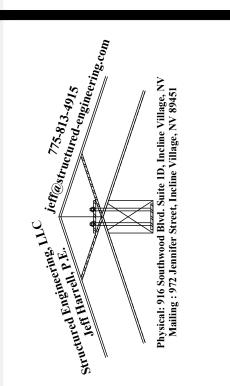
- 2x12 P.T. LEDGER W/ $\frac{5}{8}$ " DIA. x 6" TITAN BOLTS @ 8" O.C. (STAG'D)
- 2x12 P.T. LEDGER W/ $\frac{5}{8}$ " DIA. x 6" TITAN BOLTS @ 32" O.C. (STAG'D)
- (3) ST6236 AT TOP OF PSL TO TOP OF PSL.
- (4) ST6236 AT BTM OF PSL TO TOP OF TOP PLATE.
- MSTI72 AT TOP OF PSL TO TOP OF RIM JOIST AND BLK'G OVER WALL.
- 6 O.F. 7x11 % PSL = O.F. WALL ABOVE = GRID 5
- MSTI72 AT TOP OF PSL TO TOP OF STEEL BEAM. ATTACH TO STEEL BEAM WITH  $\frac{1}{16}$ " FILLET WELD ALL AROUND
- (8) MSTI72 AT BTM OF PSL TO TOP OF TOP PLATE.
- 9 MSTI72 AT BTM OF PSL TO TOP OF MUDSILL.
- (10) MSTI72 AT TOP OF TOP PLATE TO TOP OF MUDSILL
- MSTI72 AT TOP OF PSL TO TOP OF TOP PLATE. NO DOUBLE TOP PLATE OVER PSL.
- ST6236 AT TOP OF PSL TO TOP OF TOP PLATE. NO DOUBLE TOP PLATE OVER PSL.
- NOTE: W12x152 IS 13  $\frac{3}{4}$ " DEEP AND WILL STICK DOWN 1.875" BELOW BOTTOM OF 11  $\frac{7}{8}$ " TJI JOISTS.
- 1 ¾ x 9 ½ LVL W.P. DECK JOISTS @ 16" O.C. WITH SIMP. HU9 OR ITS HANGERS. SLOPE PER ARCH'L. SOFFIT REQ'D. SHT'G SHALL BE ¾" T&G APA RATED SHT'G. NAIL WITH 8d NAILS @ 6", 10" O.C. SEE ARCH'L FOR FLASHING, WATERPROOFING AND DECK SURFACE.
- (15) HSS 5x5x½ OCCURS BELOW W 10x68 STEEL BEAM
- 16 O.F. W10x68 = O.F. STUD WALL ABOVE = GRID 8
- ENTRY RAFTERS SHALL BE 1 ¾ x 9 ½ LVL @ 16" O.C. (SOFFIT REQ'D) SEE ARCH'L FOR ROOF SLOPE. HANGERS SHALL BE HU9 WHERE APP'L. ROOF SHT'G SHALL BE ½" 4½0 APA RATED ROOF SHT'G. NAIL WITH 8d NAILS @ 6", 12" O.C. SEE ARCH'L FOR FLASHING.
- (18) 1 3/4 x 9 1/2 LVL LEDGER W/ (3) 1/4" DIA. x 4 1/2" SDS SCREWS / STUD.
- DECK JOISTS "B" SHALL BE 4x8 DF#1 AT 16" O.C. WITH SIMP. HU48 HANGERS WHERE HUNG. DECKING SHALL BE 2x6 CLASS "A" DECKING PER ARCH'L.
- LEDGER SHALL BE 3x8 P.T. WITH  $(3)\frac{1}{4}$ " DIA. x 6" SDS SCREWS @ 8" O.C. TO RIM JOIST.
- 21 STAIR TREADS SHALL BE 4x12 DF#1 FLAT WITH SIMP. TA10Z TREAD SUPPORTS EA. END.
- 22) STAIR STRINGER SHALL BE 6x12 DF#1 WITH SIMP. HUCQ612 AT DECK BEAM AND SIMP. PB68 AT BASE.
- 8"W x 12" TALL CONC. LINTEL WITH (2) #5 BARS AT TOP & (4) #5 BARS AT BTM. STEEL BARS MUST CONTINUE FOR FULL LENGTH OVER TOP OF WALL. USE #4 TIES @ 12" O.C.
- 8"W x 12" TALL CONC. LINTEL WITH (2) #4 BARS TOP AND BTM AND #4 TIES AT 12" O.C. EXTEND NUMBER 4 BARS CONTINUOUS OVER CONC. WALL EA. END.
- 8" TALL CONCRETE WALL WITH #4 BARS VERT. AT 12" O.C. & (2) #5 BARS VERTICAL AT JAMBS AND CORNERS.



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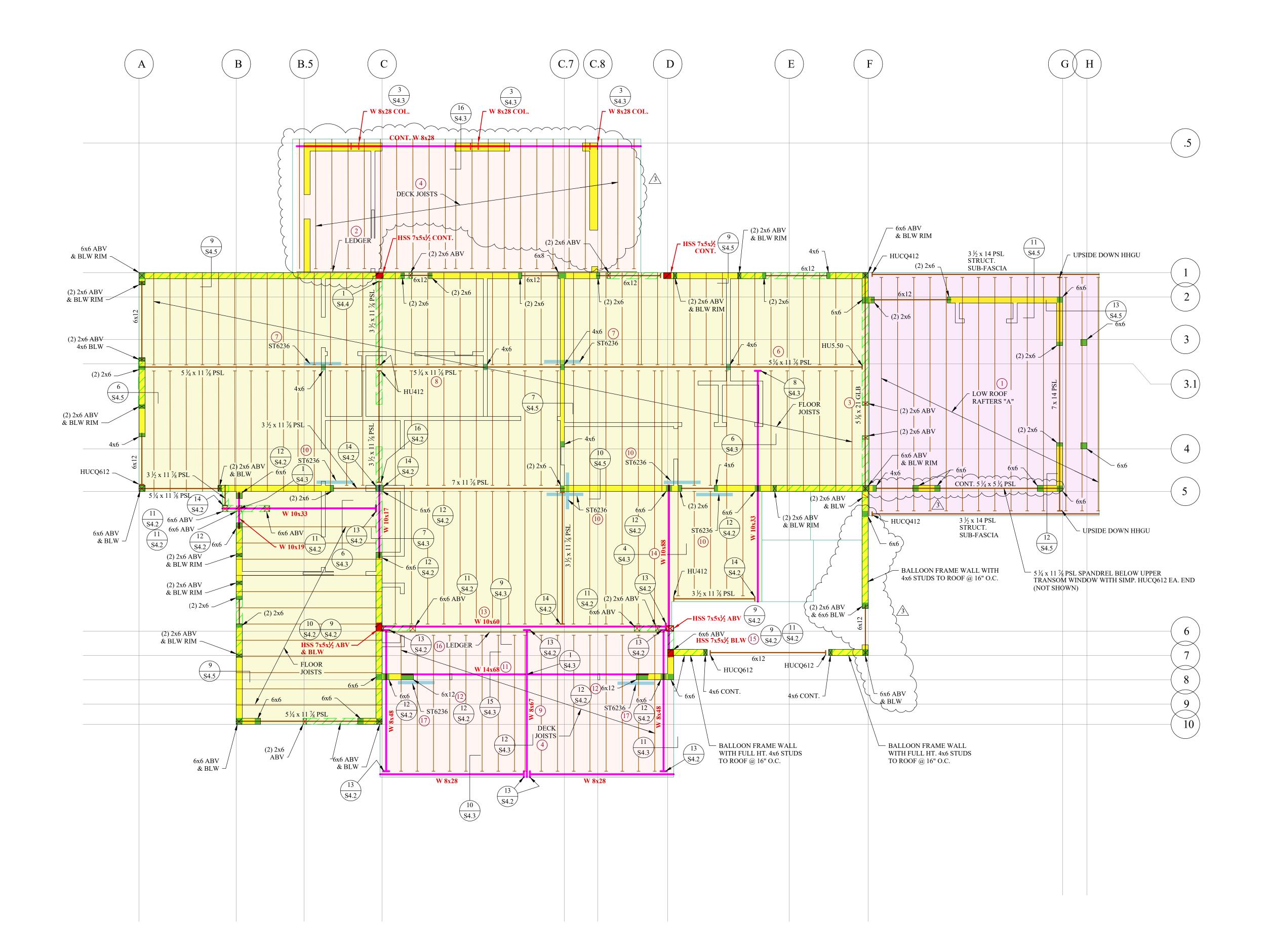
UPPER
FOUNDATION /
ENTRY ROOF /
MID FLOOR

FRAMING PLAN

DRAWING NO:

S2.3

SCALE: ½" = 1'-0"



#### FLOOR FRAMING NOTES

- 1. FLOOR SHT'G SHALL BE ¾" APA RATED T & G SHT'G. GLUE & NAIL WITH 8d NAILS AT 6",10" O.C.
- 2. FLOOR JOISTS SHALL BE 11  $\frac{7}{8}$ " TJI 360 @ 16" O.C.
- 3. JOIST HANGERS PERPENDICULAR TO BEAMS SHALL BE SIMP. IUS2.37 / 11.88 AT FACE MOUNT. AT TOP FLANGE CONDITIONS USE SIMP. ITS HANGERS (MAX).
- 4. PROVIDE STUD BELOW ALL JOISTS AT SUPPORTING WALLS.
- 5. EDGE NAIL FLOOR SHT'G TO ALL BLOCKING AND LATERAL COLLECTORS.
- 6. PROVIDE TJI JOIST OR BLK'G BELOW ALL PARTITION WALLS ABOVE.
- 7. PROVIDE DOUBLE LVL JOISTS BELOW ALL FIREPLACE HEARTHS.
- 8. MAXIMUM JOIST SPAN WITHOUT INTERMEDIATE TJI BLK'G IS

#### ROOF FRAMING NOTES

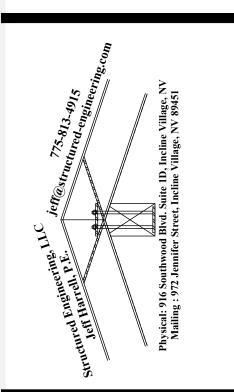
- 1. ROOF SHT'G SHALL BE % APA RATED ROOF SHT'G. NAIL WITH 8d NAILS AT 6",12" O.C.
- 2. PROVIDE STUD BELOW ALL RAFTERS.
- 3. TOP PLATE SHALL BE (2) 2x6. SPLICE WITH 4'-0" LAP SPLICE WITH .148" DIA. NAILS AT 4" O.C. OR STRAP AT SPLICE WITH SIMP. ST6236.
- 4. HEADERS WITH SPANS </= 4'-0" SHALL BE 6x8 DF#1 WITH (MIN) 2x6 TRIMMER EACH END (UNO).
- 5. HEADERS WITH SPANS > 4'-0" SHALL BE 6x12 DF#1 WITH (MIN) (2) 2x6 TRIMMERS EACH END (UNO).
- 6. OUTLOOKERS SHALL BE 6x12 DF#1 WITH 2:1 BACKSPAN, STRAP OUTLOOKER AT BACKSPAN TO TOP PLATE WITH FLAT ST6236 (UNO).
- 7. POSTS MAY BEAR DIRECTLY ON WOOD MEMBERS WITHOUT HARDWARE UNLESS SPECIFIED BY <u>"CPG"</u>.
- 8. PROVIDE SIMP. H2.5A AT ALL RAFTER / TRUSS ENDS TO TOP PLATE AT ALL EAVE CONDITIONS.
- 9. SEE SHEET 1.3 FOR TYPICAL DETAILS NOT CALLED OUT ON ROOF FRAMING PLAN.
- 10. SOFFIT REQUIRED AT ALL EXTERIOR LVL, LSL, PSL MEMBERS TO PROTECT FROM MOISTURE.
- 11. PROTECT ALL EXTERIOR GLB'S FROM MOISTURE OR USE EXTERIOR RATED WESTERN CEDARS GLB.
- 12. PROVIDE FULL DEPTH BLOCKING BETWEEN ALL FRAMING MEMBERS OVER ALL SHEAR WALLS AND EXTERIOR WALLS.
- 13. EDGE NAIL ROOF SHT'G TO ALL BLOCKING AND LATERAL COLLECTORS.
- 14. SEE SHEET S3.2 FOR SHEAR WALLS AND DIAPHRAGMS.
- 15. BUILDER SHALL CONTACT STRUCTURED ENGINEERING, LLC PRIOR TO PENTRATING ANY BEAM OR RAFTER.
- 1/ GEE CHEET CLA FOR WING CTUD COHEDING
- 16. SEE SHEET S1.3 FOR KING STUD SCHEDULE.

#### S2.4 KEYNOTES

- 1 LOW ROOF RAFTERS "A" SHALL BE 1 ¾ x 14 LVL @ 16" O.C. (MIN) WITH SIMP. HU14 HANGERS WHERE HUNG.
- (3) BMT OF GLB = BTM OF FLOOR JOISTS.
- DECK JOISTS SHALL BE 1 ¾ x 11 ¾ LVL JOISTS @ 16" O.C. (RIP TOP TO SLOPE @ ¼" / FT). (SOFFIT REQ'D AT EXT.) SHT'G SHALL BE ¾" T&G SHT'G WITH 8d NAILS @ 6", 10" O.C. SEE ARCH'L FOR FLASHING, WATERPROOFING AND DECK SUBSACE
- 0 W 10x100 SLOPES PER DECK JOISTS PER ARCH'L. BTM OF W 10x100 = BTM OF DECK JOISTS.
- 6 5 1/4 x 11 1/8 PSL CONTINUOUS FROM GRID C.7 TO GRID F.
- 7 SPLICE PSL BEAMS OVER 4x6 POST WITH SIMP. ST6236
- (8)  $5\frac{1}{4}$  x 11  $\frac{7}{8}$  PSL CONTINUOUS FROM GRID B.7 TO C.7
- 9 W 8x67 IN DECK CAVITY. SLOPE WITH DECK JOISTS PER ARCH'L.
- (10) ST6236 AT BTM OF PSL TO TOP OF TOP PLATE.
- (11) O.F. STEEL BEAM = O.F. STUD WALL AT GRID 8
- 12 STEEL BEAM OFFSET FROM C.L. OF 6x12 POST
- O.F. STEEL BEAM = GRID 6
- (14) O.F. STEEL BEAM = O.F. WALL ABOVE AT GRID D
- 15 STEEL BEAM OFFSET FROM C.L. HSS 7x5x½ COLUMN.
- 1 3/4 x 9 1/2 LVL LEDGER W/ (3) 1/4" DIA. x 4 1/2" SDS SCREWS @ 8" O.C.
- (17) ST6236 AT SIDE OF PACKOUT TO SIDE OF RIM JOIST.



Custom Residence fo
Whisper Homes LLC



REVISIONS:

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1 11-6-2024 REVISIONS

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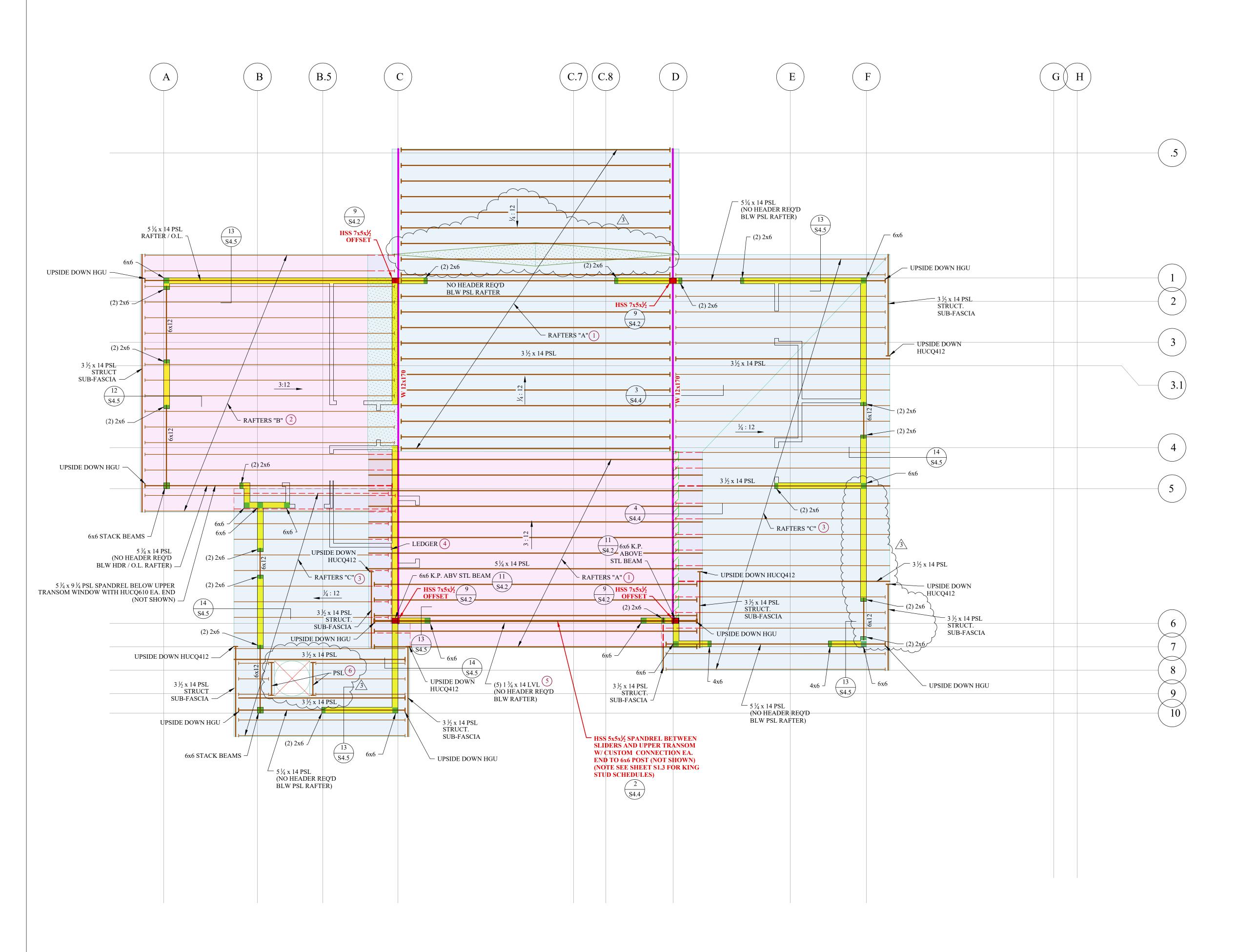
August 23, 2024

DRAWING TITLE:

LOW ROOF/ UPPER FLOOR FRAMING PLAN

DRAWING NO:

S2.4



#### ROOF FRAMING NOTES

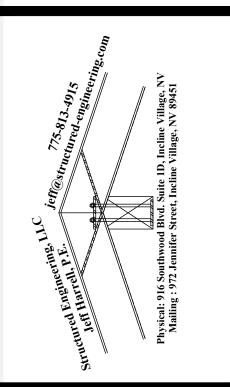
- 1. ROOF SHT'G SHALL BE % " APA RATED ROOF SHT'G. NAIL WITH 8d NAILS AT 6",12" O.C.
- 2. PROVIDE STUD BELOW ALL RAFTERS.
- 3. TOP PLATE SHALL BE (2) 2x6. SPLICE WITH 4'-0" LAP SPLICE WITH .148" DIA. NAILS AT 4" O.C. OR STRAP AT SPLICE WITH SIMP. ST6236.
- 4. HEADERS WITH SPANS </= 4'-0" SHALL BE 6x8 DF#1 WITH (MIN) 2x6 TRIMMER EACH END (UNO).
- 5. HEADERS WITH SPANS > 4'-0" SHALL BE 6x12 DF#1 WITH (MIN) (2) 2x6 TRIMMERS EACH END (UNO).
- 6. OUTLOOKERS SHALL BE 6x12 DF#1 WITH 2:1 BACKSPAN, STRAP OUTLOOKER AT BACKSPAN TO TOP PLATE WITH FLAT ST6236 (UNO).
- 7. POSTS MAY BEAR DIRECTLY ON WOOD MEMBERS WITHOUT HARDWARE UNLESS SPECIFIED BY <u>"CPG"</u>.
- 8. PROVIDE SIMP. H2.5A AT ALL RAFTER / TRUSS ENDS TO TOP PLATE AT ALL EAVE CONDITIONS.
- 9. SEE SHEET 1.3 FOR TYPICAL DETAILS NOT CALLED OUT ON ROOF FRAMING PLAN.
- 10. SOFFIT REQUIRED AT ALL EXTERIOR LVL, LSL, PSL MEMBERS TO PROTECT FROM MOISTURE.
- 11. PROTECT ALL EXTERIOR GLB'S FROM MOISTURE OR USE EXTERIOR RATED WESTERN CEDARS GLB.
- 12. PROVIDE FULL DEPTH BLOCKING BETWEEN ALL FRAMING MEMBERS OVER ALL SHEAR WALLS AND EXTERIOR WALLS.
- 13. EDGE NAIL ROOF SHT'G TO ALL BLOCKING AND LATERAL COLLECTORS.
- 14. SEE SHEET S3.3 FOR SHEAR WALLS AND DIAPHRAGMS.
- 15. BUILDER SHALL CONTACT STRUCTURED ENGINEERING, LLC PRIOR TO PENTRATING ANY BEAM OR RAFTER.
- 16. SEE SHEET S1.3 FOR KING STUD SCHEDULES.

#### S2.5 KEYNOTES

- RAFTERS "A" SHALL BE 5 ¼ x 14 PSL @ 16" O.C. WITH SIMP. HU614 EA. END TO STEEL BEAM OR BEAR OVER 2x6 CRIPPLE WALL AT 3:12 ROOF. RAFTERS ROTATED TO BE SQUARE TO ROOF SLOPE.
- RAFTERS "B" SHALL 3 ½ x 14 PSL @ 16" O.C. WITH SIMP. HU414 OR BEAR ON WALLS AND HEADERS.
- RAFTERS "C" SHALL BE 1 ¾ x 14 LVL RAFTERS @ 16" O.C. WITH SIMP. HU14 HANGERS WHERE HUNG. BEAR ON WALLS AND HEADERS.
- 1 ¾ x 14 LVL LEDGER W/ (5) ¼" DIA. x 4 ½" SDS SCREWS @ 16" O.C. TO STUDS OR STL BEAM PACKOUT.
- (5)  $1\frac{3}{4}$  x 14 LVL SISTERED TOGETHER WITH (3) .148" DIA. NAILS @ 8" O.C. AT EA. FACE.
- 6 HEAD OUT SKYLIGHT WITH 3 ½ x 14 PSL WITH SIMP. HU412 EA. END.



Custom Residence for Whisper Homes LLC



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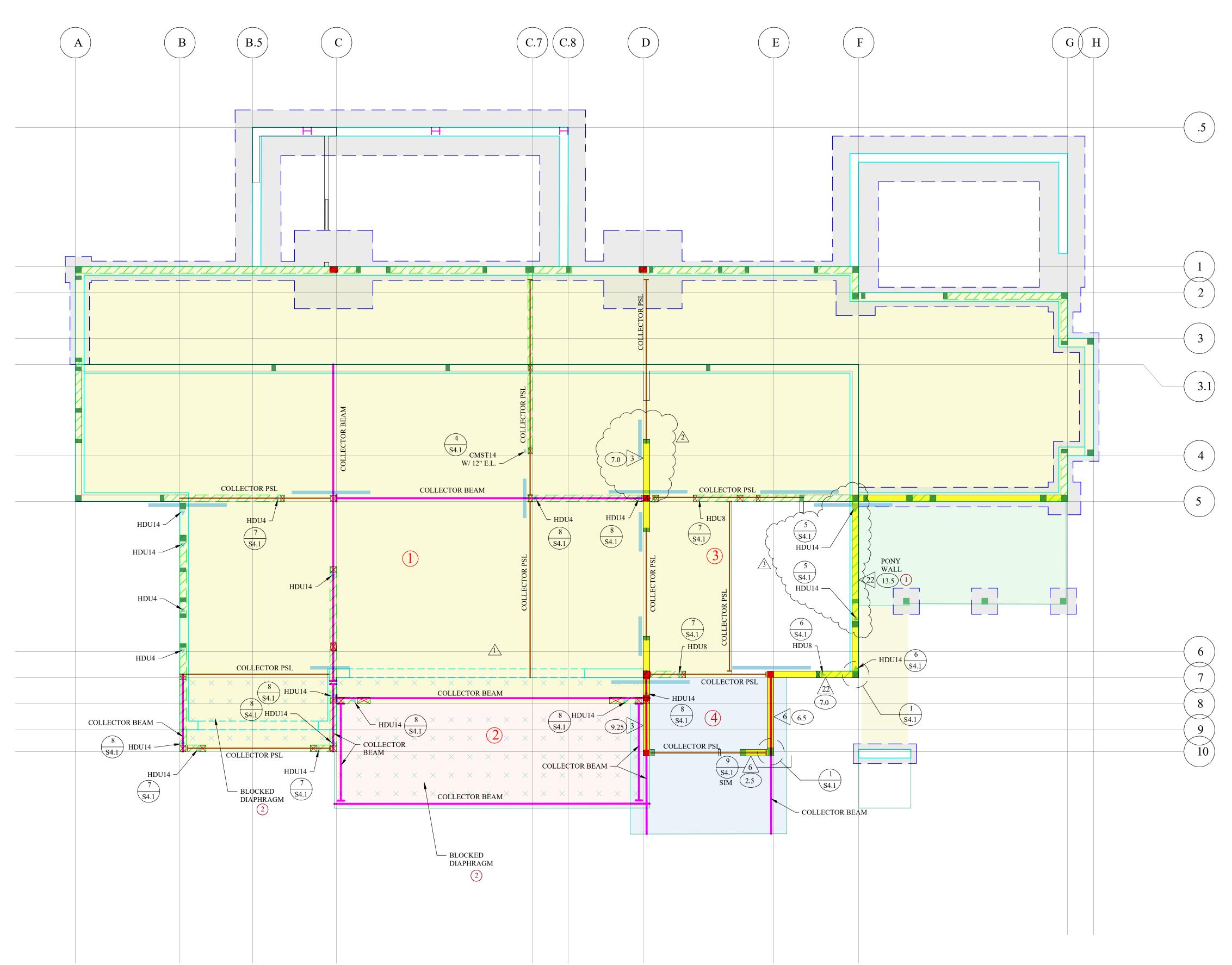
August 23, 2024

DRAWING TITLE:

UPPER ROOF FRAMING PLAN

DRAWING NO:

S2.5



NOTE: SEE SHEET S2.3 FOR HOLDOWNS AND ANCHOR BOLTS THAT OCCUR AT TOP OF STEMWALLS AND RETAININGS WALLS AT THIS LEVEL.

#### DIAPHRAGM / SHEARWALL NOTES

- 1. ROOF DIAPHRAGM SHALL BE \%" APA RATED ROOF SHT'G. NAIL WITH 8d NAILS AT 6",12" O.C. (UNO).
- SHEAR WALLS SHALL BE 1/2" LP FLAME BLOCK FIRE-RATED OSB SHEATHING. NAIL PER SHEAR WALL SCHEDULE.
- PROVIDE EDGE NAILING AT HORIZONTAL DIAPHRAGMS TO ALL COLLECTOR JOISTS, BEAMS, RAFTERS, AND BLOCKING. ADDITIONALLY, PROVIDE EDGE NAILING AT HORIZONTAL DIAPHRAGMS TO ALL BLOCKING AT EXTERIOR WALLS.
- 4. SEE FRAMING PLANS FOR COLLECTOR SIZES AND STRAP CALLOUTS.
- 5. VERTICAL STRAPS CALLED OUT ON THIS PLAN OCCUR ACROSS THIS FRAMING LEVEL.
- 6. DOUBLE TOP PLATES SHALL HAVE A MINIMUM 48" LAP SPLICE WITH (2) ROWS .148" DIA. x 3" NAILS AT 6" O.C. OR STRAPPED WITH SIMPSON ST6236.
- 7. EDGE NAIL SHEAR PLY TO ALL STUDS/POSTS WHICH HAVE HOLDOWNS OR VERTICAL STRAPS ATTACHED.
- 8. EDGE NAIL SHEAR PLY TO ALL STUDS/POSTS WHICH ARE ATTACHED TO STEEL COLUMNS. IF STEEL COLUMN OCCURS WITHIN SHEARWALL OR AT THE END OF A SHEAR WALL, THE WOOD MEMBER ALONG SIDE THE STEEL COLUMN SHALL BE A MINIMUM 4x6 WITH \( \frac{5}{8} \)" DIA. STUD BOLTS AT 16" O.C. VERT.
- WHERE POSTS / COLUMNS INTERRUPT TOP PLATE, PROVIDE ST6236 STRAP ACROSS POST / COLUMN FROM TOP PLATE TO TOP PLATE.

#### S3.1 KEYNOTES

- (1) PONY WALL SHALL MATCH SHEAR PLY ABOVE.
- BLOCK ENTIRE HATCHED REGION WITH 4x6 FLAT BLK'G. NAIL WITH 8d NAILS @ 4", 10" O.C.

NOTE: LARGE NUMBERS IN RED CIRCLES ARE NOT INTENDED FOR BUILDER. THEY ARE FOR LATERAL REGIONS IN CALCULATIONS

#### SHEAR WALL SCHEDULE

NAILS SHALL BE 3"x.148" COMMON OR 3" x .128" GALVANIZED BOX. MINIMUM PENETRATION INTO FRAMING MEMBER IS 1-½"

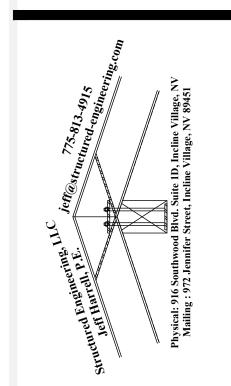
SW TYPE	SHEAR WALL SHEATHING THICKNESS	STUDS AT ADJOINING PANEL EDGES	MUDSILL ON CONCRETE OR MASONRY	SOLE PLATE ON SUBFLOOR	CAPACITY (SEISMIC)
6	1/2"	2x	2x	2x	310 PLF
4	1/2"	2x	2x	2x	350 PLF
$\sqrt{3}$	1/2"	3x	3x	2x	665 PLF
2	1/2"	3x	3x	2x	870 PLF
44	1/2"	3x	3x	2x	1020 PLF
33	1/2"	3x	3x	2x	1330 PLF
22	1/2"	3x	3x	2x	1740 PLF
1					

#### NOTES:

- Shear ply shall be APA Structural I rated sheathing.

  Shear ply at field conditions must be attached to 2x6 studs at 16" o.c. with nails at 12" o.c. Nails must be of the same type at
- Nails at double sided shearwalls must be stag'd.
- Shear ply at double sided shear walls must have panel edges stag'd so that no more than one joint occurs at any interior stud. Attachment of both shear panels to sole plate at double sided shear wall is not allowed. Exterior shear ply must continue to rim
- Engineer if exception is required.
- Block all panel edges. At shearwalls more than one panel in height, stagger layup to eliminate continuous horizontal seam.

  Contact Engineer for alternate to 3x studs at adjoining panels if required. Substitution of (2) 2x members for 3x member is not
- 9. Contact Engineer if wood splitting occurs due to edge nailing. 10. Provide edge nailing to all members that have holdowns or vertical straps attached.
- 11. Do not allow nail heads to penetrate shear ply. 12. Number in triangle indicates edge nailing with 10D nails. Double number in triangle indicates shear ply both sides with edge nailing both sides with 10D nails.



REVISIONS:

/1\ 11-6-2024 REVISIONS /2 5-7-2025 REVISIONS /3\ 8-18-2025 REVISIONS

PROJECT NO: 240601 ISSUE DATE:

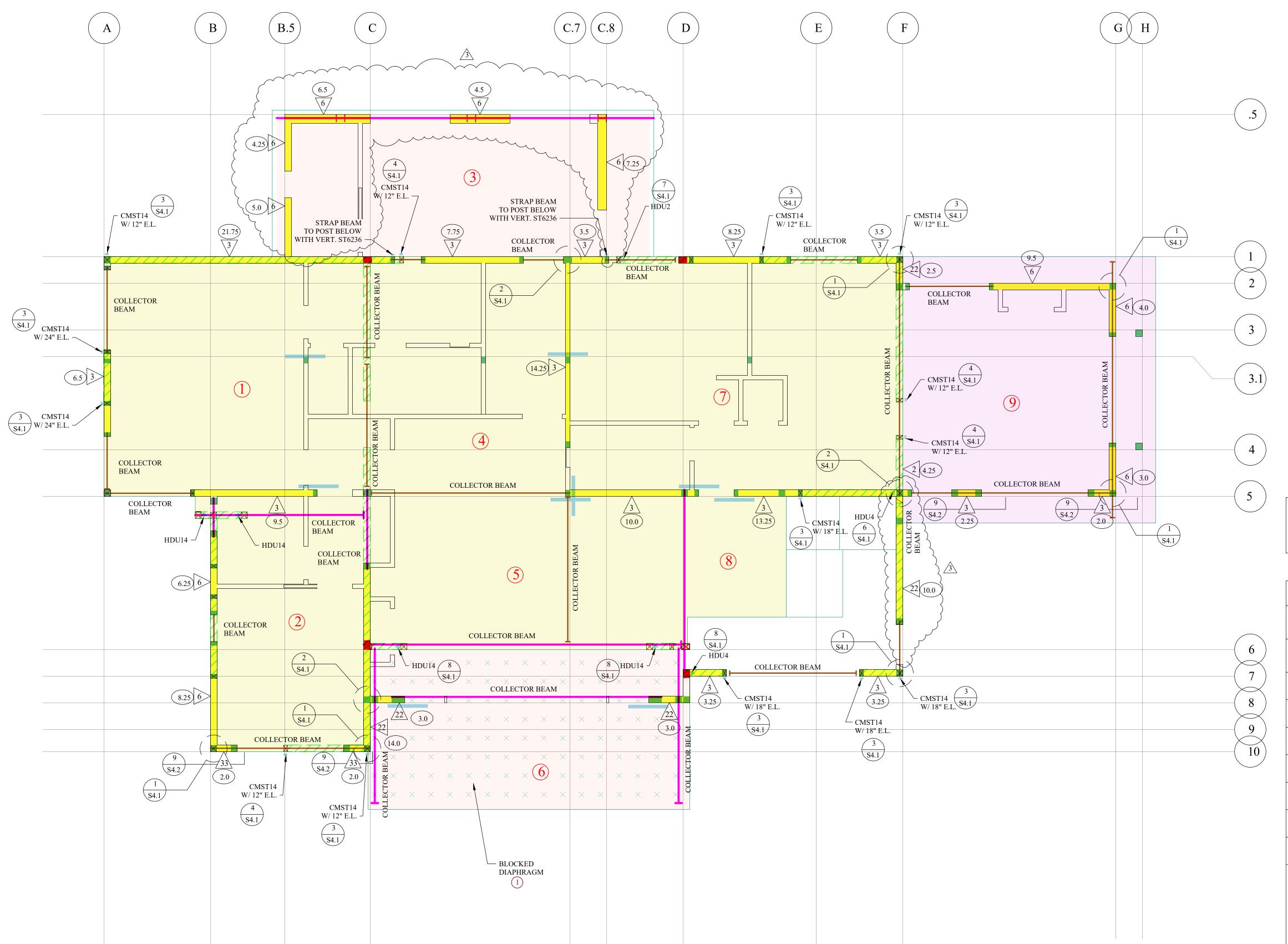
August 23, 2024

DRAWING

TITLE: ENTRY ROOF/ MID FLOOR DIAPHRAGM **PLAN** 

DRAWING NO:

S3.1



#### DIAPHRAGM / SHEARWALL NOTES

- 1. ROOF DIAPHRAGM SHALL BE %" APA RATED ROOF SHT'G. NAIL WITH 8d NAILS AT 6",12" O.C. (UNO).
- 2. SHEAR WALLS SHALL BE ½" LP FLAME BLOCK FIRE-RATED OSB SHEATHING. NAIL PER SHEAR WALL SCHEDULE.
- PROVIDE EDGE NAILING AT HORIZONTAL DIAPHRAGMS TO ALL COLLECTOR JOISTS, BEAMS, RAFTERS, AND BLOCKING.
- 4. SEE FRAMING PLANS FOR COLLECTOR SIZES AND STRAP CALLOUTS.
- 5. VERTICAL STRAPS CALLED OUT ON THIS PLAN OCCUR ACROSS THIS FRAMING LEVEL.
- 6. DOUBLE TOP PLATES SHALL HAVE A MINIMUM 48" LAP SPLICE WITH (2) ROWS .148" DIA. x 3" NAILS AT 6" O.C. OR STRAPPED WITH SIMPSON ST6236.

ADDITIONALLY, PROVIDE EDGE NAILING AT HORIZONTAL DIAPHRAGMS TO ALL BLOCKING AT EXTERIOR WALLS.

- 7. EDGE NAIL SHEAR PLY TO ALL STUDS/POSTS WHICH HAVE HOLDOWNS OR VERTICAL STRAPS ATTACHED.
- 8. EDGE NAIL SHEAR PLY TO ALL STUDS/POSTS WHICH ARE ATTACHED TO STEEL COLUMNS. IF STEEL COLUMN OCCURS WITHIN SHEARWALL OR AT THE END OF A SHEAR WALL, THE WOOD MEMBER ALONG SIDE THE STEEL COLUMN SHALL BE A MINIMUM 4x6 WITH \%" DIA. STUD BOLTS AT 16" O.C. VERT.
- 9. WHERE POSTS / COLUMNS INTERRUPT TOP PLATE, PROVIDE ST6236 STRAP ACROSS POST / COLUMN FROM TOP PLATE TO TOP PLATE.

#### S3.2 KEYNOTES

BLOCK ENTIRE HATCHED REGION WITH 4x6 FLAT BLK'G AT ALL UNSUPPORTED EDGES. NAIL WITH 8d NAILS AT 4", 10"

NOTE: LARGE NUMBERS IN RED CIRCLES ARE NOT INTENDED FOR BUILDER. THEY ARE FOR LATERAL REGIONS IN CALCULATIONS

#### SHEAR WALL SCHEDULE

NAILS SHALL BE 3"x.148" COMMON OR 3" x .128" GALVANIZED BOX. MINIMUM PENETRATION INTO FRAMING MEMBER IS 1-1/2"

TRUMING MEMBER 13 172						
SW TYPE	SHEAR WALL SHEATHING THICKNESS	STUDS AT ADJOINING PANEL EDGES	MUDSILL ON CONCRETE OR MASONRY	SOLE PLATE ON SUBFLOOR	CAPACITY (SEISMIC)	
6	1/2"	2x	2x	2x	310 PLF	
4	1/2"	2x	2x	2x	350 PLF	
3	1/2"	3x	3x	2x	665 PLF	
2	1/2"	3x	3x	2x	870 PLF	
44	1/2"	3x	3x	2x	1020 PLI	
33	1/2"	3x	3x	2x	1330 PLI	
22	1/2"	3x	3x	2x	1740 PLI	
NOTES:	1		1			

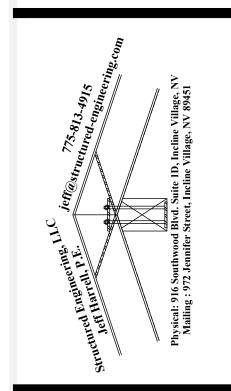
- Shear ply shall be APA Structural I rated sheathing.
- Shear ply at field conditions must be attached to 2x6 studs at 16" o.c. with nails at 12" o.c. Nails must be of the same type at Nails at double sided shearwalls must be stag'd.
- Shear ply at double sided shear walls must have panel edges stag'd so that no more than one joint occurs at any interior stud.

  Attachment of both shear panels to sole plate at double sided shear wall is not allowed. Exterior shear ply must continue to rim joist. Contact
- Engineer if exception is required. Block all panel edges.
- At shearwalls more than one panel in height, stagger layup to eliminate continuous horizontal seam.

  Contact Engineer for alternate to 3x studs at adjoining panels if required. Substitution of (2) 2x members for 3x member is not
- 9. Contact Engineer if wood splitting occurs due to edge nailing.
  10. Provide edge nailing to all members that have holdowns or vertical straps attached.
- 11. Do not allow nail heads to penetrate shear ply.

  12. Number in triangle indicates edge nailing with 10D nails. Double number in triangle indicates shear ply both sides with edge nailing both sides with 10D nails.





REVISIONS:

/1\ 11-6-2024 REVISIONS /2 5-7-2025 REVISIONS

/3\ 8-18-2025 REVISIONS

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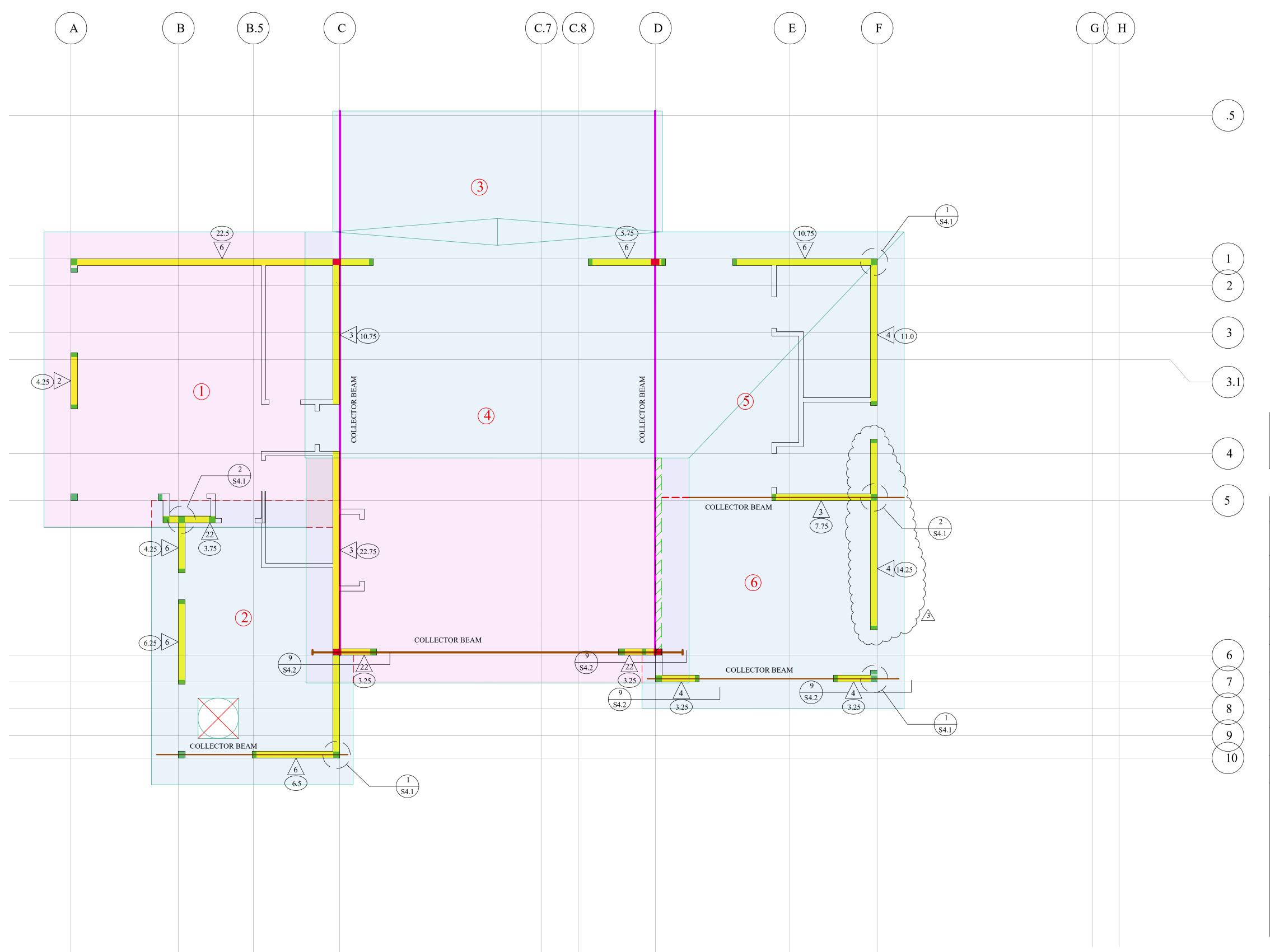
August 23, 2024

DRAWING

TITLE:

LOW ROOF/ UPPER FLOOR DIAPHRAGM **PLAN** 

DRAWING NO:



#### DIAPHRAGM / SHEARWALL NOTES

- . ROOF DIAPHRAGM SHALL BE  $\frac{5}{8}$ " APA RATED ROOF SHT'G. NAIL WITH 8d NAILS AT 6",12" O.C. (UNO).
- 2. SHEAR WALLS SHALL BE ½" LP FLAME BLOCK FIRE-RATED OSB SHEATHING. NAIL PER SHEAR WALL SCHEDULE.
- 3. PROVIDE EDGE NAILING AT HORIZONTAL DIAPHRAGMS TO ALL COLLECTOR JOISTS, BEAMS, RAFTERS, AND BLOCKING. ADDITIONALLY, PROVIDE EDGE NAILING AT HORIZONTAL DIAPHRAGMS TO ALL BLOCKING AT EXTERIOR WALLS.
- 4. SEE FRAMING PLANS FOR COLLECTOR SIZES AND STRAP CALLOUTS.
- 5. VERTICAL STRAPS CALLED OUT ON THIS PLAN OCCUR ACROSS THIS FRAMING LEVEL.
- 6. DOUBLE TOP PLATES SHALL HAVE A MINIMUM 48" LAP SPLICE WITH (2) ROWS .148" DIA. x 3" NAILS AT 6" O.C. OR STRAPPED WITH SIMPSON ST6236.
- 7. EDGE NAIL SHEAR PLY TO ALL STUDS/POSTS WHICH HAVE HOLDOWNS OR VERTICAL STRAPS ATTACHED.
- 8. EDGE NAIL SHEAR PLY TO ALL STUDS/POSTS WHICH ARE ATTACHED TO STEEL COLUMNS. IF STEEL COLUMN OCCURS WITHIN SHEARWALL OR AT THE END OF A SHEAR WALL, THE WOOD MEMBER ALONG SIDE THE STEEL COLUMN SHALL BE A MINIMUM 4x6 WITH \[ \frac{5}{8} \] DIA. STUD BOLTS AT 16" O.C. VERT.

#### S3.3 KEYNOTES

1) BLOCK ENTIRE HATCHED REGION WITH 1-3/4 x 5-1/2 LSL FLAT BLK'G AT ALL UNSUPPORTED EDGES. NAIL WITH 8d NAILS AT

NOTE: LARGE NUMBERS IN RED CIRCLES ARE NOT INTENDED FOR BUILDER. THEY ARE FOR LATERAL REGIONS IN CALCULATIONS

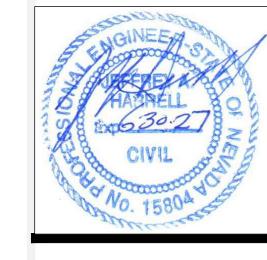
#### SHEAR WALL SCHEDULE

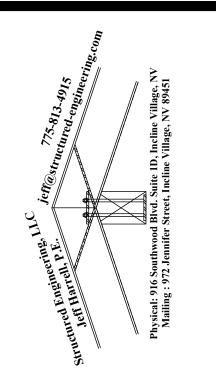
NAILS SHALL BE 3"x.148" COMMON OR 3" x .128" GALVANIZED BOX. MINIMUM PENETRATION INTO

FRAMING MEMBER IS 1-½"						
SW TYPE	SHEAR WALL SHEATHING THICKNESS	STUDS AT ADJOINING PANEL EDGES	MUDSILL ON CONCRETE OR MASONRY	SOLE PLATE ON SUBFLOOR	CAPACITY (SEISMIC)	
6	1/2"	2x	2x	2x	310 PLF	
4	1/2"	2x	2x	2x	350 PLF	
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2	1/2"	3x	3x	2x	870 PLF	
44	1/2"	3x	3x	2x	1020 PLF	
33	1/2"	3x	3x	2x	1330 PLF	
22	1/2"	3x	3x	2x	1740 PLF	

- Shear ply shall be APA Structural I rated sheathing.
- Shear ply at field conditions must be attached to 2x6 studs at 16" o.c. with nails at 12" o.c. Nails must be of the same type at
- Nails at double sided shearwalls must be stag'd.
  Shear ply at double sided shear walls must have panel edges stag'd so that no more than one joint occurs at any interior stud. Attachment of both shear panels to sole plate at double sided shear wall is not allowed. Exterior shear ply must continue to rim
- joist. Contact Engineer if exception is required. Block all panel edges.
- 7. At shearwalls more than one panel in height, stagger layup to eliminate continuous horizontal seam.

  8. Contact Engineer for alternate to 3x studs at adjoining panels if required. Substitution of (2) 2x members for 3x member is not
- 9. Contact Engineer if wood splitting occurs due to edge nailing.10. Provide edge nailing to all members that have holdowns or vertical straps attached. 11. Do not allow nail heads to penetrate shear ply.
- 12. Number in triangle indicates edge nailing with 10D nails. Double number in triangle indicates shear ply both sides with edge nailing both sides with 10D nails.





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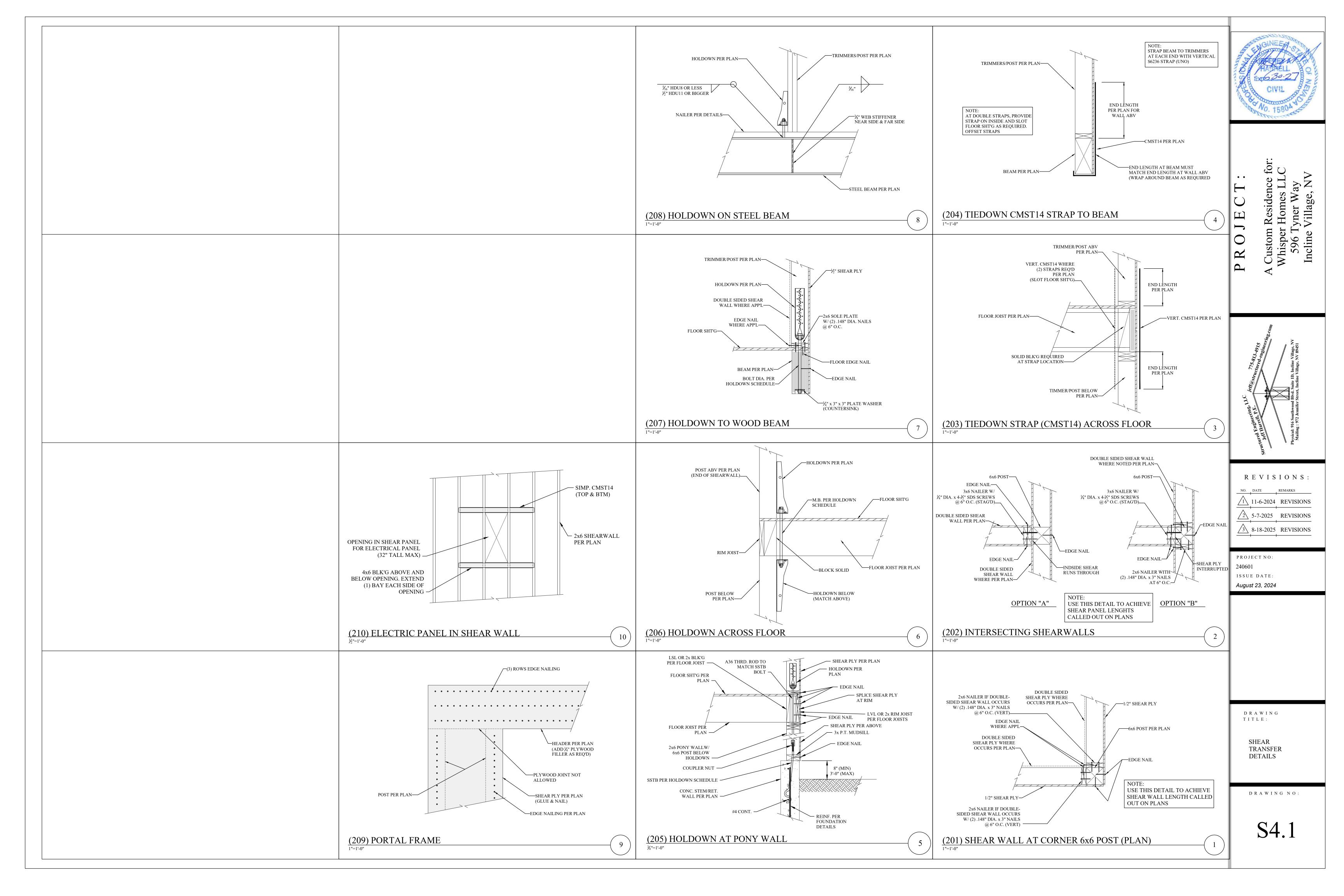
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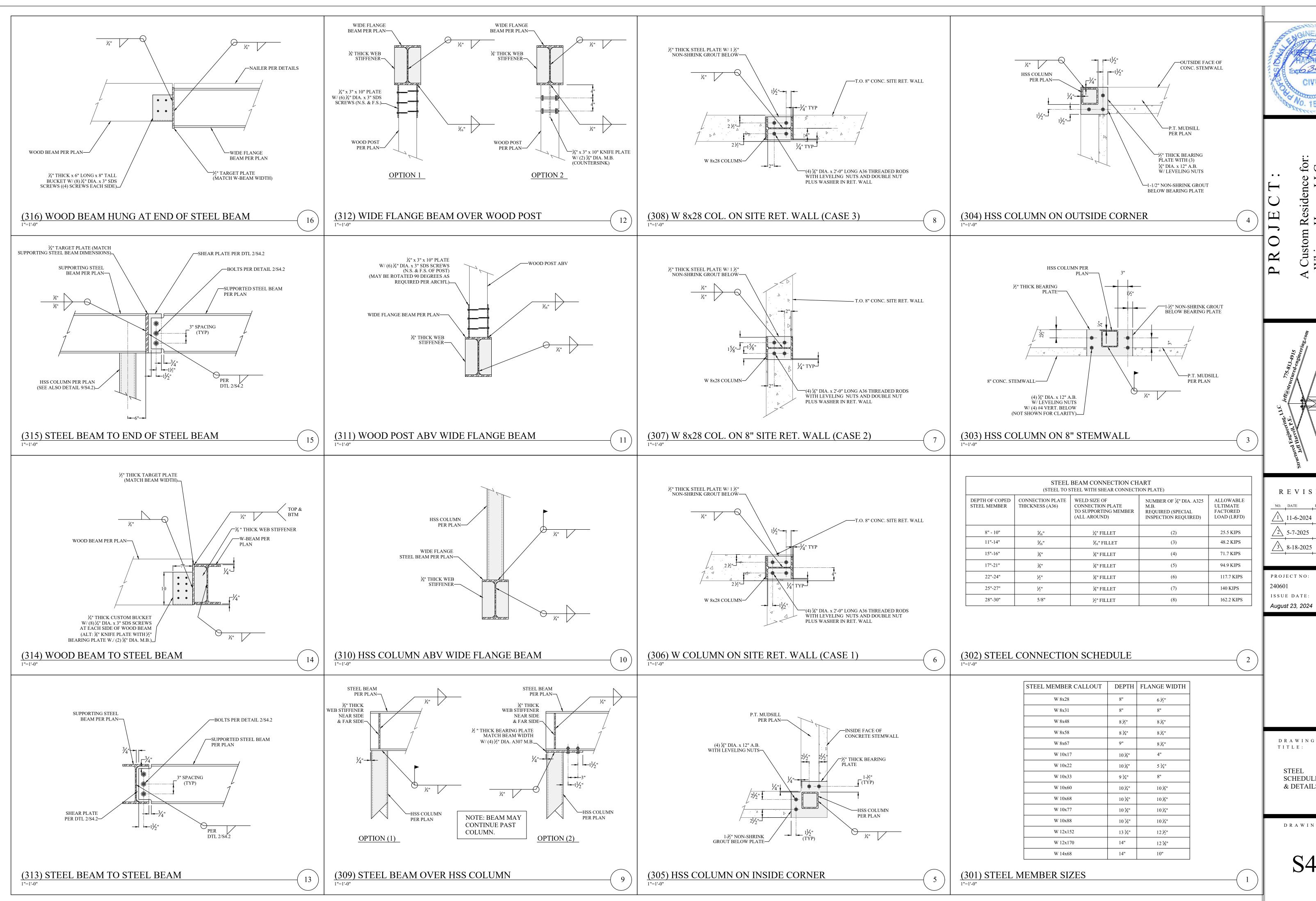
> DRAWING TITLE:

**UPPER ROOF** DIAPHRAGM **PLAN** 

DRAWING NO:

S3.3





REVISIONS: REMARKS /1\ 11-6-2024 REVISIONS

<u>∕2</u>\ 5-7-2025 REVISIONS /3\\ 8-18-2025 REVISIONS

ISSUE DATE: August 23, 2024

TITLE:

**SCHEDULES** & DETAILS

DRAWING NO:

